

Truss Verification Letter

Date:	February 1, 2022
Project:	Vannucci 'Oakridge' 4610 Tre Monte Lane Healdsburg, California 95448
Project Number:	21-12-180
To:	Casey Rogness DC Builders 11251 SE 232 nd Ave. Damascus, OR 97089
From:	J. Jacob Baglien, P.E
Re:	Vannucci Oakridge –Truss Shop Drawings

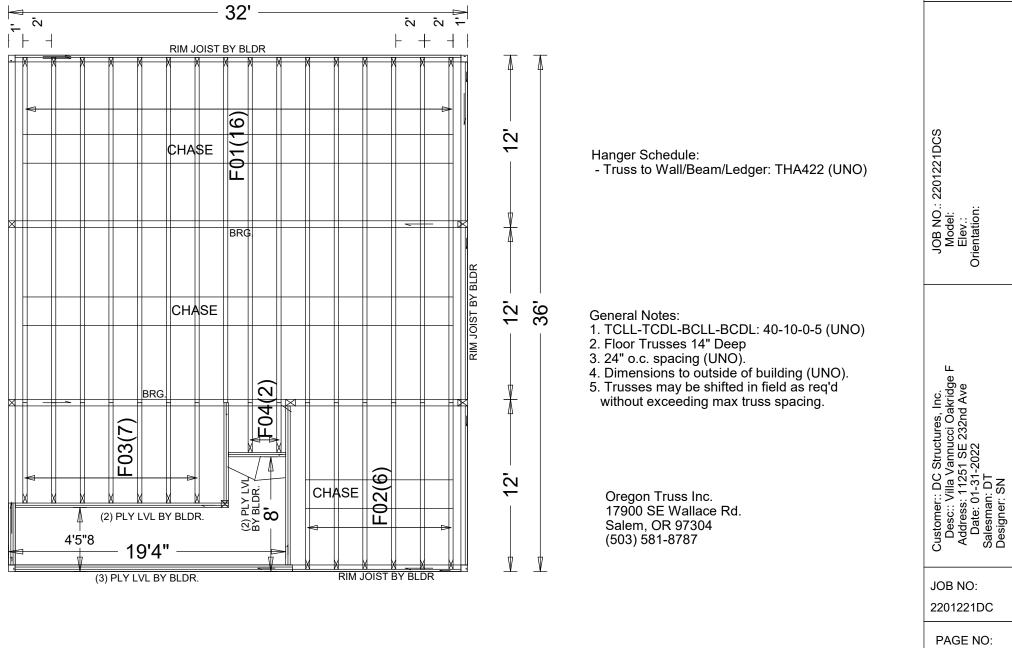
This letter is to certify that I, as the Engineer of Record for the above-listed project and location, have reviewed the attached truss shop drawings and calculations for the prefabricated floor trusses prior to submitting them to the building department and find them to be in general conformance with the plans and specifications (including, but not limited to, connections, design loads, load path, bearing points, etc.).

Project engineer stamp and signature below.



Attachments:

Truss Shop Calculations Truss Shop Layout Drawing Trusses must be braced temporarily and permanently as required for proper installation. Refer to individual truss design drawings, BCSI sheets, and/or building designer for bracing information.



1 OF 1

OREGON TRUSS ... We're Just Better





Alpine, an ITW Company 8801 Folsom Blvd., Suite 107 Sacramento, CA 95826 Phone: (800)877-3678 (916)387-0116 Fax: (916)387-1110 sacseals@itwbcg.com

Page 1:
Job Number: 2201221DCS

Job Engineering Criteria:	
Design Code: IRC 2018	IntelliVIEW Version: 21.02.00B
	JRef #: 1XcO4350010
Wind Standard: ASCE716 Wind Speed (mph): 0	Design Loading (psf): 55.00
Building Type:	

This package contains general notes pages, 4 truss drawing(s) and 1 detail(s).

ltem	Drawing Number	Truss	Item	Drawing Number	Truss
1	031.22.1638.32930	F01	2	031.22.1638.48463	F02
3	031.22.1639.14650	F03	4	031.22.1639.36017	F04
5	STRBRIBR1014				

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer. The Truss Design Engineer. The Truss Design Engineer on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc). -R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

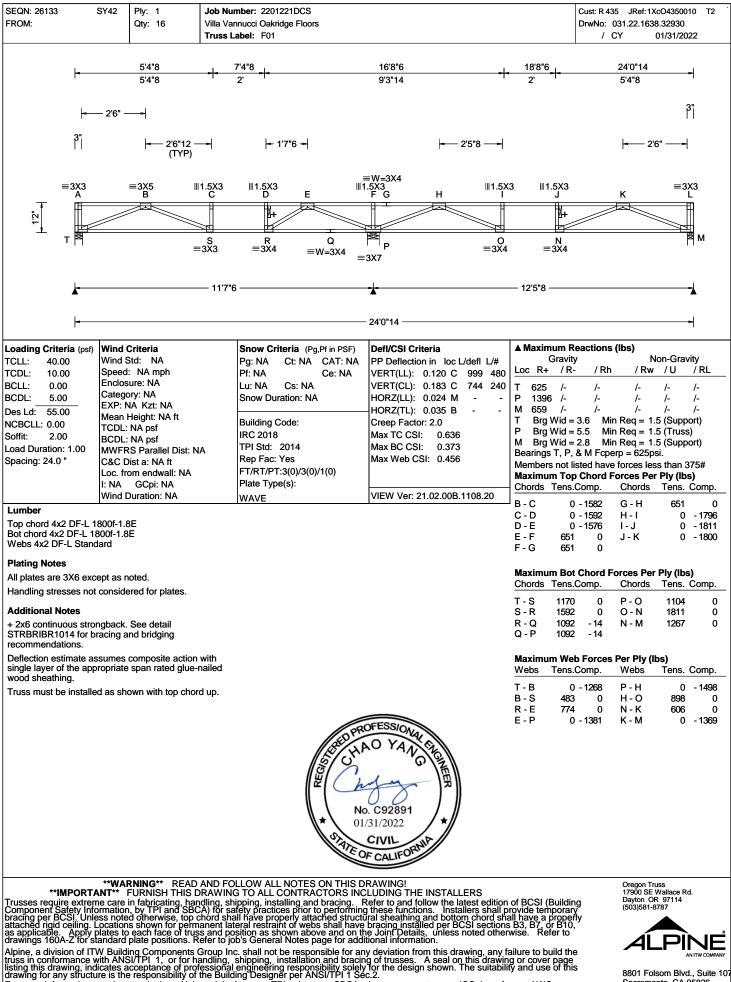
VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment. W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

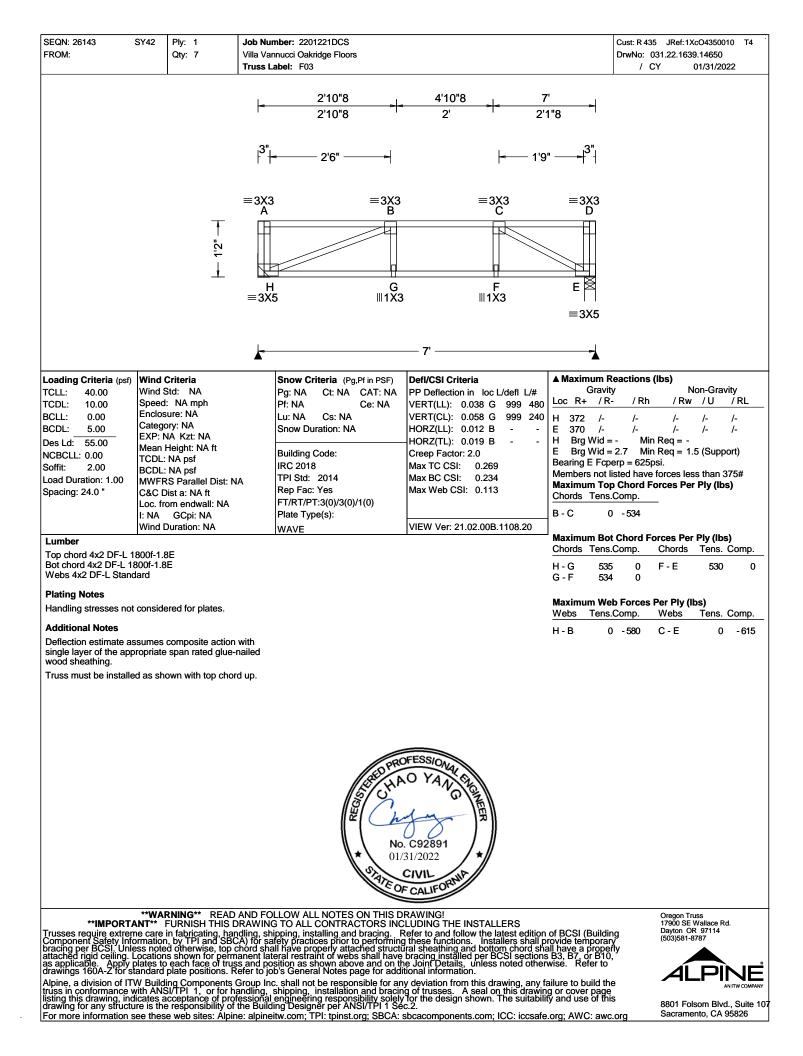
- 1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
- 2. ICC: International Code Council; www.iccsafe.org.
- 3. Alpine, a division of ITW Building Components Group Inc.: 514 Earth City Expressway, Suite 242, Earth City, MO 63045; www.alpineitw.com.
- 4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
- 5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www. sbcacomponents.com.



For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

8801 Folsom Blvd., Suite 107 Sacramento, CA 95826

SEQN: 26135 FROM:	SY42 Ply: 1 Qty: 6	Job Number: 2201221DCS Villa Vannucci Oakridge Floors Truss Label: F02		Cust: R 435 JRef:1XcO4350010 T3 DrwNo: 031.22.1638.48463 / CY 01/31/2022
		4'2"14 4'2"14 <mark>⊳ </mark> ∍	6'2"14 2' ₽	11'7"6 5'4"8
		2'6" 	⊨ 2'6"12	
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	_		— 11'7"6 ———	
Loading Criteria (psf) TCLL: 40.00 TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.00 Spacing: 24.0 "	Wind Criteria Wind Std: NA Speed: NA mph Enclosure: NA Category: NA EXP: NA Kzt: NA Mean Height: NA ft TCDL: NA psf BCDL: NA psf MWFRS Parallel Dist: C&C Dist a: NA ft Loc. from endwall: NA I: NA GCpi: NA Wind Duration: NA	Rep Fac: Yes	A PP Deflection in loc L/defl L/#	
Lumber Top chord 4x2 DF-L ⁻ Bot chord 4x2 DF-L 1 Webs 4x2 DF-L Stan	800f-1.8E			Maximum Bot Chord Forces Per Ply (lbs)ChordsTens.Comp.ChordsTens.Comp.J - 111770H - G11800
Plating Notes Handling stresses no	t considered for plates.			I-H 1600 0 Maximum Web Forces Per Ply (Ibs) Webs Tens.Comp. Webs Tens. Comp.
single layer of the app				Webs Tens. Comp. Webs Tens. Comp. J - B 0 - 1276 H - E 554 0 B - I 624 0 E - G 0 - 1275
wood sheathing. Truss must be installe	ed as shown with top cho	ord up.		
		A CONTRACTOR	No. C92891 01/31/2022	
russes require extrer component Safety Inf racing per BCSI. Unl ttached rigid ceiling. s applicable. Apply rawings 160A-Z for s	ne care in fabricating, ha ormation, by TPI and SE ess noted otherwise, top Locations shown for per plates to each face of tru tandard plate positions.	D AND FOLLOW ALL NOTES ON THIS DRAWING TO ALL CONTRACTORS I andling, shipping, installing and bracing. SCA) for safety practices prior to perform o chord shall have properly attached str. manent lateral restraint of webs shall ha uss and position as shown above and on Refer to job's General Notes page for a s Group Inc. shall not be responsible for bordling a bipping instellation and hre	 Refer to and follow the latest editioning these functions. Installers shall cutural sheathing and bottom chord save bracing installed per BCSI section n the Joint Details, unless noted other dditional information. 	n of BCSI (Building provide temporary hall have a properly rs B3, B7, or B10, erwise. Refer to failure to build the failure to build the
sting this drawing, ind rawing for any struct	dicates acceptance of pr ure is the responsibility of see these web sites: Alp	s Group Inc. shall not be responsible for handling, shipping, installation and bra ofessional engineering responsibility so if the Building Designer per ANSI/TP11 ine: alpineitw.com; TPI: tpinst.org; SBC	lely for the design shown. The suitab Sec.2. A: sbcacomponents.com; ICC: iccsa	ility and use of this ie.org; AWC: awc.org Sacramento, CA 95826



TCLL: 40.00 Wind Std: NA Pg: NA Ct: NA PP Deflection in loc L/defl L/# TCDL: 10.00 Speed: NA mph Pf: NA Ce: NA VERT(LL): 0.001 B 999 480 BCLL: 0.00 Enclosure: NA Lu: NA Cs: NA VERT(CL): 0.002 B 999 240 E 178 /- BCDL: 5.00 Category: NA Lu: NA Cs: NA VERT(CL): 0.001 B - D 175 /- NCBCLL: 0.00 TCDL: NA psf Building Code: Creep Factor: 2.0 D Brg Wid = 2.7 Min Soffit: 2.00 Port NMax TC CSI: 0.230 D Bearing D Fcperp = 625p	EQN: 26141 SY ROM:	Y42 Ply: 1 Qty: 2	Job Number: 2201221DCS Villa Vannucci Oakridge Floors Truss Label: F04		Cust: R 435 JRef:1XcO4350010 T5 DrwNo: 031.22.1639.36017 / CY 01/31/2022
And B C Image: Content of the appropriate span rated give-nalue. Image: Content of the appropriate span rated gi				2'6" <mark>⊰</mark> 3"⊣	
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Top chord 4x2 DF-L 1800f-1.8E Bot chord 4x2 DF-L 1800f-1.8E Webs 4x2 DF-L Standard Plating Notes Handling stresses not considered for plates. Additional Notes Deflection estimate assumes composite action with single layer of the appropriate span rated glue-nailed wood sheathing. Truss must be installed as shown with top chord up.	CLL: 40.00 V CDL: 10.00 S CDL: 0.00 E CDL: 5.00 C es Ld: 55.00 K CBCLL: 0.00 T C offit: 2.00 E C oad Duration: 1.00 K C oacing: 24.0 " L V	Wind Std: NA Speed: NA mph Enclosure: NA Category: NA EXP: NA Kzt: NA Mean Height: NA ft ICDL: NA psf 3CDL: NA psf 3CDL: NA psf WWFRS Parallel Dist: NA C&C Dist a: NA ft 	A Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 TPI Std: 2014 Rep Fac: Yes FT/RT/PT:3(0)/3(0)/1(0) Plate Type(s):	PP Deflection in loc L/defl L/# VERT(LL): 0.001 B 999 480 VERT(CL): 0.002 B 999 240 HORZ(LL): 0.001 B HORZ(TL): 0.001 B Creep Factor: 2.0 Max TC CSI: 0.230 Max BC CSI: 0.062 Max Web CSI: 0.048	Loc R+ / R- / Rh / Rw / U / RL E 178 /- /- /- /- /- D 175 /- /- /- /- /- E Brg Wid = - Min Req = - Min Req = - Min Req = - Min Req = -
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No. C92891 01/31/2022 * OF CALIFORNIA				AO YAN SKOR	

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING! **IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

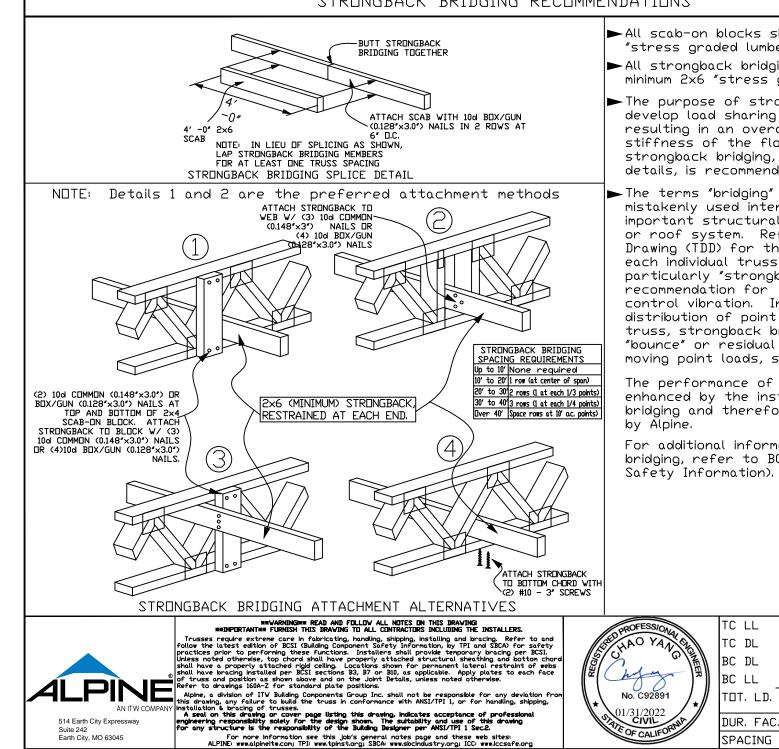
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org





8801 Folsom Blvd., Suite 107 Sacramento, CA 95826

STRONGBACK BRIDGING RECOMMENDATIONS



- ► All scab-on blocks shall be a minimum 2x4 "stress graded lumber."
- ► All strongback bridging and bracing shall be a minimum 2x6 "stress graded lumber."
- ► The purpose of strongback bridging is to develop load sharing between individual trusses, resulting in an overall increase in the stiffness of the floor system. 2x6 strongback bridging, positioned as shown in details, is recommended at 10' - 0'' o.c. (max.)
- ► The terms "bridging" and "bracing" are sometimes mistakenly used interchangeably. "Bracing" is an important structural requirement of any floor or roof system. Refer to the Truss Design Drawing (TDD) for the bracing requirements for each individual truss component. "Bridging," particularly "strongback bridging" is a recommendation for a truss system to help control vibration. In addition to aiding in the distribution of point loads between adjacent truss, strongback bridging serves to reduce "bounce" or residual vibration resulting from moving point loads, such as footsteps.

The performance of all floor systems are enhanced by the installation of strongback bridging and therefore is strongly recommended

For additional information regarding strongback bridging, refer to BCSI (Building Component Safety Information).

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DATE 10/01/14

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