



Alpine, an ITW Company 8801 Folsom Blvd., Suite 107 Sacramento, CA 95826 Phone: (800)877-3678 (916)387-0116 Fax: (916)387-1110 sacseals@itwbcg.com

Site Information:	Page 1:
Customer: American Truss Company, Inc.	Job Number: 9898
Job Description: /DODGE RESIDENCE /MEAD CLARK	
Address: 4601 PRESSLEY ROAD, SANTA ROSA, CA 95404	

Job Engineering Criteria:	
Design Code: IBC 2018 Res	IntelliVIEW Version: 18.02.01B through 19.02.02
	JRef #: 1WTH8800008
Wind Standard: ASCE 7-16 Wind Speed (mph): 110	Roof Load (psf): 20.00-10.00- 0.00-10.00
Building Type: Closed	Floor Load (psf): None

This package contains general notes pages, 22 truss drawing(s) and 6 detail(s).

ltem	Drawing Number	Truss	Item	Drawing Number	Truss
1	072.20.0720.36040	G1	2	072.20.0720.38787	T1
3	072.20.0720.43520	T2	4	072.20.0734.33507	Т3
5	072.20.0722.40320	G4	6	072.20.0723.07977	Τ4
7	072.20.0724.01680	Т5	8	072.20.0724.45277	T6*
9	072.20.0724.50113	T7*	10	072.20.0724.53447	Т8
11	072.20.0727.59853	Т9	12	072.20.0728.11763	T10
13	072.20.0728.13530	T11	14	072.20.0728.14970	T12
15	072.20.0728.16570	T13	16	072.20.0728.19413	T14
17	072.20.0728.21190	T15	18	072.20.0728.23760	T16
19	072.20.0728.26327	T17	20	072.20.0736.26803	T18
21	072.20.0721.25900	T19	22	072.20.0721.56867	G20
23	A11515ENC160118		24	GABRST160118	
25	GBLLETIN0118		26	BRCLBSUB0119	
27	BRWEBLOK1014		28	160TL	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer. The Truss Design Engineer. The Truss Design Engineer on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc). -R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment. W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

- 1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
- 2. ICC: International Code Council; www.iccsafe.org.
- 3. Alpine, a division of ITW Building Components Group Inc.: 13723 Riverport Drive, Suite 200, Maryland Heights, MO 63043; <u>www.alpineitw.com</u>.
- 4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
- 5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcindustry.com.



SEQN: 312733	GABL	Ply: 1	Job Number: 9898	Cust: R 880	JRef: 1V	NTH8800008	T2
FROM:		Qty: 1	/DODGE RESIDENCE /MEAD CLARK	DrwNo: 07	2.20.072	0.36040	
Page 2 of 2			Truss Label: G1	/ RT	т	03/12/2020	
Additional Notes							
See DWGS A11515EN	IC1601	18, GBLLETIN01	18, &				

GABRST160118 for gable wind bracing and other requirements. Stacked top chord must NOT be notched or cut in

area (NNL). Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



American Truss Company, Inc. 4550 Spring Hill Road PETALUMA CA 94952 (707) 763-8713 Ext



Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing,any failure to build the truss in conformance with ANSUTPI 1, or for handling, shipping, installation and bracing of trusses A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSUTPI 1 Sec.2. For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.com; ICC: www.iccsafe.org





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SEQN: 313018	GABL	Ply: 1	Job Number: 9898	Cust: R 880	JRef: 1WTH8800	008 T20
FROM:		Qty: 1	/DODGE RESIDENCE /MEAD CLARK	DrwNo: 072	2.20.0722.40320	
Page 2 of 2			Truss Label: G4	/ RT	T 03/12/20)20
Additional Notes						
See DWGS A11515EN GABRST160118 for gal requirements.	C1601 ble win	18, GBLLETIN01 Id bracing and oth	18, & ner			

Stacked top chord must NOT be notched or cut in area (NNL). Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

Shim all supports to solid bearing.



WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING! **IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building component Satety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, drawings 160A-Z for standard plate positions.

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For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.com; ICC: www.iccsafe.org

SEQN: 312619	COMN	Ply: 1	Job Numb	er: 9898			Cust: R 880 JRef: 1WTH8800008 T3
FROM:		Qty: 16	/DODGE R	ESIDENCE /MEAD CLARK			DrwNo: 072.20.0723.07977
			Truss Lab	el: 14			/ RTI 03/12/2020
	-	7'1"9	+ 13 - 6'	3'3" 19'4" 1"7	$\frac{25'1"11}{5'0"11}$ $\frac{32'9"1}{5'0"11}$	13	40'9"8
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					≡6 <u>X</u> 10 F		
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			40	Ē		^{₹4X6}	
		4	12	≣7X8	(a) (a)		
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0"3				N ≡4X4	(a) (a)		
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		/			11370	-340	11320
	L					451014	
	•				—		- X
	₁ 5"8	6'6"2	5'1	1"9 6'1"4	6'1"4 ₁ 7'9"1	5 ,	7'9"15
	-1 _{5"8}	6'11"10	12	11"4 19'0"7 	25'1"11 32'11	"9 	40'9"8 3'
Loading Criteria (pef)	Win	d Criteria		Snow Criteria (Pa Pf in PSF)	Defl/CSI Criteria	▲ Maximum R	eactions (lbs)
TCLL: 20.00	Win	d Std: ASCE 7-1	6	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravit	y Non-Gravity
TCDL: 10.00	Spe	ed: 110 mph		Pf: NA Ce: NA	VERT(LL): 0.198 C 999 360	Loc R+ /R	- /Rh /Rw /U /RL
BCLL: 0.00	Risk	Category: II		Lu: NA Cs: NA	VERT(CL): 0.393 C 767 240	Q 951 /-	/- /572 /33 /188
BCDL: 10.00	EXF	C Kzt: NA		Show Duration: NA	HORZ(LL): 0.061 C	M 2396 /-	/- /1422 /9 /- S /- /356 /102 /-
NCBCI I · 10.00	Mea	an Height: 15.00 ft		Code / Misc Criteria	Creep Factor: 2.0	Wind reactions	based on MWFRS
Soffit: 2.00	BCC	DL: 6.0 pst DL: 6.0 psf		Bldg Code: IBC 2018 Res	Max TC CSI: 0.631	Q Brg Width	= 5.5 Min Req = 1.5
Load Duration: 1.25	MW	FRS Parallel Dist	: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.596	J Brg Width	= 5.5 Min Reg = 2.6
Spacing: 24.0 "	C&C	Dist a: 4.08 ft		Rep Fac: Yes	Max Web CSI: 0.619	Bearings Q, M	, & J are a rigid surface.
	LOC.	GCpi: 0.18	у	Plate Type(s):		Members not li	isted have forces less than 375#
	Win	d Duration: 1.25		WAVE	VIEW Ver: 18.02.01B.0321.09	Chords Tens.	Comp. Chords Tens. Comp.
Lumber						B-C 316	-2585 F-G 2407 -269
Top chord 2x6 DF-L #2 Bot chord 1 5"x2 5625"	(g)	#2(a)				C - D 229	- 1551 G - H 437 - 225
Webs 1.5"x3.5625" DF	-L Star	#2(9) ndard(g)				E-F 2362	- 252
Lumber shall be dried t	o a ma	aximum moisture	content			Maximum Bot	Chord Forces Per Ply (lbs)
of 19% prior to installat	ion.					Chords Tens.	Comp. Chords Tens. Comp.
Bracing						B - P 2432	- 315 O - N 1442 - 175
(a) Continuous lateral r	estrain	t equally spaced	on			P-O 2438	- 321
member.						Maximum Wo	h Forces Per Ply (lbs)
Plating Notes						Webs Tens.	Comp. Webs Tens. Comp.
Connectors in green lui NDS/TPI reduction fact	mber (ors	g) designed using				C - O 147	-944 I-M 295-2338
						O-D 406	0 L-G 372 -2068
Loading Bottom abord abooked	for 10		rrant			D-N 269	-1631 L-K 208 -426
live load.	101 10.		nent			E-L 325	-1969 K-H 151 -426
Truss designed for unb	alance	ed load using 0.00	/1.00			F-L 272	- 1803 H - J 160 - 533
windward/leeward facto	ors.			OPROFESSION			
Wind				SE SELL T. TANG	A CALL		
Wind loads based on M	/WFR	S with additional C	C&C	15/2	在1991		
Right end vertical not e	xpose	d to wind pressure	9.				
Wind loading based on	both o	gable and hip roof	types.	No. C 72160)~))		
Additional Notes							
Shim all supports to so	lid bea	rina			/*//		
				CIVIL	MIA		
				E OF CALIFO	<i>`</i> //		
				03/12/2020			
	WA	RNING READ	AND FOLL	OW ALL NOTES ON THIS D	RAWING!		American Truss Company, Inc.
IMPORTAI	NT	FURNISH THIS D	RAWING 1	O ALL CONTRACTORS INC	LUDING THE INSTALLERS	of BCSI (Building	4550 Spring Hill Road PETALUMA CA 94952
Component Safety Infor	mation	n, by TPI and SBC	A) for safet	y practices prior to performing	these functions. Installers shall p	rovide temporary	♥ (707) 763-8713 Ext
attached rigid ceiling. L	ocation	ns shown for perm	anent later	al restraint of webs shall have	bracing installed per BCSI section be Joint Details, unless noted other	ns B3, B7, or B10 erwise. Refer to	ý, <u> </u>
drawings 160A-Z for sta	indard	plate positions.		hall wat ha			
Aipine, a division of ITW truss in conformance wi	/ Build	Ing Components (SI/TPI 1, or for ha	roup Inc. s ndling, ship	ping, installation and bracing	aeviation from this drawing,any f of trusses A seal on this drawing	anure to build the	AN ITW COMPANY

listing this drawing, indicates acceptance of professional, engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.com; ICC: www.iccsafe.org



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SEQN: 312612	COMN	Ply:	1	Job Numb	er: 9898					Cust: R 8	30 JRef:1W	VTH8800008	5 T21 [·]
FROM:		Qty:	1	/DODGE R	ESIDENCE /ME	AD CLARK				DrwNo:	072.20.0724	1.45277	
				Iruss Lab	91: 16"					/		03/12/2020	
				9'6"3 9'6"3	-+- 17' 7'1	4"11 0"8 +	25'1"11 7'9"	-+	3'3"13 7'9'	^{r8} 15			
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					12	C			=3X4 F T2				
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	0"3 -	5					(;	a)	5 wa (a		5.4"1		
	2		A		M	247.0			(a)		- 3'8"		
	Ť∓₹					44					⊥±		
		≡4	X6(A1)					III3X8	B3 I	IIH0310			
									=688				
			<u>-</u>				- 40'6"			<u>3</u> "8			
				8'10"8	7'9"	9 .	8'2"5	7'10"6	7'7				
			-	9'4"			25'3"7		- - 40'	9"8			
			58										
			 -		25'5"4 -								
	147	10.1			0		Deflice: C	· • • • •	A \$4	Deceti	(lbc) +		
Loading Criteria (psf)	Win	a Crit d Std	ASCF 7-1	6	Ba: NA C++	a (Pg,Pf in PSF		n teria Annin loc I (dofi I	/# Maximum /# Gra	rity	: * זס ,(צמו) א N	er LF Ion-Gravitv	,
TCDL: 10.00	Spe	ed: 11	0 mph	0	Pf:NA Cl.	Ce: NA	VERT(LL):	0.353 B 855	360 Loc R+ /	R- / Rh	/ Rw	/U /	RL
BCLL: 0.00	Encl	osure	: Closed		Lu: NA Cs	: NA	VERT(CL):	0.601 B 502	240 O 890 /	· /-	/526	/1 /1	152
BCDL: 10.00	Risk	Cate	gory: II		Snow Duratio	on: NA	HORZ(LL):	-0.139 J -	- K* 167 /	· /-	/83	/32 /0	ר כ
Des Ld: 40.00	Mea	n Heid	aht: 15.00 ft		0.1.1		HORZ(TL):	0.182 B -	- K /	353			
NCBCLL: 10.00	TCD	L: 6.0	psf		Code / Misc Bida Code: II	Criteria BC 2018 Res	May TC CS	or: 2.0	H /	2696			
Load Duration: 1 25	BCD)L: 6.0) psf Parallal Diat	h to 2h	TPI Std: 2014	4	Max BC CS	al: 0.946	Wind reaction	ns based o	n MWFRS		
Spacing: 24.0 "	C&C	Dist	a: 4.08 ft	. 11 10 211	Rep Fac: Var	ies by Ld Cas	e Max Web C	SI: 0.946	O Brg Wid	th = 5.5	Min Re Min Re	yq = 1.5	
	Loc.	from	endwall: NA	4	FT/RT:20(0)/	10(0)			Bearings O	& Kare a rig	gid surface.	74	
	\A/:	GC	Cpi: 0.18		Plate Type(s)	:		40.00.040.0004.0	Members no	t listed have	e forces les	s than 375	5#
Lumber	wine	a Dura	ation: 1.25		WAVE, HS		VIEW Ver:	18.02.01B.0321.0	Maximum T	op Chord I	-orces Per Chords	Tens Co	nmn
Top chord 2x6 DE-L #2	(a) T3	276 0	F-1 SS(a).		Wind loads	based on MW	ERS with addi	tional C&C		70 0550		004.4	4000
Bot chord 1.5"x3.5625"	DF-L i	#2(g) l	B3 2x6 DF-	L #2(g);	member des	sign.			А-В 3 В-С 4	72 - 3002 59 - 1951	D-E F-F	2914 -	1803 2499
Webs 1.5"x3.5625" DF-	L Star	ndard(g) W8,		Right end ve	ertical not expo	osed to wind p	ressure.	C - D 25	05 - 1285	F - G	3704 - 3	3643
Lumber shall be dried to) a ma	ximur	n moisture	content	Right cantile	ver is expose	d to wind						
of 19% prior to installati	on.				Wind loading	g based on bo	th gable and h	ip roof types.	Maximum E	ot Chord F	Chords	Ply (lbs)	nmn
Bracing										74 000		4750	4005
(a) Continuous lateral re	estrain	t equa	ally spaced	on					A-M 33 M-L 33	74 - 398 73 - 401	к-і І-Н	1753 - 1819 -	1805
member.		•							L-J 15	05 - 157			
Special Loads													
(Lumber Dur.Fac.=	1.25 /	Plate	Dur.Fac.=1	.25)					Maximum V	leb Forces	Per Ply (II	os)	mn
TC: From 61 plf at	0.	00 to	61 plf at	40.79								1015.00	
BC: From 60 plf at	25.	56 to	60 plf at	32.81					B-L 2	64 - 1816 01 0	J-E J-N	181 - 50 ·	- 853 - 413
Plating Notes									Č-J 3	42 - 2529	Î-G	3718 -:	3843
Connectors in green lun	nher (r	n) des	ianed usina						D-J 8	19 - 1663	G - H	2577 -2	2664
NDS/TPI reduction facto	ors.	g) 000	ignod donig	•		PROFESS/0	Va		J-K a	/6 - 2163			
All plates are 5X6 except	ot as n	oted.			181	SELL I. TAN							
Loading						-	西里	-					
Truss transfers a maxim	num he	orizon	tal load of										
3500 # (85.95 plf) ald	ong to	p chor	rd, from eith	er am and		No. C 7216	o)~))						
connections are to be d	esigne	ed by E	Engineer of	Record.									
Drag Loads: Force(#) (PLF)	Mbr	Start End		\\★\		/ *//						
3500 8	5.95 25.76	BC	25.29 40.7	9).79	102	CIVIL	/18//						
Bottom chord checked f	or 10.	00 psf	non-concu	rrent		EOFCALIE	RM						
live load.						03/17/2020							
	***/ * -										<u> </u>		
**IMPORTAN	-^wAF	FURN	J™ READ ISH THIS D	AND FOLL	OVV ALL NOTE	25 ON THIS E RACTORS IN	CLUDING THE	INSTALLERS			American 1 4550 Sprin	Truss Compani g Hill Road	ny, Inc.
Trusses require extreme	care i	in fabr	icating, har	dling, shipp	ing, installing a	and bracing. F	Refer to and fo	llow the latest edi	tion of BCSI (Build	ling	PETALUM (707) 763-{	A CA 94952 8713 Ext	
bracing per BCSI. Unles	s note	d othe	erwise, top c	hord shall h	ave properly at	ttached structu	iral sheathing	and bottom chord	shall have a prop	erly 10.			
as applicable. Apply pla drawings 160A-Z for sta	ates to	each	face of trus	s and positi	on as shown a	above and on	the Joint Detai	ls, unless noted of	otherwise. Refer	to			

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANS/ITPI 1, or for handling, shipping, installation and bracing of trusses **A seal on this drawing or cover page** listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANS/ITPI 1 Sec.2. For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.com; ICC: www.iccsafe.org



SEQN: 312623	COMN	I Ply: 1	Job Numb	er: 9898						Cust: R 88	0 JRef:1V	VTH88000)08 T22 [`]
FROM:		Qty: 1	/DODGE R	ESIDENCE /MEAD	CLARK					DrwNo: (072.20.0724 २тт	4.50113 03/12/20	20
			Truss Labe	#I: 17						/ 1	X 11	03/12/20	20
											~~~		
	-	7'1"9 7'1"9	•	14'11"5 7'9"12 ++→	19'9"15 4'10"9	+ <u>25111</u> 5'3"12	+ 32	'9"13 3"2	+-	40 7'1	9"8 1"11	+	
				10.2				-					
							5X6						
							E						
l t						. //							
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			4 🖂	≢/X8 C	/			$\geq$	-	<b>_</b> .			
2 T				The	$\leq$					$\sim$	≋4	×6	
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03		- T	$\leq$	B M		L (a)		(a)		(-)		1	
مت ا				≡4X4				$\mathcal{N}$		(a)		38	
	A	576	4	2.44					$\mathbb{A}$	/			
		≥5∧0		12			ĸ <b>Ľ~</b> ~~.	///	77,	///	///	┦╷┸	
<b>≝</b> 4	X6(A1)					II	3X6	=	4X12		113	X6	
	<u>k</u>	- 6'11"12					2					-1	
		01010											
	58	6'6"2 6'11"10		7'7"14 14'7"9	4'10"13	+ 5'7"5 25'1"11	+ 79	9"15 "11"9	+-	7'9 40'	9"15 '9"8	┥	
	<u> </u>					20111	02						
Loading Criteria (psf)	Win	d Criteria	e	Snow Criteria (	Pg,Pf in PSF)	Defl/CSI Criteri	a   / -#  ///	<b>▲</b> Maxi	Gravity	eactions	(Ibs), or *	=PLF	/itv
TCLL: 20.00	Spe	a Siu: ASOL / - n ed: 110 mph	3	PG:NA CUN	A CALLINA Ce'NA	VERT/II) 0.07	100 L/aeii L/# 7 C 999 360	Loc R	+ /R-	/ Rh	/ Rw	/U	/ RL
BCLL: 0.00	Enc	losure: Closed	I	Lu:NA Cs:N	A 00.101	VERT(CL): 0.08	AC 999 240	0 263	× /_	/-	/141	/-	/152
BCDL: 10.00	Risk	Category: II	l	Snow Duration:	NA	HORZ(LL): 0.07	73 J	N 980	) /-	, /-	/750	, /-	/-
Des Ld: 40.00	EXP Mea	2: C Kzt: NA	l			HORZ(TL): 0.09	90J	K* 135	5 /-	. /-	/72	/-	/-
NCBCLL: 10.00	TCE	DL: 6.0 psf	l	Code / Misc Cri	iteria	Creep Factor: 2.	.0	К  ц	/-11 /-13	51 •9			
Soffit: 2.00	BCE	DL: 6.0 psf		Bidg Code: IBC	2018 Res	Max TC CSI: U	0.509	Wind re	actions	based or	n MWFRS		
Load Duration: 1.25	MW	FRS Parallel Dist:	> 2h	Ren Fac: Varies	hv I.d Case	Max Web CSI: (	).510 ) 815	O Brg	Width	= 5.5	Min Re	əq = 1.5	
Spacing. 24.0	Loc.	from endwall: not	in 13.00 ft	FT/RT:20(0)/10(	0)	1100		N Bro	y Width	= 3.5 - 187	Min Ke Min Re	∋q = 1.5 ∽q = -	
		GCpi: 0.18		Plate Type(s):	,			Bearing	IS O, N,	& K are a	a rigid surfa	эч = - асе.	
	Win	d Duration: 1.25		WAVE		VIEW Ver: 18.02	2.01B.0321.09	Membe	rs not lis	sted have	forces les	s than 3	375#
Lumber				Wind				Maxim	um Top	Chord F	Forces Per	Ply (lbs	5) Oomo
Top chord 2x6 DF-L #2	2(g)	#0/~\		Wind loads bas	sed on MWF	RS with additiona	I C&C	Ciloius	1013.			Tens.	Comp.
Webs 1.5"x3.5625" DF	-L Star	#2(g) ndard(g)		Dight and verti	1. Sol not expo	and to wind press	170	A-B B-C	438 140	- 504 1225	D - E	1019	- 278 1020
Lumber shall be dried	to a ma	aximum moisture c	content	Wind loading b	esed on bot	h cable and hip ro	of types	C-D	165	- 678	F-G	2099	- 1874
of 19% prior to installa	tion.			Willia 1000011-5 -		Il gubio unap	01 (9900.						
Bracing								Maxim	um Bot	Chord F	orces Per	Ply (lbs	5)
(a) Continuous lateral	restrain	nt equally spaced of	on					Chords	Tens.	Comp.	Chords	Tens.	Comp.
member.		-						M - L	1633	- 523	K - I	1751	- 1773
Plating Notes								L-J	1601	- 1103	I-H	1770	- 1757
Connectors in green lu	umber (	g) designed using						Maxim	um Web	Forces	Per Plv (il	hs)	
NDS/TPI reduction fac	tors.							Webs	Tens.0	Comp.	Webs	Tens.	Comp
All plates are 3X4 exce	ept as n	noted.						N - B	201	- 813	J-K	1212	- 2311
Loading								B - M	1339	- 391	J-F	111	- 454
Truss transfers a maxi	mum h	orizontal load of						C-L	92 290	- 602	J-1	1842	- 2114
3500 # (85.95 plt) a	Iong to	p chord, from eithe pdicated Diaphrac	er mand		AEESSIA			L- D D - J	კიყ 174	U - 1190	F-1 I-G	1896	- 495 - 2182
connections are to be	designe	ed by Engineer of	Record.	(OPT	TTA	4		Ē-J	124	-777	G-H	1450	- 1384
Drag Loads: Force(#)	(PLF)	Mbr Start End	n	11 2 65	LL I. IAN								
3500	225.76	BC 25.29 40	9 .79	12		色度							
Bottom chord checked	i for 10.	.00 psf non-concur	rrent	1 H		<u> </u> ä							
live load.					o. C 72160	]~]]							
Truss designed for unt windward/leeward fact	balance	d load using 0.00	/1.00	\\ \		/ //							
	0.0.			<b>\\</b> *\		/*//							
Additional Notes				130	CIVIL	\st							
Shim all supports to so	lid bea	iring.		ATEC	DECALIEO	and							
					3/12/2020	/							
	*****				ON THE D								
**IMPORTA	**WAI	RNING** READ	AND FOLL	OW ALL NOTES	ON THIS D	RAWING! LUDING THE INS	TALLERS				American 4550 Sprin	Fruss Com g Hill Roar	pany, Inc. J
Trusses require extrem	e care	in fabricating, han	dling, shipp	ing, installing and	bracing. R	efer to and follow t	the latest edition	of BCSI (	Building	J	PETALUM (707) 763-	Ā CA 949 8713 Ext	52
bracing per BCSI. Unle	ss note	ad otherwise, top ch	hord shall h	ave properly attac	ched structu	al sheathing and t	bottom chord sh	all have a	properly	/			
as applicable. Apply p	lates to	b each face of trus	s and positi	on as shown abo	ove and on the	he Joint Details, u	inless noted oth	erwise. F	Refer to	,			
Alpine, a division of ITV	N Build	ling Components (	Group Inc. s	hall not be respo	nsible for an	v deviation from th	nis drawing,any f	failure to b	uild the		ΆL		

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Sacramento, CA 95826



SEQN: 312654 0	COMN Ply: 1	Job Number: 9898		Cust: R 880 JRef: 1WTH8800008 T17
FROM:	Qty: 1	/DODGE RESIDENCE /MEAD CLARK		DrwNo: 072.20.0728.13530
		Truss Label: T11		/ RTT 03/12/2020
	7'1	1"9 , 13'1"2 ,	19'0"2 , 25'1"11	26'8"8 31'1"3 35'9"8
	<b>≓</b> 7'1	1"9 <del>                                    </del>	5'11"1 6'1"8	1'6"13 4'4"11 4'8"4
			=5X	а Пара
<b>T</b>				G T
			≡3X4	<b>≋</b> 3X4
			E	Н
		12 -778		//   ₹3Х4
0		4 D 1/00 D		
10		T		
[∞] T		≡4X6 C		
			≡H0508 ≡5X	3
6.0		P = 4X12		
ï _	в	T _Q		
<u>+</u> + <u>e</u> A_		2.44		
[ [	≡3X4(A1)	12		≡5x8 ≡3x8 ⊪3x6 <b>⊥</b>
				III 3X6
	6'11"1	12	— 19'11"8 ———	
+	— 3' <del>_ = ⁵"8</del> 6'6	6"4 5'9"8	6'2"14 6'1"8	1'6"13 4'6"8 4'6"8
'	⁵ '5''8 6'1	11"12 12'9"5	19'0"2 25'1"11	26'8"8 31'3" 35'9"8
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pa Pf in PS)		▲ Maximum Reactions (lbs)
TCLL: 20.00	Wind Std: ASCE 7-1	6 Pa: NA Ct NA CAT N	A PP Deflection in loc I /defl I /#	Gravity Non-Gravity
TCDL: 10.00	Speed: 110 mph	Pf: NA Ce: NA	VERT(LL): 0.051 D 999 360	Loc R+ /R- /Rh /Rw /U /RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.103 D 999 240	R 483 /- /- /247 /21 /162
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.017 N	Q 1027 /- /- /648 /- /-
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.034 N	M 1603 /- /- /895 /- /-
NCBCLL: 10.00	TCDI: 6.0 pcf	Code / Misc Criteria	Creep Factor: 2.0	J 194 /-125 /- /94 /60 /-
Soffit: 2.00	BCDL: 6.0 psi	Bldg Code: IBC 2018 Res	Max TC CSI: 0.333	Wind reactions based on MWFRS
Load Duration: 1.25	MWFRS Parallel Dist:	: h to 2h TPI Std: 2014	Max BC CSI: 0.388	R Brg Width = 5.5 Min Req = 1.5 O Brg Width = 3.5 Min Reg = 1.5
Spacing: 24.0 "	C&C Dist a: 3.58 ft	Rep Fac: Yes	Max Web CSI: 0.920	M Brg Width = 5.5 Min Reg = $1.7$
	Loc. from endwall: not	t in 9.00 ft FT/RT:20(0)/10(0)		J Brg Width = - Min Reg = -
	GCpi: 0.18	Plate Type(s):		Bearings R, Q, & M are a rigid surface.
	Wind Duration: 1.25	WAVE, HS	VIEW Ver: 18.02.01B.0321.09	Members not listed have forces less than 375#
Lumber				Maximum Top Chord Forces Per Ply (lbs)
Top chord 2x6 DF-L #2(	(g)			Chords Tens.Comp. Chords Tens. Comp.
Bot chord 1.5"x3.5625"	DF-L #2(g)			C-D 87-1274 G-H 750-44
Webs 1.5"x3.5625" DF-	L Standard(g)			D-E 96-914
Lumber shall be dried to	a maximum moisture (	content		
	011.			Maximum Bot Chord Forces Per Ply (lbs)
Plating Notes				Chords Tens.Comp. Chords Tens. Comp.
Connectors in green lun	nber (g) designed using	]		P-O 1196 - 138 N-L 106 - 666
NDS/TPI reduction factor	ors.			O - N 831 - 66
Bottom chord chockod f	or 10.00 pcf pop concu	rront		Maximum Web Forces Per Ply (lbs)
live load.				Webs Tens.Comp. Webs Tens. Comp.
Truss designed for unba	alanced load using 0.00	)/1.00		Q-C 180-892 N-G 1080 -85
windward/leeward factor	rs.			C-P 1344 - 164 G-L 156 - 1334
Million al				E-O 460 0 L-M 197 - 1569
				E-N 154-1169 L-H 107-690 EN 75-201 L/ 275 0
Wind loads based on M	WFRS with additional C	PROFESS	ONA	1-N 75-391 II-K 375 0
Bight and vortical not av	record to wind process	OF ELL T. T.	and the second	
Wind loading based on	both goble and his roof			
wind loading based on	both gable and hip tool	types.	521	
Additional Notes			191	
Shim all supports to soli	d bearing.	No. C 72	160	
		\\★\	/*//	
		I all and	/_//	
		AITECIVIL	RNIE	
		OFCAL		
		03/12/20	20	
	**WARNING** READ	AND FOLLOW ALL NOTES ON THIS		American Truss Company, Inc.
**IMPORTAN	care in fabricating bon	JKAWING TO ALL CONTRACTORS IN adling shipping installing and bracing	CLUDING THE INSTALLERS	of BCSI (Building PETALUMA CA 94952
Component Safety Inforr	nation, by TPI and SBC	CA) for safety practices prior to performing	g these functions. Installers shall p	rovide temporary (707) 763-8713 Ext
attached rigid ceiling. Lo	cations shown for perm	nanent lateral restraint of webs shall ha	ve bracing installed per BCSI section	is B3, B7, of B10,
as applicable. Apply pla drawings 160A-Z for star	ates to each face of trus ndard plate positions	ss and position as shown above and on	the Joint Details, unless noted other	
Alpine, a division of ITW	Building Components (	Group Inc. shall not be responsible for a	ny deviation from this drawing, any fa	ailure to build the

Truss in conformation of true building on points Group into Stant to be responsible to bring deviation and bracing of trusses A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.com; ICC: www.iccsafe.org

SEQN: 312652	COMN	Ply: 1	Job Numb	er: 9898				Cust: R 880	JRef:1WTH8	800008 T19 [·]
FROM:		Qty: 1	/DODGE R	ESIDENCE /MEAD CLARK				DrwNo: 07	72.20.0728.149	970 D/2000
			Truss Lab	el: 112				/ R	11 03/12	2/2020
			<u>"8</u> +	<u>12'0"8</u> 4'10"15 4'10"8	15 <u>25'1"11</u> + 8'2"12 ►	26'8"8	<u>31'1"3</u> 4'4"11	+	35'9"8 4'8"4	
				41010 4100	02.12	1010			404	
					5)	<u>(8</u>				
T T						- /// 470 G				Ŧ
						A	"	≋3X4		
					≡4X4 E	//			5074	
			4	12		//			<i>₹3</i> ,74	
0110				D		//			7	T   9
8,10			≢4X	6		¥_dk				.10"
					0	, L				- 54" -
4 2				P = 479	-10300 -07	<b>`</b>				Ĩ
		В		⊇ ຊ					H.	
	$\bigcirc$	R	m2A-	2.44		M		ĸ	J	-
		≡3X4(A1)		12		≡6X8 ∭3X6	3	≡3X8	<b>∥</b> 3X6	T
		<b></b> 6'11"1	12		- 19'11"8	-+-		- 8'10"4		
		-	-			-			-	
+	— 3' –	5 8 6	6"4	4'8"15 5'2"4	8'2"12	1'6"13	4'6"8		4'6"8	
	-	'5"8 6'	11"12 '	11'8"11 ' 16'10"1	5 ' 25'1"11	26'8"'8	31'3"	1	35'9"8	
Loading Criteria (psf)	Win	d Criteria		Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	<b>▲</b> Ma	aximum Re	eactions (	lbs)	
TCLL: 20.00	Wine	d Std: ASCE 7-1	6	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#		Gravity	/ Dh	Non-C	Bravity
TCDL: 10.00	Spe	ed: 110 mph		Pf: NA Ce: NA	VERT(LL): 0.061 O 999 36		K+ /K-	/ КП	/RW /(	) / KL
BCLL: 0.00	Risk	Category: II		Lu: NA CS: NA	VERT(CL): 0.123 O 999 24		470 /-	/-	/232 /24	4 /162
Doc Ld: 10.00	EXP	C Kzt: NA		Show Duration. NA	HORZ(LL): 0.019 N	M ·	1065 /-	/-	/672 /- /878 /-	/- /-
NCBCLI · 10.00	Mea	n Height: 15.00 ft	t	Code / Misc Criteria	Creep Factor: 2.0	J	201 /-11	3 /-	/99 /5	5 /-
Soffit: 2.00	BCD	1:60 pst		Bldg Code: IBC 2018 Res	Max TC CSI: 0.351	Wind	d reactions	based on	MWFRS	
Load Duration: 1.25	MW	FRS Parallel Dist	: h to 2h	TPI Std: 2014	Max BC CSI: 0.566		Brg Width	= 5.5 = 3.5	Min Req =	1.5 1.5
Spacing: 24.0 "	C&C	Dist a: 3.58 ft		Rep Fac: Yes	Max Web CSI: 0.533	M	Brg Width	= 5.5	Min Req =	1.7
	Loc.	from endwall: no	t in 9.00 ft	F I/R I:20(0)/10(0) Plate Type(s):		J	Brg Width	= -	Min Req =	-
	Wine	d Duration: 1.25		WAVE, HS	VIEW Ver: 18.02.01B.0321.09	– Bear	rings R, Q, bers not lis	& M are a	forces less the	an 375#
Lumber	1					Max	imum Top	Chord Fo	orces Per Ply	(lbs)
Top chord 2x6 DF-L #2	(g)					Cho	rds Tens.	Comp.	Chords Te	ns. Comp.
Bot chord 1.5"x3.5625"	DF-L	#2(g)				В-С	C 448	- 200	D-E 1	17 - 1183
Webs 1.5"X3.5625" DF-	-L Star	ndard(g)				C - D	D 63	- 1076	G-H 6	68 - 35
of 19% prior to installati	ion.		content			Maria			D Dk-	(11)
Bassian						Cho	Imum Bot rds Tens (	Cnora Fo Comp	Chords Ter	(IDS) ns Comp
(a) Continuous lateral r	ootroin	t oqually appoad	<b>~</b>			<u> </u>	<u> </u>	200	0 N 44	104 407
member.	estrain	it equally spaced	on			р-0	פי ג 1021 C	- 399	N-L	97 - 597
Disting Notes						-				
Connectors in green lui						Max	imum Web	Forces F	Per Ply (lbs)	
NDS/TPI reduction fact	ors.	y) designed dsing	J			Web	os Tens.(	Comp.	Webs Te	ns. Comp.
Looding						Q - (	C 178	- 931	N-G 11	76 - 98
Loading Bettem abord aboeked	for 10		rront			C-F	P 1312	- 164	G-L 1	138 - 1352
live load.	101 10.		literit			E - N	V 181	- 1324	L-M 1	102 - 632
Truss designed for unb	alance	d load using 0.00	/1.00			F - N	l 129	- 506		
windward/leeward facto	ors.			PROFESS/ON						
Wind				DE FIL T. TAN	Kell (a)					
Wind loads based on M	WFR	S with additional (	C&C	55						
member design.				1 Sta						
Right end vertical not e	xposed	d to wind pressure	e.	AL 0 70400						
wind loading based on	both g	able and hip root	types.	NO. C 72160	/ //					
Additional Notes				N <b>*</b> \	/ * //					
Shim all supports to sol	id bea	ring.			/ 11					
				AN CIVIL	MA MA					
				OFCALIFO	//					
				03/12/2020						
******	**WA	RNING** READ	AND FOLL	OW ALL NOTES ON THIS DE	RAWING!				American Truss	Company, Inc.
Trusses require extreme	NI™ I ∋carei	FUKNISH THIS I	ndlina. shipr	O ALL CONTRACTORS INCI	LODING THE INSTALLERS efer to and follow the latest edition	n of BC	SI (Building	1	4550 Spring Hill PETALUMA CA	94952
Component Safety Infor	mation is note	, by TPI and SBC	CA) for safet	y practices prior to performing ave properly attached structure	these functions. Installers shall a sheathing and bottom chord shall a	provide all have	temporary	/	(/U/) 763-8713	EXt
attached rigid ceiling. Lo as applicable. Apply no	ocation ates to	ns shown for perro	nanent later	al restraint of webs shall have on as shown above and on th	bracing installed per BCSI section be Joint Details. unless noted oth	ns B3, Ì erwise	B7, or B10, Refer to			
drawings 160A-Z for sta	ndard	plate positions.							ALF	<b>INE</b>
Alpine, a division of ITW	Buildi	ing Components	Group Inc. s	hall not be responsible for any	deviation from this drawing, any	tailure t	o build the		~	AN ITW COMPANY

Irruss in conformance with ANSI/TP1 1, or for handling, shipping, installation and bracing of trussesA seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TP1 1 Sec.2. For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.com; ICC: www.iccsafe.org



8801 Folsom Blvd., Suite 107

Sacramento, CA 95826





8801 Folsom Blvd., Suite 107

Sacramento, CA 95826

SEQN: 312666	COMN	Ply:	1	Job Numb	er: 9898						Cust: R 8	BO JRef:1V	VTH88000	08 T11 [·]
FROM:		Qty:	1	/DODGE R	ESIDENCE /MEAD C	LARK					DrwNo:	072.20.072	3.23760	
				Truss Lab	ei: 116						/	RH	03/12/202	20
	<b>L</b> =		7'1"13	8'9'	11 12'9"12	17'4"7		22'2"4	26'10	"4	31'1"3	35'9"8	<del></del>	
			7'1"13	' 1'7'	'14 4'0"1 '	4'6"10	I	4'9"13	4'8"1	I	4'2"15	4'8"4	I	
							=6X6			III1 5X3		111	384	
							F	G	1	H	I		Ĵ,	
			10		F	$\square$		T		P			यी ।	i i
			4		-								´	
				≢7X8 C		$\sim$							g	
			_	TK						$\mathbb{N}$				2.3"2
<del>-</del>	в	_	$\square$									//		
				∥1.5X3				Ū.		8				,
	_ <b>⊤</b>  ¶¶ ≡3X4(/	A1)	-	≡5X	.6					N ¹²⁸ =5X8	=3X8		K 12X4	
		,	12 2	.44						<b>∥</b> 3X6	0,10			
	<b>k</b>		6'11"12 —	-		1:	9'11"8 —							
	_			_						-			-	
<b> </b>	5"8		6'6"4			4'6"10	+-	4'9"13	4'9"13		4'6"8	4'6"8	<b></b>	
	5.9		61112	862	2 1263	170-14		211011	268.8		313	35.8.8		
Loading Criteria (psf)	Win	d Crite	eria		Snow Criteria (Po	Pf in PSF)	Defl/CS	Criteria		▲ Maxim	num Reactions	(lbs)		
TCLL: 20.00	Win	d Std:	ASCE 7-1	6	Pg: NA Ct: NA	CAT: NA	PP Def	ection in loc	L/defl L/#		Gravity	N	lon-Gravi	ity
TCDL: 10.00	Spe	ed: 11	0 mph		Pf: NA	Ce: NA	VERT(L	L): 0.022 P	999 360	Loc R+	/R- /Rh	/Rw	/0	/ RL
BCLL: 0.00	Risk	Cate	ory: II		Lu: NA Cs: NA	^		CL): 0.044 P	999 240	T 488	-  -	/274	/12	/167
Des I d: 40.00	EXP	C K	zt: NA		Show Duration. N	<u> </u>	HORZ(	L): 0.008 M		N 1394	5 /- /- 4 /- /-	/034	/29 /42	/- /-
NCBCLL: 10.00	Mea	n Heig	ght: 15.00 ft 		Code / Misc Crite	eria	Creep F	actor: 2.0		K 262	/- /-	/109	/4	/-
Soffit: 2.00	BCE	DL: 6.0	psf		Bldg Code: IBC 2	018 Res	Max TC	CSI: 0.300	)	T Bra	actions based o Width = 5.5	n MWFRS Min R	ea = 1.5	
Load Duration: 1.25	MW	FRS F	Parallel Dist	: h/2 to h	Rep Fac: Yes		Max BC	- CSI: 0.289	)	S Brg	Width = 3.5	Min R	əq = 1.5	
Spacing. 24.0	Loc.	from (	a: 3.58 π endwall: no	t in 9.00 ft	FT/RT:20(0)/10(0)	)	indx in		-	N Brg K Brg	Width = 5.5 Width = -	Min Re Min Re	eq = 1.5 eq = -	
		GC	Cpi: 0.18		Plate Type(s):					Bearings	s T, S, & N are a	a rigid surfa	ace.	
	Win	d Dura	ation: 1.25		WAVE		VIEW V	/er: 18.02.01E	3.0321.09	Member	s not listed have	e forces les	s than 37	75# •\
Lumber	2(a)									Chords	Tens.Comp.	Chords	Tens. (	Comp.
Bot chord 1.5"x3.5625	2(9) "DF-L :	#2(g)								C - D	7 - 508	E-F	137	- 829
Webs 1.5"x3.5625" DF	-L Star	ndard(	g) 							D - E	117 - 1029	F-G	84	- 454
of 19% prior to installa	to a ma tion.	aximun	n moisture	content									<b>D</b> I (II )	
Plating Natas										Chords	Tens.Comp.	Chords	Tens. (	) Comp.
Connectors in green lu	mber (	a) des	ianed usina							R - Q	514 - 119	P-0	729	- 154
NDS/TPI reduction fac	tors.	3,	.g							Q - P	948 - 191	0 - M	431	- 86
All plates are 3X4 exce	ept as n	oted.										<b>D D</b> <i>(</i>		
Loading										Webs	Tens.Comp.	Webs	DS) Tens. (	Comp.
Bottom chord checked	for 10.	00 psf	non-concu	rrent						S-C	210 - 943	0.6	308	0
Truss designed for un	nalanco	d load		/1 00						C - R	756 - 158	G - M	225	- 991
windward/leeward fact	ors.	u loau	rusing 0.00	/1.00						R-D	104 - 423	M - N	355	- 1359
Purlins						_				F-O	463 - 70 96 - 397	101 - 1	125	- 515
In lieu of structural par	nels use	e purlir	ns to brace	all flat	PRO	FESSION								
TC @ 24" oc.					BEGEL	LT. TAN	<u>[</u> \$]							
Wind					53		E G							
Wind loads based on I	MWFR	S with	additional (	C&C	101		一百							
Right end vertical not e	exposed	d to wi	nd pressure	<u>,</u>	No.	C 72160	· )^	` ))						
Wind loading based or	n both g	able a	and hip roof	types.				//						
Additional Natas	-				<b>\</b> * \		/*/	//						
Shim all supports to so	olid bea	rina			Star 1	CIVIL	NIA /							
					I E OF	CALIFO								
					03	/12/2020								
	**WAI	RNING	** READ	AND FOLL	OW ALL NOTES O	N THIS D	RAWING	!				American	Truss Comp	any, Inc.
**IMPORTA	NT**	FURN in fabr	ISH THIS D	RAWING T	O ALL CONTRAC	rors INC	LUDING	THE INSTAL	LERS atest edition	of BCSI (F	Buildina	4550 Sprir PETALUM	g Hill Road A CA 9495	52
Component Safety Info bracing per BCSI. Unle	rmation ss note	n, by T d othe	PI and SBC erwise top c	A) for safet	y practices prior to ave properly attach	performing ed structur	these fui al sheath	nctions. Insta ing and botto	illers shall pi m chord sha	ovide tem	porary	(707) 763-	3/13 Ext	
attached rigid ceiling. I as applicable. Apply p	ocation	ns sho each	wn for pern face of trus	nanent later s and positi	al restraint of webs on as shown above	shall have and on th	bracing i ne Joint D	nstalled per E etails, unless	SCSI section s noted othe	s B3, B7, rwise. R	or B10, efer to	_		• — [—]
drawings 160A-Z for st	andard V Ruildi	plate plate	positions.	Group Inc. s	hall not be response	ible for an	v deviatio	n from this dr	awing any fo	ilure to bu	uild the	AL		NE
truss in conformance w	ith ANS		1, or for ha	ndling, ship	ping, installation an	d bracing	of trusses	A seal on the	is drawing (	or cover p	bage		A	N ITW COMPANY
and use of this draw	ving fo	or any	structure i	s the respo	s: AL PINE: www.alpin	ilding Des	signer pe		Sec.2.		www.icceafe.org	8801 Fol Sacrame	som Blvd. ento, CA 9	, Suite 107 5826

and use of this drawing for any structure is the responsibility of the Building Designer per ANS/ITPI 1 Sec.2. For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.com; ICC: www.iccsafe.org







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## CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

#### Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

Web Member	Specified CLR	Alternative Reir	iforecement
Size	Restraint	T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2×4	1-2×4
2x3 or 2x4	2 rows	2×6	2-2×4
2x6	1 row	2×4	1-2×6
2x6	2 rows	2×6	2-2×4( <del>X</del> )
2×8	1 row	2×6	1-2×8
2×8	2 rows	2×6	2-2×6( <b>ж</b> )

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.



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# Apine, a division of ITV Bulling Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, Installation & bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Bullding Designer per ANSI/TPI 1 Sec2.

For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.org; ICC: www.lccsafe.org

## 2×4 WEB WITH WEB-BLOCK^o

REFER TO ENGINEER'S SEALED TRUSS DESIGN DRAWING FOR LUMBER, PLATES AND OTHER INFORMATION NOT SHOWN BELOW.

PLATES FASTENING WEB-BLOCK TO WEB SHALL BE W3X3 OR LARGER. PLATE POSITIONING TOLERANCES ON WEB-BLOCKS SHALL BE PERMITTED TO BE VARIED UP TO 6 INCHES FROM GIVEN POSITIONS, EXCEPT THAT PLATES AT THE END OF THE WEB-BLOCK MUST BE WITHIN 6 INCHES OF THE END OF THE WEB-BLOCK.





	COFESSIO:	##varningi## read and follov all notes on this dravingi ##important## furnish this draving to all contractors including the installers.		REF	2x4 Web Block
	SSELL T. TANGA ST	Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid celling. Locations shown for permanent lateral restraint of webs		DATE	10/01/14
				DRWG	BRWEBLOK1014
	<b>ELPINE</b>	shall have bracing installed per BCSI sections 83, 87 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.			
	No. C 7216QN ITW CONPANY	Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping,			
	/*//	A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing			
	13389 Lakefront Drive	for any structure is the responsibility of the bulkardy Designer per ANSU/1911 Sec2. For more information see this job's general notes page and these web sites: ALPINE: www.alphehetw.com: TP: www.toinst.org: SBCA: www.sbcindustrv.org: ICC: www.iccsafe.org			
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### TRULOX INFORMATION DETAIL



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