

E

Type

✓

Plans

Sep 14-0161

Permit Number

9141

Street Number

Green Valley Rd

Street Name

Cr A

Community Code

130-090-020

APN

**APPLICATION FOR  
CLEARANCE FOR:  
*AD D LF***

**PERMIT & RESOURCE MANAGEMENT DEPARTMENT - WELL & SEPTIC SECTION  
2550 VENTURA AVENUE, SANTA ROSA, 95403 TELEPHONE (707) 565-1900**

**APPLICATION FOR PRIVATE  
SEWAGE DISPOSAL PERMIT**

**APPLICATION FOR  
CLEARANCE FOR:**

Application is hereby made to the Permit & Resource Management Department for a permit to construct or repair a sewage disposal system as described below in compliance with code of Sonoma County or for clearance for other construction.

APPLICANT: PLEASE PRESS HARD (USE BLACK INK).  
FILL IN BETWEEN HEAVY LINES ONLY.  
SEE BACK SIDE FOR TERMS OF PERMIT

9141 GREEN VALLEY RD  
SUITE 100  
SAN MARCOS, CA 92078

JOB ADDRESS 111 N. CROSS ST. ROSS RD  
NEAREST CROSS STREET

ASSESSOR'S PARCEL No. 130-090-020

SUBDIVISION \_\_\_\_\_  
CITY GRATION / SEBASTOPOL

SEWAGE DISPOSAL SYSTEM CONTRACTOR Joe P. E. K.  
ADDRESS

**GENERAL CONTRACTOR**

## **OWNER BILL DECI ABATION**

I hereby affirm under penalty of perjury that I am exempt from the Contractor's License Law for the following reason (Sec. 7031.5, Business and Professions Code: Any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractor's License Law (Chapter 9 (commencing with Section 700) of Division 3 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars (\$500.00). V

I, as owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044 Business and Professions Code). The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself through his or her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he or she did

I am exempt under  
Section 7044, R.C. for this reason:  
**SONOMA COUNTY**

Layout Plan  
Approved by:  
J. C. Thomas

Senitis F:\FORMS\WSSWSS-013 in process  
rev. 1/22/05

This permit application must be signed on pertinent signature lines by the same person (i.e., contractor or owner/builder). A letter of authorization from the owner must accompany this application if an agent is signing on the owner's behalf.

APPLICANT: PLEASE PRESS HARD (USE BLACK INK).  
FILL IN BETWEEN HEAVY LINES ONLY.  
SEE BACK SIDE FOR TERMS OF PERMIT

914/GREEN VALLEY

JOB ADDRESS 111 N. CROSS ST. ROSS RD  
NEAREST CROSS STREET

ASSESSOR'S PARCEL No. 130-090-020

SUBDIVISION \_\_\_\_\_  
CITY GRATION / SEBASTOPOL

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rev. 1/22/05

SEP 14 0161

# PEARSON & ASSOCIATES

## CIVIL ENGINEERING SERVICES

April 2, 2014  
P/A Job: 8.2581

County of Sonoma  
Resources Management Dept.  
Well and Septic Section  
2550 Ventura Avenue  
Santa Rosa, Calif. 95403

Subject: Update of 3/24/14  
Letter submitted  
for Office Replacement  
Manzana Products  
9141 Green Valley Road

Attn.: Mario Kalson, R.S.

Per our subject site inspections; the existing septic system was determined to meet current code and therefor was planned to be used (with expansion) for the new office building. The existing leachline looked like it has been functioning and appears to be still in good condition and the 1200 gal. septic tank had very little solids. Based on the existing septic system permitted plans, and the subject site reviews, the existing system has a 75' long, 4' $\pm$  deep leachline with 2' of rock below the pipe.

Our client has asked that we move the up-dated septic system from the location shown on the 1/10/14 plans submitted last week to the location of the shown expansion mound area. This chance is so the driveway area where the leachlines exist and were proposed will not be lost for the clients use of parking and driveway width.

Based on the soils found in the backhoe pits done at the site reviews and 10/22/02 pits "A" & "B" done for perc testing of 5/4/07 (ave. perc rate was 40 mpi at 30" and 28 mpi at 66"), I propose using the 40 mpi rate for sizing of a mound for the office septic system (0.487 gal./s.f./day).

The planned new office building will replace the existing office use that has been up to 10 employees. The existing office will only have one to two employee for the scales and be mostly used for storage. The planned new office building will be connected to the existing septic system tank & will have up to 10 employees.

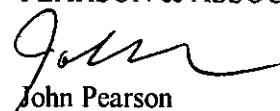
Up to ten employees is what the current plan use is for the new office and two employees in the existing office; for a factor of safety, the system is being designed for up to 15 employees. Sizing of the mound for 15 employees = 15 employees x 15 gal./employee/day = 225 GPD. See the attached mound calculations for mound sizing and pumping needs.

The plans dated 4/2/14 submitted with this letter are for a mound design with 200% expansion mounds, all to be in the area of the 3/27/13 ground water pit "B" (& 10/22/02 pits "A" and "B").

Let me know if I need to provide any additional information regarding the above.

Very truly yours,

PEARSON & ASSOCIATES

  
John Pearson

attachments: copy of 3/24/14 letter  
mound sizing calcs.

copy: client, file



**PEARSON & ASSOCIATES**  
**CIVIL ENGINEERING SERVICES**

March 24, 2014  
P/A Job: 8.2581

County of Sonoma  
Resources Management Dept.  
Well and Septic Section  
2550 Ventura Avenue  
Santa Rosa, Calif. 95403

Subject: Update of  
8/25/2012 Site Review  
for Office Replacement  
Manzana Products  
9141 Green Valley Road

Attn.: Mario Kalson, R.S.

Per our subject site inspections; the existing septic system meets current code and therefore is planned to be used (with expansion) for the new office building. The existing leachline looked like it has been functioning and appears to be still in good condition and the 1200 gal. septic tank had very little solids. Based on the existing septic system permitted plans, and the subject site reviews, the existing system has a 75' long, 4'± deep leachline with 2' of rock below the pipe. See the attached data.

Based on the soils found in the backhoe pits done at the site reviews and 10/22/02 pits "A" & "B" done for perc testing of 5/4/07 (ave. perc rate was 40 mpi at 30" and 28 mpi at 66"), I propose using the 40 mpi rate for sizing of this office septic system (0.487 gal./s.f./day).

The planned new office building will replace the existing office use that has been up to 10 employees. The existing office will only have one to two employee for the scales and be mostly used for storage. The planned new office building will be connected to the existing septic system & will have up to 10 employees.

Up to ten employees is what the current plans are for the new office and two employees in the existing office; for a factor of safety, the system is being designed for up to 15 employees. Sizing of the septic system leachline for 15 employees = 15 employees x 15 gal./employee/day

$$0.487 \text{ gpd/s.f.} \times 4 \text{ s.f./ft. of trench.} = 116 \text{ feet needed.}$$

The submitted plans include increasing the existing leachfield size by adding a leachline to the north. Added leachline needed is: 116' less the existing 75' = 41'; the plan is to add 70'. Future replacement systems will be in the area of the 3/27/13 ground water pit "B" (& 10/22/02 pits "A" and "B") and be a mound type design based on the 40 mpi infiltration rate; see the attached sizing chart.

Let me know if I need to provide any additional information regarding the above.

Very truly yours,

**PEARSON & ASSOCIATES**

John Pearson

attachments: 8/23/12 & 3/27/13 site inspection data  
mound sizing chart

copy: client, file

**County of Sonoma**  
**Permit & Resource Management Department**  
Well & Septic Section  
2550 Ventura Avenue, Santa Rosa, CA 95403-2829  
(707) 565-1900

**SEPTIC SYSTEM INSPECTION**

Site Address:

9141 Green Valley Rd

Owner:

SEP14-0161

**REQUEST FOR INSPECTION**

Date of call: \_\_\_\_\_ Time: \_\_\_\_\_

Caller: \_\_\_\_\_

Caller's Phone No.: \_\_\_\_\_

Remarks: For Final: 1.) OPR 2) 189  
3.) Engineers letter 4) Final cover/erosion  
control 5.) Monitoring wells 6.) start-up.

Call taken by: \_\_\_\_\_

**INSPECTION NOTICE**

- Stop work immediately - Call Environmental Health Specialist  
Telephone \_\_\_\_\_ Hours \_\_\_\_\_
- OK to cover leachfield  tank
- Provide Engineer's letter of approval
- Provide "As Built" plan to scale
- Call for inspection on pump & alarm
- Corrections needed - see remarks below
- OK to cover with Engineer's approval
- Issue Operational Permit

For further information call: \_\_\_\_\_

Hours & Day: \_\_\_\_\_

Remarks: Tank sizes ok. Watertight  
pass. Floats ok. Alarm audio/  
visual working. Line lengths  
ok/level. Squirt ok.

Environmental Health Specialist's Signature

Date

Received by: J-JESUS FONCE

Contractor's Signature

- Posted

**PEARSON & ASSOCIATES**  
**CIVIL ENGINEERING SERVICES**

September 6, 2015  
P/A Job: 8.2581

County of Sonoma  
Permit and Resources Management Dept.  
Environmental Health Division  
2550 Ventura Avenue  
Santa Rosa, California 95403

Subject: Inspection of Septic  
System Installation  
9141 Green Valley Road,  
Graton, Cal.

Attention: Chris Balanesi, R.S.

Gentlemen:

This letter provides the conclusions of our inspections of the septic system installed at the above subject site. We previously designed a mound type sewage disposal system for this site (dated 4/2/14) and this was approved by your office.

No notable changes in the installed septic system from the original design were found during construction inspection; therefore, the original design plans can be considered as the as-built plans. Note that the septic tank and pump tank were moved to the south of the mound instead of above the mound. After our 10/30/14 inspection the monitoring wells were placed and inspected along with the control box and seeding of the construction area. The elec. #189 inspection may still need to be done along with payment of the operational permit fee.

Based on our inspections, it appears that the septic system was installed in conformance with the intent of the approved plan and Sonoma County Public Health accepted standards and should function properly under normal loading conditions.

If there are any questions regarding the above, please call.

Very truly yours,

PEARSON & ASSOCIATES

  
John Pearson, R.C.E.

copy: client 1  
file 1

PROJECT: 9141 GREEN VALLEY RD - GRATON JOB No.: 8.2581  
OFFICE REPLACEMENT. DATE: 4-2-14

## CALCULATIONS FOR MOUND SEWAGE DISPOSAL SYSTEM

SEE "DESIGN AND CONSTRUCTION MANUAL FOR WISCONSIN MOUNDS, BY JAMES C. CONVERSE, 1978 AND "SITING, DESIGN, CONSTRUCTION AND PERFORMANCE FOR WISCONSIN MOUNDS" BY JAMES C. CONVERSE / E. JERRY TYLER, 1984

## I. SYSTEM DESIGN DATA:

AVERAGE SLOPE = 4 %      15 WORKERS X 15 GPD/W. = 225 GPD ✓  
BEDROOMS: WASTE LOAD = x 120 G.P.D. = 225 G.P.D.  
AVERAGE PERCOLATION RATE: 40 M.P.I. (BASED ON PITS & 5/4/07 PERCS)  
USE DESIGN LOAD RATE OF: 0.487 GAL/S.F./DAY ✓  
DEPTH TO GROUND WATER = 3' + \* INCHES. ✓  
\* NO INDICATION OF GR. WATER 3/27/13.

## II. DESIGN:

GRAVEL BED ABSORPTION AREA USING MED. TEXTURE SAND (0.8 G.P.D./S.F.) AND 225 G.P.D.  
AREA = 225 G.P.D. / 0.8 G.P.D./S.F. = 282 S.F. ✓  
BED SIZE: USING WIDTH (A) = 5 FEET,  
LENGTH = 282 S.F. / 5 FT. = 56.2 FT.: USE: 57  
BED = 5 FEET X 57 FEET. ✓

MOUND SIZE (BASED ON 4 % AVERAGE SLOPE), SEE PAGE 2.

LINEAR LOADING RATE = 225 GPD ÷ 57 (L) = 3.94 GPD/L.F. ✓

# Foerster & Associates

SHEET 2 OF 10

PROJECT: \_\_\_\_\_

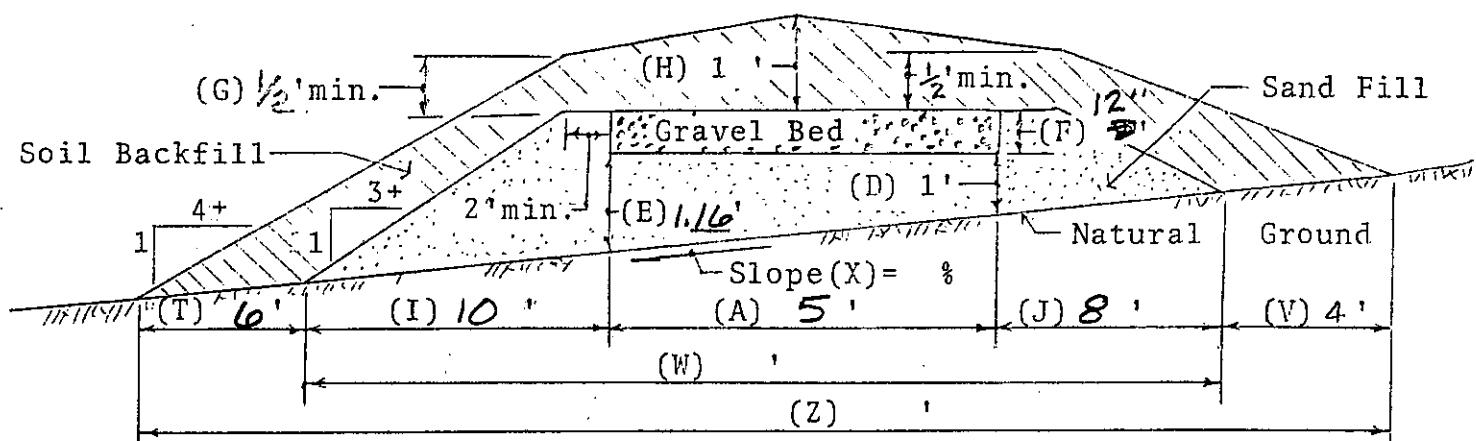
JOB NO.: \_\_\_\_\_

SUBJECT: MOUND SIZE CALCS

DATE: \_\_\_\_\_

CHKD. BY: \_\_\_\_\_

PV: \_\_\_\_\_



$$\text{Gravel Bed Length (B)} = \underline{57'}$$

$$E = D + A \times X = 1' + 4' \times .04 = 1.16'$$

$$T = 0-2\%: 4'; 2-4\%: 6'; 4-6\%: 8'; 6-8\%: 10'; 8-12\%: 12' (\text{SCPH}) = \underline{6}$$

$$I = 2' + (E + F) \times 3 \times \text{downslope (f)} = 2' + (1.16' + 1') 3 \times 1.14 = \underline{9.4} (\text{USE } 10')$$

$$J = 2' + (F + D) \times 3 \times \text{upslope (f)} = 2' + (1' + 1') 3 \times .89 = \underline{7.3} (\text{USE } 8')$$

$$K = (E + F) \times 3 + 2 = (1.16' + 1') 3 + 2' = \underline{8.5} (\text{USE } 9')$$

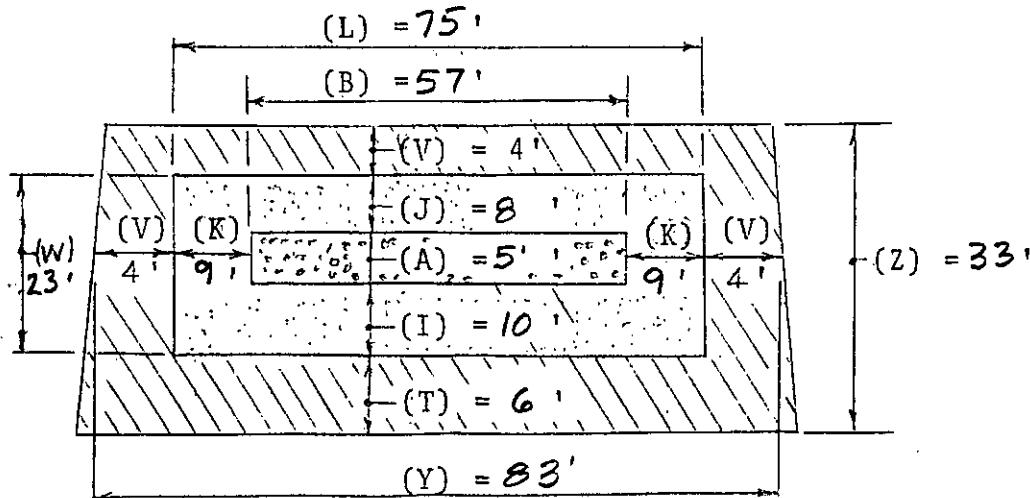
$$L = B + 2K = 57' + 2(8.5') = \underline{75'}$$

$$W = I + A + J = 23' + 5' + 8' = \underline{36'}$$

$$Y = L + 2V = 75' + 2(4') = \underline{83'}$$

$$Z = W + V + T = 36' + 4' + 6' = \underline{46'}$$

SLOPE %	DOWNSLOPE (I) CORRECTION FACTOR		UPSLAPE (J) CORRECTION FACTOR	UPSLOPE (J) CORRECTION FACTOR
	1.00	.94		
0	1.00	.94	.89	.86
2	1.06	.95	.91	.88
4	1.12	.96	.92	.89
6	1.14	.97	.93	.90
8	1.15	.98	.94	.91
10	1.16	.99	.95	.92
12	1.17	1.00	.96	.93



SAND AREA UNDER BED AND DOWN SLOPE OF BED

$$= \underline{57} \text{ FT.} \times (\underline{5} \text{ FT.} + \underline{10} \text{ FT.})$$

$$= \underline{855} \text{ SQ. FT. AVAILABLE SAND BASE AREA}$$

MINIMUM SAND BASE AREA NEEDED

$$= \underline{225} \text{ G.P.D.} / \underline{487} \text{ G.P.D./ S.F.}$$

$$= \underline{462} \text{ S.F. NEEDED}$$

MORE THAN ENOUGH BASAL AREA WILL BE PROVIDED.

DISTRIBUTION SYSTEM:

TOTAL 3/16 INCH HOLES IN SYSTEM DISTRIBUTION:

USE 1 LINE~~s~~ (2 LATERALS) AT 1 FOOT SPACING AS  
SHOWN ON PLAN (NUMBER OF 3/16" HOLES/LINE WITH  
36 INCH SPACING PER HOLE = 19 HOLES PER LINE.)

$$\text{TOTAL HOLES} = \underline{1} \text{ LINES} \times \underline{19} \text{ HOLES/LINE} = \underline{19}$$

$$\text{TOTAL FLOW} = \underline{19} \text{ HOLES} \times 0.00\frac{94}{80} \text{ G.P.M./HOLE*} = \underline{18} \text{ G.P.M.}$$

\* FLOW BASED ON ATTACHED ~~F10: 13 & 25 FEET HEAD.~~  
~~TBL 2~~

MANAFOLD SIZE:

WITH 1 1/2 INCH P.V.C. PIPE AND A TOTAL FLOW OF  
18 G.P.M., FRICTION LOSS = 2.1 FT./ 100 FT OF  
PIPE (SEE TABLE 9): USE 1 1/2 INCH P.V.C. SCH. 40 PIPE  
LATERAL SIZE: USE 1 1/4 INCH P.V.C. SCH. 40 PIPE FOR LATERALS

PUMP / SUMP SIZING:

PUMP: USE HYDROMATIC SHEF 40, OR = (SEE SHEET 4)

DOSE: USE 75\* GALLONS (TABLE 5) OR TEN X PIPE VOLUME.

PIPE VOLUME = 25 FEET X .092 GAL/FT. OF 1 1/2 INCH  
PIPE + 56 FEET X .064 GAL/FT. OF 1 1/4 INCH PIPE  
= 5.89 GALLONS. TEN X PIPE VOL. = 59 GALLONS

\* USE 100 GALLONS PER DOSE.

PROJECT:

SUBJECT: SEPTIC SYSTEM PUMP CALCULATIONS

JOB NO.: \_\_\_\_\_

DATE: \_\_\_\_\_

BY: J M P

CIVIL ENGR

## I. TOTAL DESIGN HEAD (T.D.H.):

A. STATIC HEAD ( $H_S$ ):

$$\begin{aligned} 1. \text{ PUMP SET AT ELEVATION} &= 100 \\ 2. \text{ HIGHEST LEACH LINE INVERT} &= 100 \\ 3. H_S = 106 - 100 &= 6' \end{aligned}$$

B. FRICTION HEAD LOSS ( $H_F$ ) AT 19 G.P.M.:

$$\begin{aligned} 1. \frac{1}{2}'' \text{ P.V.C. FRICTION LOSS } (F_L) &= 2.1 / 100' \\ 2. \text{ LENGTH PIPE} &= 20' + \text{EQUIVALENT LENGTH FOR BENDS, ETC.} \\ &= 20' + 32' (4.90^\circ \text{ BENDS}) + 1' (\text{UNION}) + 1' (\text{GATE V.}) + 9 (\text{TEE}) \\ &+ 13' (\text{CHECK V.}) = \end{aligned}$$

$$3. H_F = (2.1 / 100') \times 76' = 1.6'$$

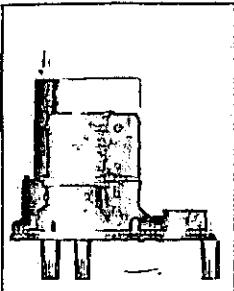
$$C. T.D.H. = H_S + H_F = 6' + 1.6' + 5' \text{ SAT.} = 12.6 \quad \text{USE 15 FEET}$$

II. USE PUMP: \_\_\_\_\_ (SEE ATTACHED CURVE), OR EQUAL.

Table 1

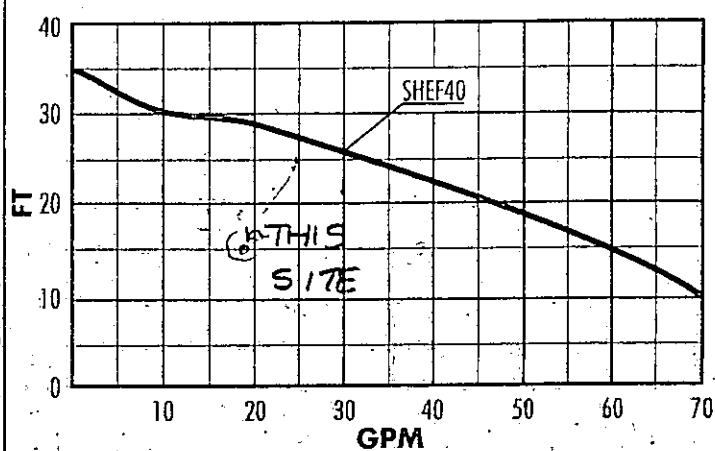
Friction losses Through Plastic Fittings  
In terms of Equivalent Lengths of Plastic Pipe

Type of Fitting	Nominal Size Fitting and Pipe			
	1-1/4"	1-1/2"	2"	2-1/2"
90° STD. Elbow	7.0	8.0	9.0	10.0
45° Elbow	3.0	3.0	4.0	4.0
STD. Tee (Diversion)	7.0	9.0	11.0	14.0
Check Valve	11.0	13.0	17.0	21.0
Coupling or Quick Disconnect	1.0	1.0	2.0	3.0
Gate Valve	0.9	1.1	1.4	1.7



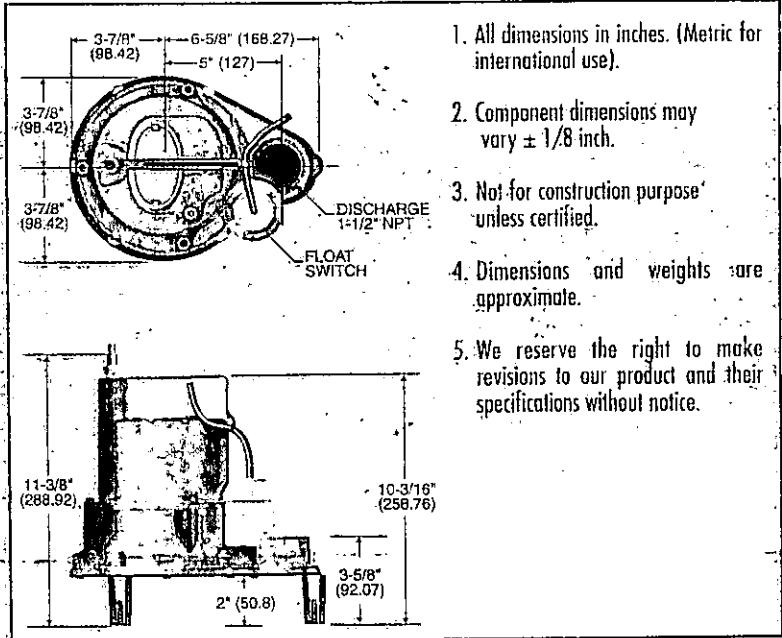
## Engineering Details - SHEF40

### Performance Data



Total Head (feet)	10	14	17	21	25	28	30	35
(m)	3.0	4.3	5.2	6.1	7.6	8.5	8.8	10.7
GPM (US GPM)	70	60	50	40	30	20	10	0
(liters/sec)	4.4	3.8	3.2	2.5	1.9	1.3	.63	0

### Dimensional Data



1. All dimensions in inches. (Metric for international use).

2. Component dimensions may vary  $\pm 1/8$  inch.

3. Not for construction purpose unless certified.

4. Dimensions and weights are approximate.

5. We reserve the right to make revisions to our product and their specifications without notice.

### Materials of Construction

Handle	Stainless Steel
Lubricating Oil	Dielectric Oil
Motor Housing	Cast Iron
Pump Casing	Cast Iron
Shaft	Steel
Mechanical Shaft Seal	Seal Faces: Carbon/Ceramic Seal Body: Anodized Steel Spring: Stainless Steel Bellows: Buna-N
Impeller	Engineered Thermoplastic
Upper Bearing	Bronze Sleeve Bearing
Lower Bearing	Single Row Ball Bearing
Bottom Plate	Polyester Coated Steel
Fasteners	Stainless Steel
Legs	Engineered Thermoplastic



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- Your Authorized Local Distributor -



TABLE 5. Recommended Dosing Quantity for Various Sized Homes

Home Size No. Bedrooms	Dosing Quantity* Gal/Dose
1	50
2	75
3	115
4	150
5	200

TABLE 6. Void Volume for Various Diameter Pipes

Diameter inch	Volume gal/ft length
1	.041
1 1/4	.064
1 1/2	.092
2	.164
3	.368
4	.655
6	1.47

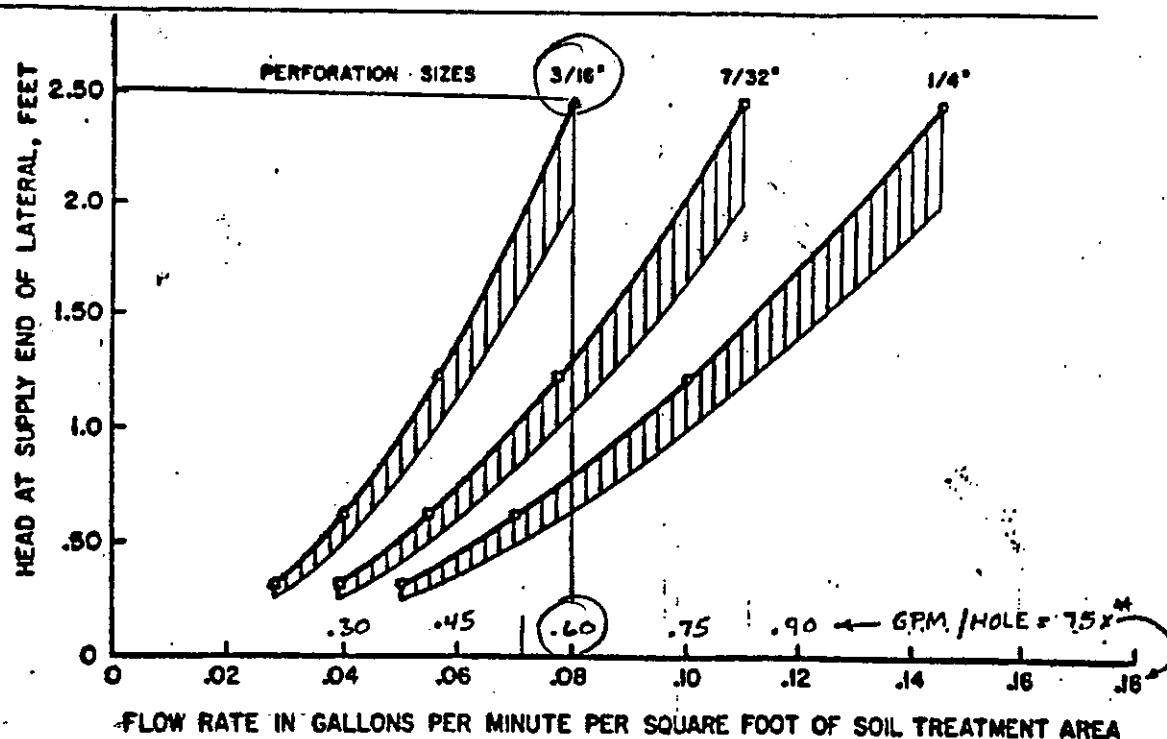


Fig. 13 Flow rate vs. head for perforation spacing of 30 in. and lateral spacing of 3' ft. The lower curve for each perforation represents the head at the distal end of lateral (Machmeier 1975).

\* CHART IS BASED ON 30"(2.5') X 3' AREA / PERFORATION (7.55F)  
† 7.5 SF / PERFORATION X FLOW RATE = FLOW / PERFORATION

Pearson & Associates

SHEET 1 OF 10

PROJECT: \_\_\_\_\_

SUBJECT: SYSTEM DISTRIBUTION  
LATERAL HOLE DISCHARGE CHKD. BY: \_\_\_\_\_

JOB NO.: \_\_\_\_\_

DATE: \_\_\_\_\_

BY: JMP

Table 2.\* Flow rate as a function of pressure head and hole diameter in drilled PVC pipe

Pressure Head ft	psf	Hole diameter (in.)				
		3/32	1/8	5/32	3/16	7/32
- Flow rate (gpm) -						
1	0.43	0.10	0.18	0.29	0.42	0.56
2	0.87	0.15	0.26	0.41	0.59	0.80
3	1.30	0.18	0.32	0.50	0.72	0.98
4	1.73	0.21	0.37	0.58	0.83	1.13
5	2.16	0.23	0.41	0.64	0.94	1.26

\* FROM "DESIGN & INSTALLATION OF LOW PRESSURE PIPE WASTE  
TREATMENT SYSTEMS" BY CRAIG COGGER, ET AL. 1982 N.C.S.U.

TABLE 9. Friction Loss in Schedule 40 Plastic Pipe ( $C = 150$ )

	Pipe Diameter (in)								
Flow	1	1 1/4	1 1/2	2	3	4	6	8	10
gpm	ft/100 ft								
1	0.07								
2	0.28	0.07							
3	0.60	0.16	0.07						
4	1.01	0.25	0.12						
5	1.52	0.39	0.18						
6	2.14	0.55	0.25	0.07					
7	2.89	0.76	0.36	0.10					
8	3.63	0.97	0.46	0.14					
EACH LAT.	4.57	1.21	0.58	0.17					
PUMP TO LAT. = 18	5.50	1.46	0.70	0.21					
11	1.77	0.84	0.25						
12	2.09	1.01	0.30						
13	2.42	1.17	0.35						
14	2.74	1.33	0.39						
15	3.06	1.45	0.44	0.07					
16	3.49	1.65	0.50	0.08					
17	3.93	1.86	0.56	0.09					
18	4.37	2.07	0.62	0.10					
19	4.81	2.28	0.68	0.11					
20	5.23	2.46	0.74	0.12					
25	3.75	1.10	0.16						
30	5.22	1.54	0.23						
35		2.05	0.30	0.07					
40		2.62	0.39	0.09					
45		3.27	0.48	0.12					
50		3.98	0.58	0.16					
60			0.81	0.21					
70			1.08	0.28					
80			1.38	0.37					
90			1.73	0.46					
100			2.09	0.55	0.07				
125				0.85	0.12				
150				1.17	0.16				
175				1.56	0.21				
200				0.28	0.07				
250				0.41	0.11				
300	Velocities in this area become too great for the various flow rates and pipe diameter.					0.58	0.16		
350						0.78	0.20	0.07	
400						0.99	0.26	0.09	
450						1.22	-0.32	0.11	
500						0.38	0.14		
600						0.54	0.18		
700						0.72	0.24		
800							0.32		
900							0.38		
1000							0.46		

## MOULD SIZING CHART



### Upslope width ( $J$ )

no variation with bed width.

### Downdhill side sand depth (E)

\* THIS IS SOME COPIES OVER SIZE ONE SCOPE.

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COUNTY OF SONOMA  
PUBLIC HEALTH DEPARTMENT

TABLE I

## PERK RATE CONVERTED TO GAL/SQ FT/DAY

1-3 MPI =	.1.2 GAL/SQ FT/DAY	47 MPI =	.437 GAL/SQ FT/DAY
4 MPI =	1.143 GAL/SQ FT/DAY	48 MPI =	.43 GAL/SQ FT/DAY
5 MPI =	1.086 GAL/SQ FT/DAY	49 MPI =	.423 GAL/SQ FT/DAY
6 MPI =	1.029 GAL/SQ FT/DAY	50 MPI =	.417 GAL/SQ FT/DAY
7 MPI =	.971 GAL/SQ FT/DAY	51 MPI =	.41 GAL/SQ FT/DAY
8 MPI =	.914 GAL/SQ FT/DAY	52 MPI =	.403 GAL/SQ FT/DAY
9 MPI =	.857 GAL/SQ FT/DAY	53 MPI =	.397 GAL/SQ FT/DAY
10 MPI =	.8 GAL/SQ FT/DAY	54 MPI =	.39 GAL/SQ FT/DAY
11 MPI =	.786 GAL/SQ FT/DAY	55 MPI =	.383 GAL/SQ FT/DAY
12 MPI =	.771 GAL/SQ FT/DAY	56 MPI =	.377 GAL/SQ FT/DAY
13 MPI =	.757 GAL/SQ FT/DAY	57 MPI =	.37 GAL/SQ FT/DAY
14 MPI =	.743 GAL/SQ FT/DAY	58 MPI =	.363 GAL/SQ FT/DAY
15 MPI =	.729 GAL/SQ FT/DAY	59 MPI =	.357 GAL/SQ FT/DAY
16 MPI =	.714 GAL/SQ FT/DAY	60 MPI =	.35 GAL/SQ FT/DAY
17 MPI =	.7 GAL/SQ FT/DAY	61 MPI =	.345 GAL/SQ FT/DAY
18 MPI =	.686 GAL/SQ FT/DAY	62 MPI =	.34 GAL/SQ FT/DAY
19 MPI =	.671 GAL/SQ FT/DAY	63 MPI =	.335 GAL/SQ FT/DAY
20 MPI =	.657 GAL/SQ FT/DAY	64 MPI =	.33 GAL/SQ FT/DAY
21 MPI =	.643 GAL/SQ FT/DAY	65 MPI =	.325 GAL/SQ FT/DAY
22 MPI =	.629 GAL/SQ FT/DAY	66 MPI =	.32 GAL/SQ FT/DAY
23 MPI =	.614 GAL/SQ FT/DAY	67 MPI =	.315 GAL/SQ FT/DAY
24 MPI =	.6 GAL/SQ FT/DAY	68 MPI =	.31 GAL/SQ FT/DAY
25 MPI =	.593 GAL/SQ FT/DAY	69 MPI =	.305 GAL/SQ FT/DAY
26 MPI =	.587 GAL/SQ FT/DAY	70 MPI =	.3 GAL/SQ FT/DAY
27 MPI =	.58 GAL/SQ FT/DAY	71 MPI =	.295 GAL/SQ FT/DAY
28 MPI =	.573 GAL/SQ FT/DAY	72 MPI =	.29 GAL/SQ FT/DAY
29 MPI =	.567 GAL/SQ FT/DAY	73 MPI =	.285 GAL/SQ FT/DAY
30 MPI =	.56 GAL/SQ FT/DAY	74 MPI =	.28 GAL/SQ FT/DAY
31 MPI =	.553 GAL/SQ FT/DAY	75 MPI =	.275 GAL/SQ FT/DAY
32 MPI =	.545 GAL/SQ FT/DAY	76 MPI =	.27 GAL/SQ FT/DAY
33 MPI =	.538 GAL/SQ FT/DAY	77 MPI =	.265 GAL/SQ FT/DAY
34 MPI =	.531 GAL/SQ FT/DAY	78 MPI =	.26 GAL/SQ FT/DAY
35 MPI =	.523 GAL/SQ FT/DAY	79 MPI =	.255 GAL/SQ FT/DAY
36 MPI =	.516 GAL/SQ FT/DAY	80 MPI =	.25 GAL/SQ FT/DAY
37 MPI =	.509 GAL/SQ FT/DAY	81 MPI =	.245 GAL/SQ FT/DAY
38 MPI =	.501 GAL/SQ FT/DAY	82 MPI =	.24 GAL/SQ FT/DAY
39 MPI =	.494 GAL/SQ FT/DAY	83 MPI =	.235 GAL/SQ FT/DAY
40 MPI =	.487 GAL/SQ FT/DAY	84 MPI =	.23 GAL/SQ FT/DAY
41 MPI =	.479 GAL/SQ FT/DAY	85 MPI =	.225 GAL/SQ FT/DAY
42 MPI =	.472 GAL/SQ FT/DAY	86 MPI =	.22 GAL/SQ FT/DAY
43 MPI =	.465 GAL/SQ FT/DAY	87 MPI =	.215 GAL/SQ FT/DAY
44 MPI =	.457 GAL/SQ FT/DAY	88 MPI =	.21 GAL/SQ FT/DAY
45 MPI =	.45 GAL/SQ FT/DAY	89 MPI =	.205 GAL/SQ FT/DAY
46 MPI =	.443 GAL/SQ FT/DAY	90+ MPI =	.2 GAL/SQ FT/DAY

# **Non-Standard/Innovative Sewage Disposal System Operational Permit Application**

WLS-009

## **Purpose:**

This application is used to apply for a Nonstandard/Innovative Sewage Disposal System Operation Permit and is submitted to the Sonoma County Permit and Resource Management Department (PRMD) Well and Septic Division. If an operational permit has been issued and the property has been sold, the permit is transferable upon change of ownership.

The applicant must complete, sign and date below, retain pink copy, and submit top copies with payment to the Sonoma County Permit and Resource Management Department. See the current Well and Septic Division fee schedule for filing fees.

Original Application

Change of Ownership:

Date of Change \_\_\_\_\_

Applicant  Owner  Architect  Engineer

MANZANA PRODUCTS CO. INC.

Name

9141 GREEN VALLEY RD.

Mailing Address

SEBASTO POL, CA 95472

City/Town

State/Zip

707 823 5313

Phone

Fax

## **Project Site Information**

9141 GREEN VALLEY RD

Address(es)

City/Town

State/Zip

130-090-020

Assessor's Parcel Number(s)

SEP

File Number

I (we) understand that the permit is valid from the date of issuance to the end of the current fiscal year (June 30) thereafter, the permit is valid for one year and must be renewed annually. Permits are transferable upon change of ownership. I (we) agree to comply with all applicable State and County codes, and the rules and regulations set forth by PRMD. Including, but not limited to, performance of self-monitoring inspections on the sewage disposal system.

MANZANA PRODUCTS CO. INC.

Owner's Name

mark taylorself 4-4-14

Signature

Date

SAME

Mailing Address

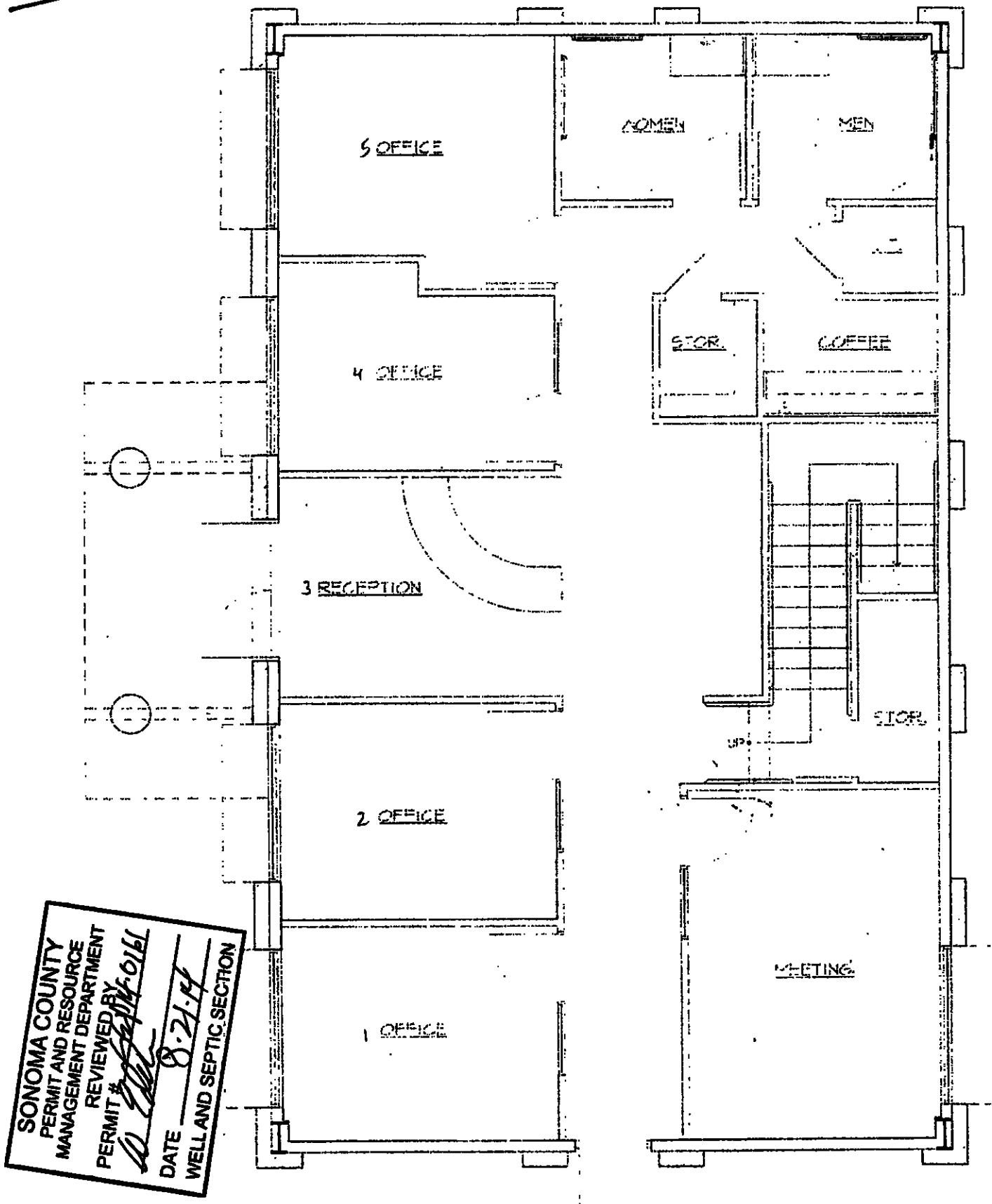
Owner's Name

Signature

Date

**Filing Fee:** See the current PRMD Fee Schedule

SIGN THIS  
(3 ITEM)

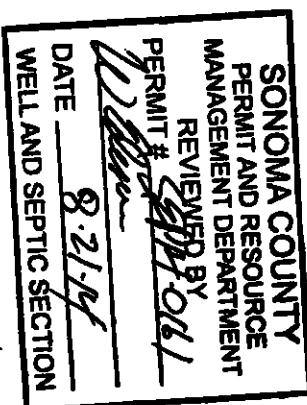
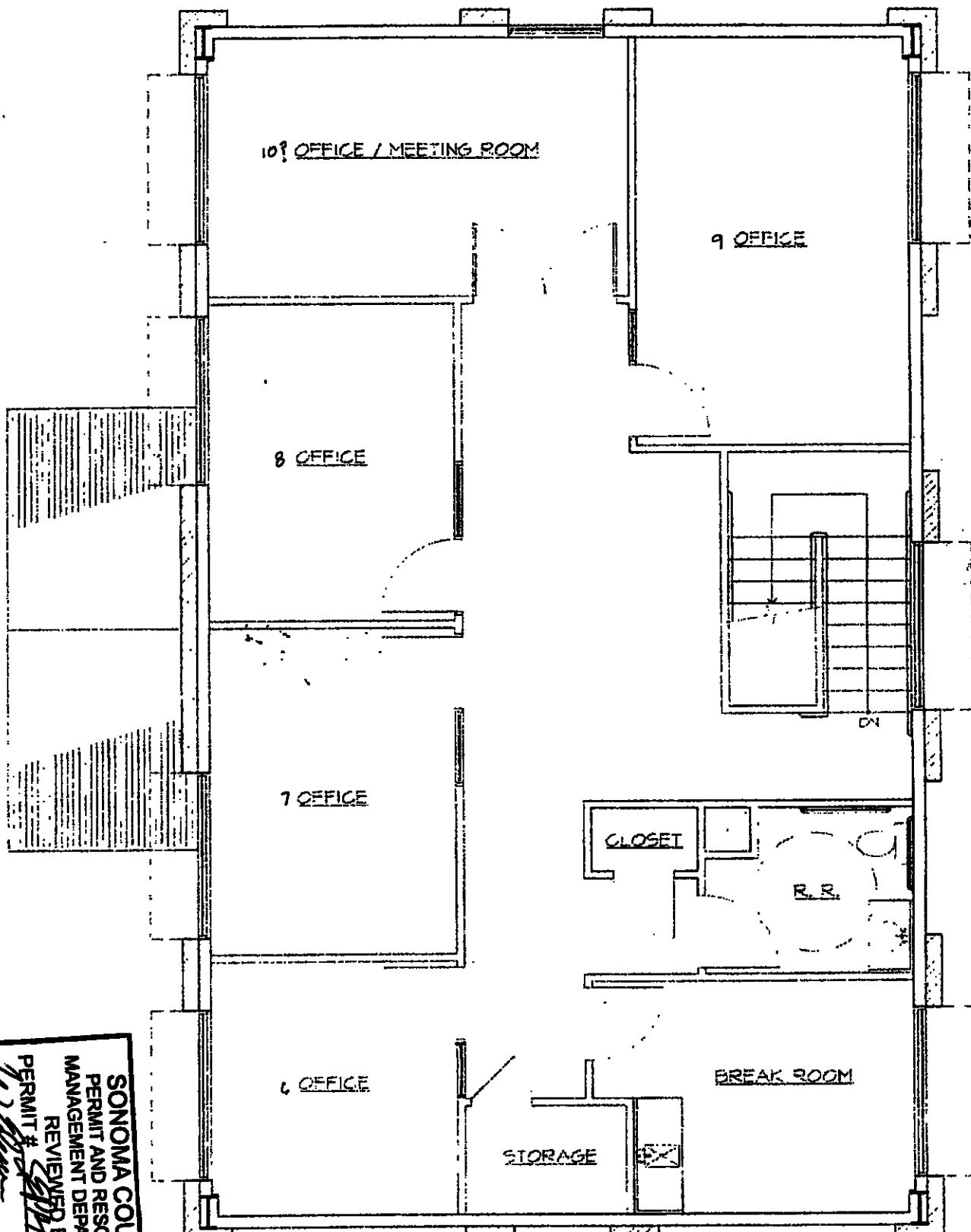


## 1st FLOOR PLAN

NO SCALE



REC. 3/13/14  
PLANS



**2nd FLOOR PLAN**

NO SCALE

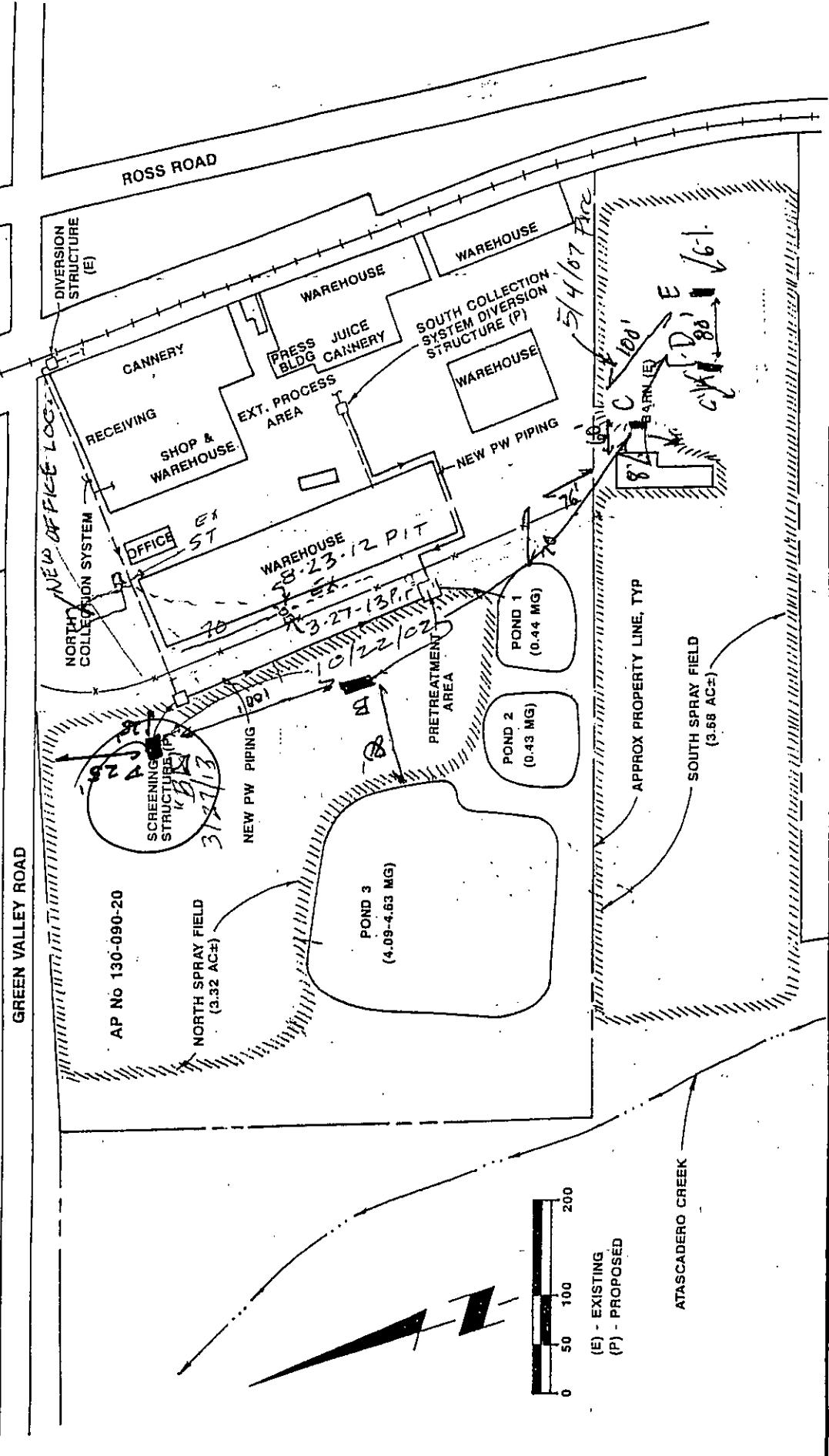




MANZANA  
PRODUCTS CO., INC.  
GRATON, SONOMA CO., CA  
SCHEMATIC SITE PLAN

PROJECT NO. 94112.8  
DATE 2-23-95  
BY MW CHK BP  
SHEET NO. 01

AP No 130-090-02



1400 North Dutton Avenue #22

Santa Rosa, California 95401

(707) 521-0775

Contact