



# BLD12 · 0107

**Permit Number** 

14701

Street Number

BODEGA HWY

Street Name

TWI

**Community Code** 

026-120-006

APN

COUNTY OF SONOMA - PERMIT AND RESOURCE MANAGEMENT DEPARTMENT 2550 Ventura Avenue, Santa Rosa, CA 95403 (707) 565-1900 FAX (707) 565-1103 Please Print Date Your Name: Applied: 2 Cres INFORMATION WITHIN HEAVY LINE TO BE COMPLETED BY APPLICANT SITE LOCATION INFORMATION - PRINT CLEARLY JOB ADDRESS: BODEDA BODEGA HWY City: A型24-120-006 Project 1079741850 Project 7079221289 Cross-Street: Ernall address: Som & S/cciecon Hauth 68,000 501 (N) water Storage Tank Garage 35,a<u>w</u> Decks OWNER NAME AND ADDRESS APPLICANT NAME AND ADDRESS Kistice Vinequal Grecie Constructor, Inc Mailing Address: Box 1797 Mailing Address: 4707 Vine Hill ZIP:95476 Bonpon 3 - SCO3 Fex: ( )

CONTRACTOR INFORMATION **啊り7)922** OTHER PERSONS (ARCHITECT, ENGINEER, ETC.) Company Name: Grecic Controton, Inc Address 21F95471 State: City: State: ZIP: Day P4707 974- (890 F®2707 ዓኒշ -1585 Day Ph: ( WORKER'S COMPENSATION DECLARATION immunder penalty of perjury one of the following declarations: and will maintain a certificate of consent to self-insure for worker's compensations: Exp. Date: CONSTRUCTION LENDING DECLARATION I hereby affirm under penalty of perjury that there is a construction lending agency for the pethe work for which this permit is issued. (Sec. 3097, Civ. C.). provided for by Section 3700 of the Labor Code, for the performance of the work for which this provided for by Section 3700 of the Labor 5500, is also permit is issued.

There and will maintain worker's compensation insurance, as required by Section 3700 of the Labor 500e, for the performance of the work for which this permit is issued. My worker's compensation insurance carrier and policy number are: Lenders Name Whe Known arrier collection need not be completed if the permit is for one hundred dollars (\$100) or less).

This section need not be completed if the permit is for one hundred dollars (\$100) or less).

I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the worker's compensation laws of California, and agree that if I should become subject to the worker's compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with make provisions. VOH Zoning CLOWA BO-100 TFile No. PUT0-0050 Acres
Existing Use/Structures
Proposed Use/Structures
Zoning Min. Yard Requirements: Front 30 Left 10 Right 10
NOTE: Fire Safe Standards require all parcels greater than 1 Acre to have a result. 250.60 Proposed Use/Structures 1

Zoning Min. Yard Requirements: Front 30

NOTE: Fire Safe Standards require all parcels greated unless mitigated. Mitigation Required Back Exp. Date:

WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL, AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000), IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST, AND ATTORNEY'S FEES. Exp. Date: Address subject to change Approval for Permit Issua OWNER-BUILDER DECLARATION

I hereby affirm under penalty of perjury that I am exempt from the Contractor's License Law for the following reason (Sec. 7031.5, Business and Professions Code: Any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also PERMIT NUMBER: Conditions: permit to Constant, alter, improve, denoting, or requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractor's License Law (Chapter 9 (commencing with Section 7000) of Division 3 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars (\$500).): ver Connection: ☐ Available ☐ Fees Paid Approved by: Road Encroachment: Fees Paid only Septic System Permit/Clearance# Weter 19/2012 100 Year El TGRD 10-0037 By my signature below I acknowledge that, except for my personal residence in which I must have resided for at least one year prior to completion of the improvements covered by this permit, I cannot legally sell a structure that I have built as an owner-builder if it has not been constructed in its entirety by licensed contractors. I understand that a copy of the applicable law, Section 7044 of the Business and Professions Code, is available upon request when the application is submitted or at the following website: http://www.leginfo.ca.gov/calaw.html. Drainage Review 9-12

Date Signature of Property Owner or Authorized Agent

LICENSED CONTRACTOR'S DECLARATION

I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with Section 7000) of Division 3 of the Business and Professions Code, and my license is in full force and effect.

Lic. Class B Lic. No. 806540

Exp. Data 30 13 Contractor G C1

ASBESTOS DECLARATION

Written asbestos notification pursuant to Part 61 of Title 40 of the Code of Federal Regulations is required when asbestos exists in buildings, or portions thereof, undergoing demolition. I hereby declare that demolition authorized by this permit is from construction that (□ does) (□ does not) contain asbestos, or that □ no demolition is authorized by this permit.

I certify that I have read this application and affirm under penalty of perjury that the above information is correct. I agree to comply with all local Ordinances and State laws relating to building construction. I hereby authorize representatives of the County of Sonoma to enter upon the above-mentioned property for inspection purposes. If, after making the Certificate of Exemption for the Worker's Compensation provision of the Labor Code I should become subject to such provisions, I will forthwith comply. In the event do not comply with the Workman's Compensation law, this permit shall be deemed evoked.

PERMITTEE SIGNATURE 797

☐ Owner

Contractor

Sonnie

<u>92476</u>

Other Licensed Professional

cruiter s:\handoutskss

☐ Post FIRM

☐ Alquist Priolo Report Available

Certificate of Occupancy

☐ Ge

No of Units

131)	SPECIAL INSPECTION REQ	UIRED	☐ YES	☐ NO IF YES, SEE ADDITIONAL SHEET
	INSPECTION RECORD	DATE	NAME	REMARKS
101)	ROUGH GRADING			68,000 gal Steel Water
103)	FOUNDATION			tank -
	FORMS/SETBACK		<del></del>	
	FOOTING ' WALLS			
106)	UFER GROUND #			
104)	CAISSONS/PIERS			
105)	SLAB			
107)	UNDERGROUND UTILITIES			
110)	MASONRY			
109)	RETAINING WALLS			
113)	FIREPLACE			
	FOOTING			
	HEARTH/PROTECTION			
	THROAT			
114)	CHIMNEY			
120)	UNDERFLOOR/UNDERSLAB			
115)	HYDRONICS			
116)	U/F ELECTRICAL		<u></u>	
117)	U/F MECHANICAL			
118)	U/F PLUMBING			
119)	U/F FRAMING			
139)	U/F INSULATION			
126)	SHEAR WALLS NTERIOR	L. <u></u>	l <u> </u>	
127)	DIAPHRAGMS	]		
	ROOF DI FLOOR		<u> </u>	
134)	SIDING/SHEATHING		, <del></del>	
125)	HOLD DOWNS			
132)	CLOSE-IN	<u> </u>		
122)	ROUGH ELECTRICAL			
123)	ROUGH MECHANICAL			
124)	ROUGH PLUMBING		-	
128)	ROUGH FRAME			
160)	SMOKE DETECTORS			
139)	INSULATION			
142)	WALLBOARD			
143)	FIREWALLS			
135)	STUCCO/PLASTER			
	ATH SCRATCH	7		
137)	ROOFING			
130)	TUB/SHOWER PAN			
162)	FIRE DAMPERS/DOORS			
164)	SUSPENDED CEILING ROUGH ELEC.	<u></u>		
165)	EXITING - RAMPS/STAIRS	Un.		
163)	HANDRAILS/GUARDRAILS			
100)	CORRIDORS/DOORS			
166)	ACCESSIBILITY COMPLIANCE			650) SUSMP INSPECTION
144)	WATER TANKS			651) NPDES EROSION COMPLIANCE
	SLAB		·	652) NPDES SEDIMENT COMPLIANCE
170)	TEMPORARY OCCUPANCY	· ·		653) NPDES DOCS/SWPPP
171)	TEMPORARY ELECTRICAL			FIRE INSPECTION REQUIRED DATE NAME
172)	TEMPORARY GAS			☐ Yes ☐ No
174)	ELECTRIC METER AUTHORIZATION			759) KNOX BOX
152)	PANEL BOARDS/SERVICE			760) PROPANE TANK HOLD DOWNS
189)	SEPTIC ELECTRIC FINAL			770) SPRINKLER FINAL
175)	GAS METER AUTHORIZATION			771) ABOVEGROUND HYDROSTATIC
153)	GAS PRESSURE TEST			772) UNDERGROUND HYDROSTATIC
<u> </u>	HOUSE YARD			773) UNDERGROUND FLUSH
190)	MANUF. HOME FOUNDATION			774) THRUST BLOCKS
191)	MANUF. HOME INSTALLATION			775) PIPE WELD
	CONTINUITY etalpe/exipte			775) PIPE WELD  776) HYDRANTS/APPLIANCES  777) PUMP ACCEPTANCE
	STAIRS/SKIRTS RIDGE BOLTING			
102\	MANUF. HOME COND. FINAL			778) WATER SUPPLY/TANK 779) ALARM SYSTEM
193)	SWIMMING POOLS			780) HOOD & DUCT SYSTEM
104\	PRE-GUNITE			781) ABOVEGROUND TANK/DISPENSER
194) 195)	PRE-DECK			198) FIRE FINAL
195)	PRE-PLASTER/FENCE			CLEARANCES:
196)	VINYL/FIBERGLASS POOL EXCAVATION			FIRE
102)	GRADING FINAL			HEALTH DEPARTMENT
176)	ELECTRICAL FINAL			ZONING
177)	MECHANICAL FINAL			ZONING PARTITION
178)	PLUMBING FINAL		A /	
199)	FINAL	6/18/14	Ar/	PLAN RETENTION REQUIRED?
	OCCUPANCY (OK TO OCCUPY)	/ /	V	☐ Yes ☐ No

#### COUNTY OF SONOMA - PERMIT AND RESOURCE MANAGEMENT DEPARTMENT 2550 Ventura Avenue, Santa Rosa, CA 95403 (707) 565-1900 FAX (707) 565-1103 Please Print Date Applied: Your Name INFORMATION WITHIN HEAVY LINE TO BE COMPLETED BY APPLICANT SITE LOCATION INFORMATION - PRINT CLEARLY City: Lot Directions #1 Contract Price: Describe Project: Living Area ا بي いたいほんたい 55. w APPLICANT NAME AND ADDRESS OWN ER NAME AND ADDRESS 1 مصطنعيمانهم Mailing Address: Mailino Address: State City: City: . 471 Fax:( )) ) ) Day Ph: ( 9711191 Fax: ( Day Ph: (~ ₹ OTHER PERSONS (ARCHITECT, ENGIN R, ETC.) CONTRACTOR INFORMATION Name Company Name: Address Address ZIP: City: ZIP: City State: 167 Day Ph: { Fax: ( Day Ph: ( 4 7 151 WORKER'S COMPENSATION DECLARATION Irm under penalty of perjury one of the following declarations: and will maintain a certificate of consent to self-insure for worker's compe Exp. Date: DI have TUCTION LENDING DECLARATION Halty of perjury that there is a construction lending agency for the performance of ermit is issued. (Sec. 3097, Civ. C.). CONS ed for by Section 3700 of the Labor Code, for the perform permit is issued. permit is issued. [I thave and will maintain worker's compensation insurance, as required by Section 3700 of to Code, for the performance of the work for which this permit is issued. My worker's compensation in the performance of the work for which this permit is issued. nce carrier and policy number are: . 1116 Policy No. (This section need not be completed if the permit is for one hundred dollars (\$100) or less). I certify that in the performance of the work for which this permit is issued. I shall not employ any person in any manner so as to become subject to the worker's compensation laws of California, and agree that if I should become subject to the worker's compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions. Policy FOR DEPARTMENT USE Flie No. \_\_\_i🏊 ゴゼミリ Acres Varids Existing Use/Structure Proposed Use/Structures Zoning Min. Yard Requirements: Front 31) Left Right 1 Back NOTE: Fire Safe Standards require all parcels greater than 1 Acre to have a min. 30' unless mitigated. Mitigation Required Address subject to change Exp. Date: Applicant: WARNING: FAILURE TO SECURE WORKER'S CO SHALL SUBJECT AN EMPLOYER TO CRIMINAL PE THOUSAND DOLLARS (\$100,000), IN ADDITION TO PROVIDED FOR IN SECTION 3706 OF THE LABOR CO Approval for Permit Issu COMPENSATION COVERAGE IS TO THE COST OF COMPENSATION ODE, INTEREST, AND ATTORNEY two . Hamane Date OWNER-BUILDER DECLARATION NER-BUILDER DECLARATION I hereby affirm under penalty of perjury that I am exempt from the Contractor's License Law for following reason (Sec. 7031.5, Business and Professions Code: Any city or county which require permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, a requires the applicant for such permit to file a signed statement that he or she is licensed pursuant the provisions of the Contractor's License Law (Chapter 9 (commencing with Section 7000) Division 3 of the Business and Professions Code) or that he or she is exempt therefrom and the best for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects that applicant to a civil penalty of not more than five hundred dollars (\$500).): PERMIT NUMBER: CI I, as owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044 Business and Profession Code: The Contractors License Law does not apply to an owner of property who builds improves thereon, and who does such work himself or herself or through his or he owner of property who builds improves thereon, and who does such work himself or herself or through his or her owner of the provided that such improvements are not intended or offered for sale. If, how for, the provided that such improvements are not intended or offered for sale. If, how for, the provided that such improvements are not intended or offered for sale. If, how for, the provided that such improvements are not intended or offered for sale. ad Encroach building or improvement is sold within one year of completion, the owner-buildburden of proving that he or she did not build or improve for the purpose of sale.). □ I, as owner of the property, am exclusively contracting with incensed contractor project (Sec. 7044, Business and Professions Code: The Contractors License apply to an owner of property who builds or improves thereon, and who contractors with a contractor(s) licensed pursuant to the Contractors License Law.). □ I am exempt under Sec.\_\_\_\_\_\_\_, B & P.C. for this nave the ptic System Permit/Clear: construct the ☐ No 100 Year Flood E ن، ت y my signature below I acknowledge that, except for my person have resided for at least one year prior to completion of the permit, I cannot legally sell a structure that I have built as an constructed in its entirety by licensed contractors. I understallaw, Section 7044 of the Business and Professions Coop., is residence in which I must vements covered by this er-builder if it has not bee and that a copy of the applicab ed by:\_ , is available upon request when this ication is submitted or at the following website: htt v.leginfo.ca.gov/calaw.html. Signature of Property Owner or A LICENSED CONTRACTOR'S DECLARATION affirm under penalty of period that London I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with Section 7000) of Division 3 of the Business and Professions Code, and my license is in full force and effect. INSPECTION AREA Work Authorized Lic. No.\_ Exp. Dát Contractor \_\_: ASBESTOS DECLARATION ion pursuant to Part 61 of Title 40 of the Code of Federal Regulation: Post Fil No Plans Subject to F Written asbestos notification pursuant to Part 61 of Title 40 of the Code of Federal Regulating required when asbestos exists in buildings, or portions thereof, undergoing demolition. In declare that demolition authorized by this permit is from construction that (CI does) (CI does) Pre FIRM I here contain asbestos, or that \(\Q\) no demolition is authorized by this permit. I certify that I have read this application and affirm under penalty of perjury that the above inform is correct. I agree to comply with all local Ordinances and State laws relating to building construing the hereby authorize representatives of the County of Sonoma to enter upon the above-ment property for inspection purposes. If, after making the Certificate of Exemption for the labor Code I should become subject to such provision and for comply. In the event I do not comply with the Workman's Compensation law, this permit should become subject to such provision. Machine Space for Permit Fee d revoked. PERMITTEE SIGNATURE ZIP CITY ADDRES: :□ Contractor □ Other Licensed Profe



February 6, 2012

11627.08

Mr. Richard Schuh Nielsen – Schuh Architects 1346 Legs Lane Sonoma, CA 95476

Dear Mr. Schuh:

Geotechnical Plan Review Kistler Vineyards - Pozzi Ranch Winery 14701 Bodega Highway Sonoma County, California

This letter presents the results of our geotechnical plan review for the planned 68,000 gallon water tank at Kistler Vineryards – Pozzi Ranch Winery. The project site is located at 14701 Bodega Highway, Sonoma County, California. BAI previously performed a Geotechnical Investigation for the project; the results presented in a report dated August 16, 2011.

We reviewed the geotechnical engineering related items on the following plans and calculations prepared by BlueScope Water: Sheet 1 of 1, and calculations, dated 10/17/11. Based on our review, we conclude that the geotechnical (soil) engineering related portions of these plans are in substantial conformance with the intent of our recommendations. Tank foundation support should conform to the recommendations presented in our August 16, 2011 report.

Please contact us at 707-838-3027 if you have any questions.

Sincerely yours,

J. Erich Rauber, G.E. Geotechnical Engineer - 2887

No. 2887

Geotechnical Engineer - 2894

KAC/JER/kac

Two Copies Submitted

Keith A. Colorado, G.

Sam Turner, Gracie Construction, Inc., sam@gracieconstruction.com
John Schock, BlueScope Water, 601 Noble Street, Madera, CA 93637

Job: MACR11077

# 111 West Ocean Boulevard, Suite 1370 Long Beach, CA 90802 Phone (562) 628-0110

## DESIGN OF UNANCHORED 68,000 GALLON CORRUGATED STEEL WATER TANK

OWNER: KISTLER VINEYARDS POZZI RANCH WINERY LOCATION: 14071 BODEGA HWY, SEBASTOPOL, CA 95465

SUPPLIER: BLUESCOPE WATER SOLUTIONS, INC.
MANUFACTURER: BLESCOPEWATER SOLUTIONS, INC.

## I. MATERIAL SPECIFICATIONS:

STEEL SHEET- 2.5x.5 NOMINAL CORRUGATED SHEETS PER ASTM A-653-00 CS TYPE A W/ MIN Fy=36 KSI. METAL JOISTS - ASTM A653 GR. 50 Fy=55 KSI MIN. GALVANIZED PER ASTM G60. (BY AEP SPAN CO.) PIPE - ASTM A53 PLATES AND SHAPES - ASTM A36 FASTENERS - HUCKBOLT C6L PER ASTM A307 GR C, Fy=36 KSI BOLTS - ASTM A307 GR B, ZINCED PER ASTM A384 or A385

## II. CRITERIA

IN CONFORMANCE WITH THE 2010 CBC REFERENCE TO ASCE 7-05 SECTION 15.7.7 STATIC DESIGN IS PER AWWA D103-97 'FACTORY COATED BOLTED STEEL TANKS FOR WATER STORAGE' AND LATERAL DESIGN IS PER AWWA D100-05 'WELDED STEEL WATER TANKS':

SEISMIC USE GROUP II IMPORTANCE FACTOR Ie := 1.25

DECK LOAD  $R_{LL} := 20$  PSF

WIND VELOCITY V := 100 MPH

SNOW LOAD  $R_{SL} := 0$  PSF

PER "GEOTECHNICAL INVESTIGATION KISTELR VINEYARDS POZZI RANCH WINERY" BY BRUNSING ASSOCIATES, INC, DATED AUGUST 16, 2011, PROJECT NO. 11627.08 THE ALLOWABLE SOIL BEARING, PRESSURE FOR STATIC AND TRANSIENT LOADING ARE:

 $q_{allow} := 2500$  PSF  $q_{seismic} := \frac{4}{3} \cdot q_{allow} = 3333$  PSF

## III: TANK DIMENSIONS:

DIAMETER D:= 27.75 FT COURSE HEIGHT Hc:= 3.75 FT

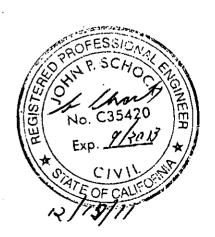
SHELL HEIGHT Ht := 15.0 FT FREEBOARD FB := 4 FT

MAXIMUM WATER HEIGHT H := Ht - FB H = 11 FT

DISTANCE FROM BOTTOM TO VORTEX BREAKER PLATE h = .5 FT

USEABLE CAPACITY NET :=  $\pi \cdot D^2 \cdot .25 \cdot (H - h) \cdot 7.48 = 47501$  GALLONS

DECK PITCH :=  $\frac{2}{12}$  = 0.17 SPECIFIC GRAVITY G := 1.0



# **Bluescope Water**

Job: MACR11077

HEIGHT OF DECK  $H_r := PITCH \cdot \frac{D}{2} = 2.31$  FT DENSITY  $\gamma_w := 62.4$ 

## IV. DESIGN OF 14 GA, BOTTOM COURSE USING (5) 1/4" DIA. FASTENERS/PITCH:

HEIGHT

H1 := Ht - FB = 11 FT

NO. OF FASTENERS

n1 := 5

THICKNESS t1 := .0747 IN

AREA OF FASTENER = a1 := .0491

RADIUS = r := D..5.12 = 166.5 IN DIA. OF FASTENER = d1 := .25

YIELD STRENGTH OF FASTENERS = Fy1 := 36000 PSI

YIELD STRENGTH OF SHEETS = Fv2 := 36000

P\$I

STATIC HEAD: P1 := .433·H1 = 4.76 PSI

MAXIMUM HOOP STRESS:  $S1 := \frac{P1 \cdot r}{r} = 10616$  PSI

LBS/PITCH:  $F1 := S1 \cdot t1 \cdot 2.66 = 2109.49$  LBS

SHEAR CAPACITY OF FASTNERS/PITCH:  $n1.4 \cdot Fy1 \cdot a1 = 3535$  > F1 = 2109

LBS

**NET SECTION CAPACITY OF 1ST ROW:**  $(2.66 - 2.d1) \cdot t1 \cdot .6 \cdot Fy2 = 3485$  > F1 = 2109

BEARING STRESS CAPACITY:

LBS

SHEAR TEAR-OUT EDGE SPACING:

 $\frac{\text{F1} \cdot .5}{2 \cdot 11 \cdot .4 \cdot \text{Fv2}} = 0.49$  IN <u>USE 3/4 INCH MIN.</u>

## V. DESIGN OF 14 GA. SECOND COURSE USING (4) 1/4" DIA. FASTENERS/PITCH:

HEIGHT H2 := Ht - Hc - FB = 7.25 FT

NO. OF FASTENERS

n2 := 4

THICKNESS t2 := .0747 IN

AREA OF FASTENER a2 := .0491

IN

RADIUS r = 166.5 IN

DIA. OF FASTENER d2 := .25

STATIC HEAD: P2 := .433·H2 = 3.14 PSI

MAXIMUM HOOP STRESS:  $S2 := \frac{P2 \cdot r}{r^2} = 6997$  PSI

LBS/PITCH:  $F2 := S2 \cdot t2 \cdot 2.66 = 1390$  LBS

SHEAR CAPACITY IN FASTENERS/PITCH:  $n2 \cdot .4 \cdot Fy1 \cdot a2 = 2828$  > F2 = 1390

LBS

# Bluescope Water

NET SECTION CAPACITY OF 1ST ROW: 
$$(2.66 - 2 \cdot d2) \cdot t2 \cdot .6 \cdot Fy2 = 3485$$
 >  $F2 = 1390$ 

$$F2 = 1390$$

LBS

Job: MACR11077

BEARING STRESS CAPACITY:  $n2 \cdot t2 \cdot d2 \cdot .9 \cdot Fy2 = 2420$  > F2 = 1390 LBS

$$n2 \cdot t2 \cdot d2 \cdot .9 \cdot Fy2 = 2420$$

SHEAR TEAR-OUT EDGE SPACING:

$$\frac{F2 \cdot .5}{2 \cdot t2 \cdot .4 \cdot Fy2} = 0.32 \qquad \text{IN} \qquad \qquad \underline{\text{USE 3/4 INCH MIN.}}$$

## VI. DESIGN OF 16 GA. THIRD COURSE USING (3) 1/4" DIA. FASTENERS/PITCH:

HEIGHT =  $H3 := Ht - 2 \cdot Hc - FB = 3.5$  FT NO. OF FASTENERS = n3 := 3

AREA OF FASTENER = a3 := .0491

RADIUS = r = 166.5 IN

THICKNESS = t3 := .0598 IN

DIA. OF FASTENER = d3 := .25 IN

JOINT EFFICIENCY = E := 1.0

STATIC HEAD: P3 := .433·H3 = 1.52 PSI

MAXIMUM HOOP STRESS:  $S3 := \frac{P3 \cdot r}{F + 3} = 4219.58 \text{ PSI}$ 

LBS/PITCH:  $F3 := S3 \cdot t3 \cdot 2.66 = 671.2$  LBS

SHEAR CAPACITY OF FASTENERS/PITCH:  $n3.4 \cdot Fyl \cdot a3 = 2121$  > F3 = 671

NET SECTION CAPACITY OF 1ST ROW:  $(2.66 - 2 \cdot d3) \cdot t3 \cdot .6 \cdot Fy2 = 2790$  > F3 = 671 LBS

BEARING STRESS CAPACITY:

 $n3 \cdot t3 \cdot d3 \cdot .9 \cdot Fy2 = 1453$  > F3 = 671 LBS

SHEAR TEAR-OUT EDGE SPACING:  $\frac{\text{F3..5}}{2 \cdot \text{t3..4-Fv2}} = 0.19$  IN USE 3/4 INCH MIN.

## VII. DESIGN OF 16 GA. FOURTH COURSE USING (2) 3/16" DIA. FASTENERS/PITCH:

HEIGHT =  $H4 := Ht - 3 \cdot Hc - FB = -0.25$  FT NO. OF FASTENERS = n4 := 2

RADIUS = r = 166.5 IN

DIA. OF FASTENER = d4 := .1875

JOINT EFFICIENCY = E := 1.0

STATIC HEAD: P4 := .433 H4 = -0.11 PSI

MAXIMUM HOOP STRESS:  $S4 := \frac{P4 \cdot r}{F_1 \cdot t^4} = -301.4$  PSI

LBS/PITCH:  $F4 := S4 \cdot t4 \cdot 2.66 = -47.94$  LBS.

## Bluescope Water

SHEAR CAPACITY OF FASTENERS/PITCH:  $n4.4 \cdot Fy \cdot 1.44 = 795$  > F4 = -48

LBS

Job: MACR11077

NET SECTION CAPACITY OF 1ST ROW:  $(2.66 - 2.44) \cdot 44 \cdot .6 \cdot \text{Fy2} = 2951$  > F4 = -48

BEARING STRESS CAPACITY:

 $n4 \cdot t4 \cdot d4 \cdot .9 \cdot Fy2 = 727$  > F4 = -48

LBS

SHEAR TEAR-OUT EDGE SPACING:

$$\frac{\text{F4..5}}{2.14.4 \cdot \text{Fy2}} = -0.01$$
 IN USE 3/4 INCH MIN.

## VIII. DESIGN OF ROOF

THICKNESS OF ROOF tr := .0478

20-GA.

NUMBER OF RAFTERS  $N_r := 22$ 

RADIAL SPACING OF RAFTERS AT EDGE OF 40" DIAMETER SUPPORT DISK:

$$s1 := \frac{\pi \cdot d1 \cdot 12}{Nr \cdot .5} = 11.41$$
 !N

RADIAL SPACING OF RAFTERS AT TANK WALL:

$$s2 := \frac{\pi \cdot D \cdot 12}{Nr} = 47.55$$
 IN

ROOF LOADS:

20 GA DECKING

 $R_{DL1} := 1.41$ 

**C2.6 X 6-14 GA RAFTERS** 

 $R_{DL2} := 2.9$  PSF

TOTAL ROOF DL

 $R_{DL} := 1.41 + 2.90$  PSF

**ROOF LIVE LOAD** 

 $R_{\rm FL} = 20$  PSF

TOTAL ROOF LOAD

 $R_{TL} := R_{DL} + R_{LL}$   $R_{TL} = 24.31$ 

For a C2.6 x 6 -14 GA:

Sx := 1.55 A := .866

I = SPACING OF FASTENERS

1:= 12.0 INCHES

d := 6.0 INCHES

Af := .195 SQ INCHES

Aweb := .450 SQ INCHES

TOTAL LOAD = 
$$W := \frac{D-d1}{2} \cdot R_{TL} \cdot \frac{s2}{12} = 1176$$
 LBS

Job: MACR11077

MAXIMUM MOMENT =

$$M := .1283 \cdot W \cdot \frac{D - d1}{2} = 1843$$

ALLOWABLE BENDING STRESS, lesser of:

$$Fb1 := \frac{12000 \cdot 1 \cdot 1000}{\frac{1 \cdot d}{\Delta f}}$$

$$Fb1 = 32500$$

**PSI** 

$$fb := \frac{M \cdot 12}{Sx}$$
  $fb = 14265$ 

$$Fb2 = 30000$$

SHEAR STRESS:

$$f_V := \frac{.67 \cdot W}{A \text{web}}$$
  $f_V = 1751$   $f_V := .4 \cdot 50000 = 20000$  PSI

$$fv = 1751$$

IX. DESIGN OF CENTER SUPPORT COLUMN, HSS 4" x 4" x 1/4":

$$P := \frac{\pi \cdot D^2 \cdot R_{TL}}{4 \cdot 3} \qquad P = 4901$$

$$r := 1.51$$

INCHES

$$A2 := 3.59$$

SQ INCHES

$$h := 17$$

FEET

SLENDERNESS = 
$$\frac{1 \cdot h \cdot 12}{r} = 135$$
 < 175 OK!

$$fa := \frac{P}{A^2}$$

 $fa := \frac{P}{A2}$  fa = 1365 < 8,190 PSI PER TABLE C-50 AISC MANUAL

X. 26" DIAMETER x (2) 1/4" THICK SUPPORT PLATES ON TOP OF COLUMN

USING ROARK'S FORMULA #2 FOR CIRCULAR FLAT PLATE, EDGES SUPPORTED WITH UNIFORM LOAD OVER CONCENTRIC AREA OF RADIUS Ro:

TOTAL APPLIED LOAD

$$W := P = 4901$$
 LBS

LAMINATED PLATE THICKNESS

$$t := .25 + .25 = 0.5$$
 IN

RADIUS OF WHERE STRESS IS COMPUTED

$$r := 2.82$$
 |N

RADIUS OF RING LOAD

RADIUS OF PLATE

$$a := 13$$
 IN

YOUNGS MODULUS

$$E := 29000000$$

# Bluescope Water

POISSON'S RATIO

$$m := \frac{1}{3} = 3.33$$

FOR r >= Ro

**RADIAL STRESS -**

$$Sr := \frac{3 \cdot W}{\left(2 \cdot \pi \cdot m \cdot t^2\right)} \cdot \left[ (m+1) \cdot \log\left(\frac{a}{r}\right) + (m-1) \cdot \frac{Ro^2}{\left(2 \cdot r^2\right)} - (m-1) \cdot \frac{Ro^2}{\left(2 \cdot a^2\right)} \right] = 11198 \text{ PSI}$$

TANGENTAL STRESS -

$$St := \frac{3 \cdot W}{\left(2 \cdot \pi \cdot m \cdot t^2\right)} \left[ (m-1) + (m+1) \cdot \log\left(\frac{a}{r}\right) + (m-1) \cdot \frac{Ro^2}{\left(2 \cdot r^2\right)} - (m-1) \cdot \frac{Ro^2}{\left(2 \cdot a^2\right)} \right] = 17750 \quad PSI$$

ALLOWABLE STRESS

$$(.75) \cdot 36000 = 27000$$

**PSI** 

**Job: MACR11077** 

## XI. SIZING OF CENTER POLE BEARING PLATE

WIDTH OF SQUARE PLATE REQUIRED =  $\sqrt{\frac{P}{g_{\text{allow}}}} = 1.4$  FT

THICKNESS REQUIRED FOR PLATES:

$$fc := \frac{P}{v^2} = 8.51$$
 PSI

$$d1 := \frac{w-4}{2} = 10$$

 $t := \sqrt{\frac{\text{fc} \cdot \text{d1}^2 \cdot 3}{6.36000}}$  t = 0.34 INCHES <u>USE 24" SQ x 1/2" THICK PLATE</u>

## XII. SEISMIC FORCES

IMPULSIVE RESPONSE MODIFCATION FACTORS FOR GROUND SUPPORTED TANK:

**UNANCHORED TANK** 

$$Ri := 2.5$$

CONVECTIVE RESPONSE MODIFICATION FACTORS FOR GROUND SUPPORTED TANK:

**UNANCHORED TANK** 

$$Rc := 1.5$$

MAPPED ACCELERATION PARAMETERS

FROM ASCE 7-05 AT:

LADITUDE

38.357119N

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LONGITUDE -122.928122W

FOR 5% DAMPED, AT 0.2 SEC PERIOD FOR SITE CLASS B

Ss := 1.500

SHORT-PERIOD SITE COEFFICIENT (TABLE 26)

Fa := 1.0

FOR SITE CLASS D

FOR 5% DAMPED, AT 1.0 SEC PERIOD FOR SITE CLASS B

S1 := 0.693

LONG-PERIOD SITE COEFFICIENT (TABLE 26)

Fv := 1.5

FOR SITE CLASS D

MAXIMUM CONSIDERED EARTHQUAKE SPECTRAL RESPONSE ACCELERATION AS A MULTIPLE OF GRAVITY

 $SMS := Fa \cdot Ss = 1.5$ 

 $SM1 := Fv \cdot S1 = 1.04$ 

SCALING FACTOR FROM MAXIMUM CONSIDERED EARTHQUAKE SPECTRAL RESPONSE ACCELERATION TO DESIGN EARTHQUAKE SPECTRAL RESPONSE ACCELERATION

$$U := \frac{2}{3} = 0.67$$

**DESIGN REPONSE SPECTRA** 

 $SDS := U \cdot SMS = 1$ 

 $SD1 := U \cdot SM1 = 0.693$ 

NATURAL PERIOD OF STRUCTURE (VERY SMALL) BECAUSE GROUND SUPPORTED  $T_i := 0$ THERFORE. THE DESIGN SPECTRAL RESPONSE ACCELERATION FOR IMPULSIVE COMPONENT

Sai := SDS = 1

REGION-DEPENDENT TRANSITION PERIOD FOR LONGER PERIOD GROUND MOTION

TL := 12 SEC F

PER MAPPED ASCE 7 (SHOWN IN FIGURE 19 IN AWWA D100-05)

SLOSHING PERIOD COEFFICIENT

Ks := 
$$\frac{0.578}{\sqrt{\tanh \frac{(3.68 \cdot H)}{D}}} = 0.61$$

NATURAL PERIOD OF SHELL-FLUID SYSTEM CONVECTIVE (SLOSHING) PERIOD

Tc := Ks 
$$\sqrt{D}$$
 = 3.21 SEC

DAMPING SCALING FACTOR K:

K := 1.5

Tc = 3.21 < TL = 12 SEC

THEREFORE, DESIGN SPECTRAL RESPONSE ACCELERATION FOR THE CONVECTIVE COMPONENT

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Sac := 
$$\frac{\text{K} \cdot \text{SD1}}{\text{Tc}} = 0.32$$
 <= SDS = 1

IMPULSIVE DESIGN ACCELERATION OF g

Ai := 
$$\frac{\text{Sai} \cdot \text{Ie}}{1.4 \cdot \text{Ri}} = 0.36 >= \frac{0.36 \cdot \text{S1} \cdot \text{Ie}}{\text{Ri}} = 0.12$$

CONVECTIVE DESIGN ACCELERATION OF g

$$Ac := \frac{Sac \cdot Ie}{1.4 \cdot Rc} = 0.19$$

WEIGHT OF TANK CONTENTS

WT := 
$$62.4 \cdot \text{G} \cdot \text{H} \cdot \left( \frac{\pi \cdot \text{D}^2}{4} \right) = 415139$$
 LBS

WEIGHT OF ROOF SYSTEM

$$Wr := R_{DL} \pi \cdot (D)^2 \cdot .25 = 2607$$
 LBS

WEIGHT OF SHELL

FOURTH COURSE (16 GA) Ws4 :=  $3.75 \cdot \pi \cdot D \cdot 3.52 = 1151$  LBS

THIRD COURSE (16 GA) Ws3 :=  $3.75 \cdot \pi \cdot D \cdot 3.52 = 1151$  LBS

SECOND COURSE (14 GA) Ws2 :=  $3.75 \cdot \pi \cdot D \cdot 4.36 = 1425$  LBS

BOTTOM COURSE (14 GA) Ws1 :=  $3.75 \cdot \pi \cdot D \cdot 4.36 = 1425$  LBS

APPURTENANCES Wsa = 300 LBS

TOTAL SHELL WEIGHT Ws := Wsa + Ws1 + Ws2 + Ws3 + Ws4 = 5452 LBS

WEIGHT OF TANK BOTTOM (14 GA)

$$Wf := \pi \cdot \frac{D^2}{4} \cdot 3.28 = 1984$$
 LBS

EFFECTIVE IMPULSIVE WEIGHT OF TANK CONTENTS

TANK ASPECT RATIO 
$$\frac{D}{H} = 2.523$$

FOR D/H < 1.333

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Wi := 
$$\left(1.0 - 0.218 \cdot \frac{D}{H}\right) \cdot WT = 1$$
 LBS

FOR D/H > 1.333

Wi := 
$$\frac{\tanh\left(0.866 \cdot \frac{D}{H}\right)}{.0.866 \cdot \frac{D}{H}} \cdot WT = 185272 \qquad LBS$$

EFFECTIVE CONVECTIVE WEIGHT OF TANK CONTENTS FOR ANY VALUE OF D/H

We := 
$$0.230 \cdot \frac{D}{H} \cdot \tanh \left( \frac{3.67 \cdot H}{D} \right) \cdot WT = 215976$$
 LBS

CENTROIDAL HEIGHTS OF LATERAL FORCES APPLIED TO EFFECTIVE WEIGHTS

FOR D/H < 1.333 
$$Xi := \left(0.5 - 0.094 \cdot \frac{D}{H}\right) \cdot H = \bullet \qquad FT$$

FOR D/H > 1.333 THE HEIGHT FOR IMPULSIVE FORCE EXISTING CASE

$$Xi := 0.375 \cdot H = 4.13$$
 FT

FOR ALL PRPORTIONS OF D/H THE HEIGHT FOR CONVECTIVE FORCE

$$Xe := \left[ 1.0 - \frac{\left( \cosh\left(\frac{3.67 \cdot H}{D}\right) - 1 \right)}{\left(\frac{3.67 \cdot H}{D} \cdot \sinh\left(\frac{3.67 \cdot H}{D}\right) \right)} \right] \cdot H = 6.3 \qquad FT$$

SHELL CENTROID  $X_S := Ht \cdot .5 = 7.5$ 

$$Xs := Ht \cdot .5 = 7.5$$

## XIII. DESIGN FORCES

DESIGN BASE SHEAR

$$Vf := \sqrt{[Ai \cdot (Ws + Wr + Wf + Wi)]^2 + (Ac \cdot Wc)^2} = 81206$$
 LBS

DESIGN OVERTURNING MOMENT FOR RINGWALL FOUNDATION OR GROUND SUPPORTED **EXISTING CASE** 

$$Ms := \sqrt{\left[Ai \cdot (Ws \cdot Xs + Wr \cdot Ht + Wi \cdot Xi)\right]^2 + \left(Ac \cdot Wc \cdot Xc\right)^2} = 399426$$
 FT - LBS

#### DESIGN OVERTURNING MOMENT FOR PAD FOUNDATION

$$\frac{D}{H} = 2.52$$

FOR D/H < 1.333

$$Ximf := \left(0.50 + 0.06 \cdot \frac{D}{H}\right) \cdot H = \blacksquare \qquad FT$$

FOR D/H > 1.333

Ximf := 0.375 
$$\left[ 1.0 + 1.333 \left( \frac{0.866 \cdot \frac{D}{H}}{\tanh \left( 0.866 \cdot \frac{D}{H} \right)} - 1.0 \right) \right] \cdot H = 10.95 \text{ FT}$$

#### FOR ALL PROPORTIONS OF D/H THE HEIGHT FOR THE CONVECTIVE FORCE

$$Xcmf := \left[ 1.0 - \frac{\left( cosh\left(\frac{3.67 \cdot H}{D}\right) - 1.937\right)}{\left(\frac{3.67 \cdot H}{D} \cdot sinh\left(\frac{3.67 \cdot H}{D}\right)\right)} \right] \cdot H = 9.8 \qquad FT$$

## DESIGN OVERTURNING MOMENT AT TOP OF PAD FOUNDATION

$$Mmf := \sqrt{\left[Ai \cdot (Ws \cdot Xs + Wr \cdot Ht + Wi \cdot Ximf)\right]^2 + \left(Ac \cdot Wc \cdot Xcmf\right)^2} = 856099 \qquad FT - LBS$$

## XIV CHECK OF WIND OVERTURNING

VELOCITY EXPOSURE COEFFICIENT FOR EXPOSURE "C" KZ := 1.09

GUST FACTOR  $G_F := 1.0$ 

FORCE COEFICIENT FOR TANK SHELL  $Cf_{shell} := 0.60$ 

FORCE COEFFICIENT FOR TANK DECK

Cf<sub>deck</sub> := 0.50

WIND IMPORTANCE FACTOR 1 := 1.15

VELOCITY PRESSURE  $q_z := 0.00256 \cdot K_2 \cdot I \cdot V^2 = 32.09$  PSF

WIND PRESSURE ON SHELL  $P_{Wshell} := q_z \cdot G_F \cdot Cf_{shell} = 19.25$  PSF

WIND PRESSURE ON DECK

$$P_{\text{Wdeck}} := q_z \cdot G_F \cdot Cf_{\text{deck}} = 16.04$$

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MOMENT FROM SHELL

$$M_{\text{shell}} := \frac{D \cdot \text{Ht}^2}{2} \cdot P_{\text{Wshell}} = 60108$$

MOMENT FROM DECK

$$M_{deck} := \frac{D \cdot H_r}{2} \cdot \left( H + \frac{H_r}{3} \right) \cdot P_{Wdeck} = 6060$$
 FT-LBS

$$M_{wind} := M_{shell} + M_{deck} = 66168$$

- SEISMIC GOVERNS

**PSF** 

#### WATER DEPTH NEEDED IN TANK TO PREVENT WIND OVERTURNING

FACTOR OF SAFETY

$$d := \frac{FOS \cdot M_{wind}}{\gamma_w \left(\pi \cdot .125 \cdot D^3\right)} = 0.19$$
 < TANK DRAIN INVERT = .33 FT

## XV. COUNTERACTING FORCES

THICKNESS OF TANK BOTTOM

tb := .0747 INCHES

PORTION OF TANK CONTENTS RESISTING OVERTURNING FOR TANK TO BE LESSER OF:

$$wL := 1.28 \cdot H \cdot G \cdot D$$

$$wL = 391$$

LBS/FOOT OF SHELL CIRCUMFERENCE

$$wL := 7.9 \cdot tb \cdot \sqrt{Fy2 \cdot H \cdot G}$$
  $wL = 371$ 

$$wI = 371$$

LBS/FOOT OF SHELL CIRCUMFERENCE

#### WEIGHT OF TANK SHELL AND PORTION OF ROOF

ROOF FACTOR (n=1.0 FOR FREE STANDING, n=0.66 WITH CENTERPOLE)

$$n := 0.66$$

$$wt := \frac{Ws + n \cdot Wr}{\pi \cdot D} = 82$$

wt :=  $\frac{Ws + n \cdot Wr}{\pi \cdot D}$  = 82 LBS/FOOT OF SHELL CIRCUMFERENCE

## XVI. SEISMIC OVERTURNING RATIO

VERTICAL DESIGN ACCELERATION

$$Av := 0.14 \cdot SDS = 0.14$$

$$J := \frac{Ms}{D^2 \cdot [wt \cdot (1 - 0.4 \cdot Av) + wL]} = 1.16$$

WHEN:

## **Bluescope Water**

Job: MACR11077

J < 0.785 THERE IS NO SHELL UPLIFT

0.785 < J < 1.54 THERE IS SHELL UPLIFT, BUT TANK IS STABLE CHECK SHELL COMPRESSION

J > 1.54 ANCHOR TANK or THICKEN BOTTOM ANNULUS

## XVII. HYDRODYNAMIC SEISMIC HOOP TENSILE STRESS AT BOTTOM OF SHELL

#### IMPULSIVE HOOP TENSILE FORCE

$$\frac{D}{H} = 2.52$$

$$Y := H = 11$$

 $.75 \cdot D = 20.81$  FT

FOR D/H < 1.333 and Y<0.75D

Ni := 2.77·Ai·G·D<sup>2</sup> 
$$\left[ \frac{Y}{0.75 \cdot D} - 0.5 \left( \frac{Y}{0.75 \cdot D} \right)^2 \right] = 296$$
 PSi

FOR D/H < 1.333 and Y>0.75D

$$Ni := 1.39 \cdot Ai \cdot G \cdot D^2 = 382$$

PSI

## FOR D/H > 1.333 EXISTING CONDITION

Ni := 4.5·Ai·G·D·H·
$$\left[\frac{Y}{H} - 0.5\cdot\left(\frac{Y}{H}\right)^2\right]$$
·tanh $\left(0.866\cdot\frac{D}{H}\right) = 239$  PSI

CONVECTIVE HOOP TENSILE FORCE FOR ALL PROPORTIONS OF D/H:

Nc := 
$$\frac{0.98 \cdot \text{Ac} \cdot \text{G} \cdot \text{D}^2 \cdot \left[ \cosh \left[ \frac{3.68 \cdot (\text{H} - \text{Y})}{\text{D}} \right] \right]}{\cosh \left( \frac{3.68 \cdot \text{H}}{\text{D}} \right)} = 64 \qquad \text{PSI}$$

## HYDROSTATIC HOOP TENSILE FORCE

$$Nh := 2.6 \cdot G \cdot Y \cdot D = 794$$

PSI

$$\sigma s := \frac{\sqrt{Ni^2 + Nc^2 + (Nh \cdot Av)^2}}{t1} \qquad \sigma s = 3633$$

$$\sigma := \frac{Nh}{t!} = 10624 \qquad PSI$$

#### TOTAL STRESS

$$\sigma t := \sigma + \sigma s = 14258$$
 PSI

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ALLOWABLE TENSILE STRESS

$$\sigma a := \frac{4}{3} \cdot .4 \cdot Fy2 = 19200$$
 >  $\sigma t = 14258$  PSI OK!

XVIII. LONGITUDINAL SHELL COMPRESSION FROM SEISMIC FOR UNANCHORED TANK

FOR CLASS 2 STEEL

$$R := D.6 = 166.5$$
 IN

$$0 < \frac{t1}{R} = 0.00045$$

 $0 < \frac{t1}{R} = 0.00045$  < 0.0035372 THEREFORE, ELASTIC BUCKLING CONTROLS

ALLOWABLE COMPRESSIVE STRESS:

$$\sigma_a := 17.5 \cdot 10^5 \cdot \frac{\text{t1}}{\text{R}} \left[ 1 + 50000 \cdot \left( \frac{\text{t1}}{\text{R}} \right)^2 \right] = 793.04$$
 PSI

PRESSURE STABLIZING BUCKLING COEFFICIENT

$$P := P1 = 4.76$$
 PSI

$$t1 = 0.0747$$
 IN

**FOR** 

$$\frac{P}{E} \cdot \left(\frac{R}{t_1}\right)^2 = 0.82 > 0.064$$

$$\Delta C_c := 0.045 \cdot ln \left[ \frac{P}{E} \cdot \left( \frac{R}{t1} \right)^2 + 0.0018 \right] + 0.194 = 0.18$$
 BUT NO MORE THAN 0.22

CRITICAL BUCKLING STRESS

$$\Delta\sigma_{cr} := \frac{\Delta C_c \cdot E \cdot t1}{R} = 2406 \quad \text{PSI}$$

SEISMIC ALLOWABLE LONGITUDINAL SHELL COMPRESSION STRESS:

$$\sigma_e := 1.333 \cdot \left( \sigma_a + \frac{\Delta \sigma_{cr}}{2} \right) = 2661$$
 PSI

MAXIMUM LONGITUDINAL SHELL COMPRESSION STRESS

WHEN J = 1.16 <= 0.7785

$$\sigma c := \left[ (wt) \cdot (1 + 0.4 \cdot Av) + \frac{1.273 \cdot Ms}{D^2} \right] \cdot \frac{1}{12 \cdot t1} = 834$$
  $\sigma_e = 2661$  PS

OR WHEN 0.785 < J = 1.16 <= 1.54

**Bluescope Water** 

Job: MACR11077

$$\sigma c := \left[ \frac{wt \cdot (1 + 0.4 \cdot Av) + wL}{\left( 0.607 - 0.18667 \cdot J^{2.3} \right)} - wL \right] \cdot \frac{1}{12 \cdot t1} = 1059 \quad < \quad \sigma_e = 2661 \quad PSI$$

XIX, CHECK SLIDING FROM SEISMIC

COEFFICIENT OF FRICTION

$$\phi := 30$$

Vallow := 
$$\tan\left(\phi \cdot \frac{\pi}{180}\right) \cdot (\text{Ws} + \text{Wr} + \text{Wi} + \text{Wc})(1 - 0.4 \cdot \text{Av})$$

Vallow = 223080 >

$$Vf = 81206$$

**LBS** 

THEREFORE, TANK DOES NOT SLIDE.

XX. SLOSHING WAVE-HEIGHT FROM SEISMIC

< 4 CONVECTIVE DESIGN ACCELERATION FOR SLOSHING IS -</p>

$$A_f := \frac{K \cdot SD1 \cdot Ie}{Tc} = 0.4$$

FOR SEISMIC USE GROUP II WITH SDS>= 0.33g FREEBOARD REQUIREMENT IS -

$$d := 0.7 \cdot 0.5 \cdot D \cdot A_f = 3.93$$
 < FB = 4

XXI. SOIL BEARING PRESSURE BENEATH TANK

METHOD FROM US NAVY - NAVFAC DM-7.1 (Page 7.2-134) -

**ECCENTRICITY** 

$$e := \frac{Ms}{(WT + Ws + Wr)} = 0.94$$

FT

**RADIUS** 

$$R := D.0.5$$

**EFFFECTIVE AREA** 

$$Ae := \pi \cdot R^2 - \left( e \cdot \sqrt{R^2 - e^2} + R^2 \cdot a \sin \left( \frac{e}{R} \right) \right)$$

SOIL BEARING

$$q := \frac{(WT + Ws + Wr)}{Ae} = 731$$
 <  $q_{seismic} = 3333$ 

$$q_{\text{seismic}} = 3333$$
 PS

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## SITE EVALUATION SHEET

<u>Address</u>	14701 BODEGA H	WY TWI PC# DLD 12-010/			
Inspecto	r REX PETERSON	Date 1-10-1 2			
	sed construction appears to be located in: 026-17	20-006			
Flood	[ ] FIRM Flood Zone (ASFH) BFE =ft. NAVD.	[ ] Portions of property in flood zone but project site not in flood zone.			
Hazard:	Lowest finish floor at 12 above BFE =ft. NAVD.	[] Building is in FIRM Floodway.			
	[] Design for moving water is recommended	[] Main building on site is Post-FIRM.			
	Section is Ft/sec	[] Sensitive drainage area, review by drainage section recommended.			
	Section is Ft/sec	[] Appears to be a "substantial improvement" (40%), therefore flood			
	[ ] Area subject to flooding (not on adopted FIRM).	regulations apply.  [] Located inside the Laguna de Santa Rosa below elevation of 75 ft			
	[ ] Project is on flood zone major damage list.	(Ordinance #4906).			
Geo- technical:	[] Flood Prone Urban Area defined by Ordinance #4906. [] Area of suspected slides, slumps, earth flow, or soil creep. (a)	[] Area without recommended setback from stream (Drainage Division recommendations).			
	[ ] Area of previous fill placement. (g)	[ ] Area of high moisture content in soil. (f)			
	[ ] Area of suspected expansive soil. (c)	Area subject to high erosion (water or wind).			
	[] Area without sufficient slope setback as set forth in UBC Section 1806. (b)	[] Area of soft soil due to past deep ripping or cultivation below minimum foundation depth. (h)			
	[ ] Area subject to possible liquefaction. (e)	[] Area within 1000 feet of a solid waste disposal site.			
	[] Area of suspected soft, compressible, or organic soil with low bearing capacity.	[] Non exempt structure per test bulletin B-28.			
	Soils Investigation:	Required Available [] Not Required []			
Geologic:	[ ] Located in the Alquist-Priolo Special Studies Zone.	[] Geologic report required (see CGS Publication 42).			
Seismic:	Seismic Design Category (SDC) D E [ ]	[ ] Pictures available in S Drive			
General:	[] Building addition will affect the required light and ventilation in an existing room.	[] Indications of existing substandard conditions that are not addressed by the proposed construction.			
	[ ] Existing electric meter must be replaced.	[] Indications of past work done without a permit.			
	[ ] Existing gas meter must be replaced.	[] Grading permit required for road, driveway, or site preparation.			
	Slope is	[] Site is likely to be acceptable for conventional construction methods.			
Wind:	Exposure "B" Exposure "C" Exposure "D"	N.S.C. Air Pollution Control District[] Yes [] No			
an	25.0.6				
Five	. State:				
Tra Van hou					
T : D :	CO				
inight	iell - tew, Mostly				
* · · · ·					
OK to slandfiell					
Tank - engineered					
		,			