

10/13/2022



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Site Information:	Page 1:
Customer: American Truss Company, Inc.	Job Number: 0954
Job Description: WIGHT ART STUDIO	
Address: 22151 BUFANO COURT, JENNER , CA 95450	

Job Engineering Criteria:	
Design Code: CBC 2019 Res	IntelliVIEW Version: 21.02.01 through 22.02.00 JRef #: 1XJQ8800002
Wind Standard: ASCE 7-16 Wind Speed (mph): 110 Building Type: Closed	Design Loading (psf): 40.00

This package contains general notes pages, 11 truss drawing(s) and 4 detail(s).

Item	Drawing Number	Truss
1	286.22.1249.38883	A
3	286.22.1315.22127	A2
5	286.22.1250.09927	A1B
7	286.22.1315.29000	A4
9	286.22.1315.34950	A5
11	286.22.1315.53330	F2
13	BRCLBSUB0119	
15	GBLLETIN0118	

Item	Drawing Number	Truss
2	286.22.1250.02740	A1
4	286.22.1250.07147	A1A
6	286.22.1315.26720	A3
8	286.22.1315.31500	A4A
10	286.22.1315.38863	F1
12	A11515ENC160118	
14	GABRST160118	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

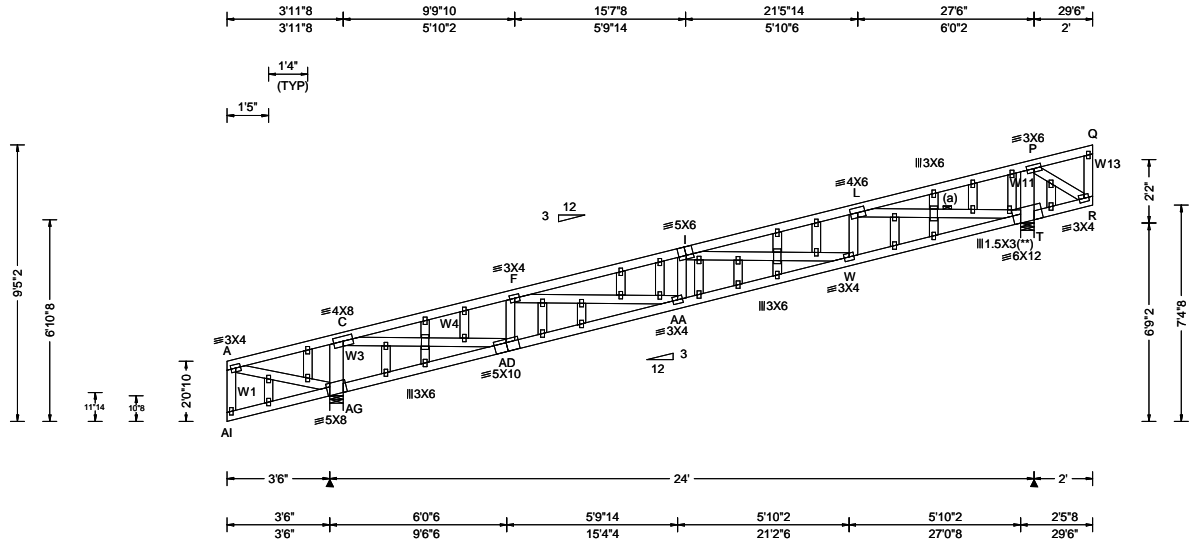
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoclin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com.

SEQN: 2630 FROM:	GABL Ply: 1 Qty: 1	Job Number: 0954 WIGHT ART STUDIO Truss Label: A	Cust: R 880 JRef: 1XJQ8800002 T7 DrwNo: 286.22.1249.38883 / RTT 10/12/2022
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 110 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 6.0 psf BCDL: 6.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.25	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: CBC 2019 Res TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.211 Y 999 360 VERT(CL): 0.437 Y 646 240 HORZ(LL): 0.065 X - - HORZ(TL): 0.137 X - - Creep Factor: 2.0 Max TC CSI: 0.321 Max BC CSI: 0.683 Max Web CSI: 0.794 VIEW Ver: 21.02.01.1216.14	Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL AG 1279 - / - / 700 - / 180 T 1134 - / - / 562 / 40 - / - Wind reactions based on MWFRS AG Brg Wid = 5.5 Min Req = 1.5 (Support) T Brg Wid = 5.5 Min Req = 1.5 (Support) Bearings AG & T are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. C - F 334 - 2422 I - L 313 - 2495 F - I 438 - 3265
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Lumber
Top chord 1.5"x3.562" DF-L #2(g)
Bot chord 1.5"x3.562" DF-L #2(g)
Webs 2x4 DF-L Standard(g) W1,
W13 1.5"x3.562" DF-L Standard(g); W3,
W11 2x6 DF-L #2(g); W4 2x4 DF-L #2(g);
Lumber shall be dried to a maximum moisture content of 19% prior to installation.

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
Connectors in green lumber (g) designed using NDS/TPI reduction factors.
All plates are 1.5X3 except as noted.
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading
Truss designed to support 0-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.
Bottom chord checked for 10.00 psf non-concurrent live load.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.

Additional Notes
See DWGS A11515ENC160118, GBLETTIN0118, & GABRST160118 for gable wind bracing and other requirements.
Shim all supports to solid bearing.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
AD-AA	2468 -546	W - T	2436 -404
AA- W	3252 -589		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
AG- C	178 -1120	I - W	176 -757
C -AD	2342 -288	W - L	440 -50
AD- F	119 -580	L - T	408 -2382
F -AA	809 -79		

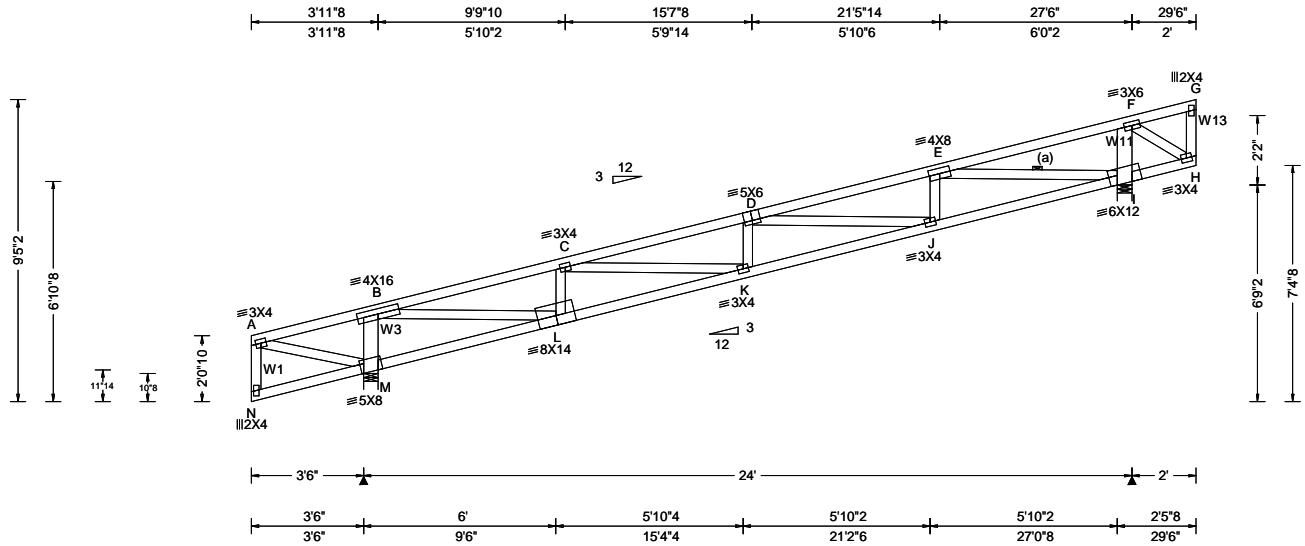


****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

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SEQN: 2607 FROM:	FLAT Qty: 12	Ply: 1	Job Number: 0954 WIGHT ART STUDIO Truss Label: A1	Cust: R 880 JRef: 1XJQ8800002 T1 DrwNo: 286.22.1250.02740 / RTT 10/13/2022
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 110 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 6.0 psf BCDL: 6.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.25	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: CBC 2019 Res TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.213 D 999 360 VERT(CL): 0.441 D 640 240 HORZ(LL): 0.061 J - - HORZ(TL): 0.128 D - - Creep Factor: 2.0 Max TC CSI: 0.403 Max BC CSI: 0.649 Max Web CSI: 0.952 VIEW Ver: 21.02.01.1216.14	▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL M 1274 - / - / /704 - / /180 I 1129 - / - / /562 /40 - / - Wind reactions based on MWFRS M Brg Wid = 5.5 Min Req = 1.5 (Support) I Brg Wid = 5.5 Min Req = 1.5 (Support) Bearings M & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 241 -2455 D - E 208 -2541 C - D 316 -3295
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Lumber

Top chord: 1.5"x3.562" DF-L #2(g);
Bot chord: 1.5"x3.562" DF-L #2(g);
Webs: 2x4 DF-L Standard(g); W1,
W13 1.5"x3.562" DF-L Standard(g); W3,
W11 2x6 DF-L #2(g);

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

Connectors in green lumber (g) designed using NDS/TPI reduction factors.

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.
Truss designed for unbalanced load using 0.00/1.00 windward/leeward factors.

Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.

Additional Notes

Lumber shall be dried to a maximum moisture content of 19% prior to installation.
Shim all supports to solid bearing.



Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
L - K	2503 -457	J - I	2484 -319
K - J	3282 -475		

Maximum Web Forces Per Ply (lbs)

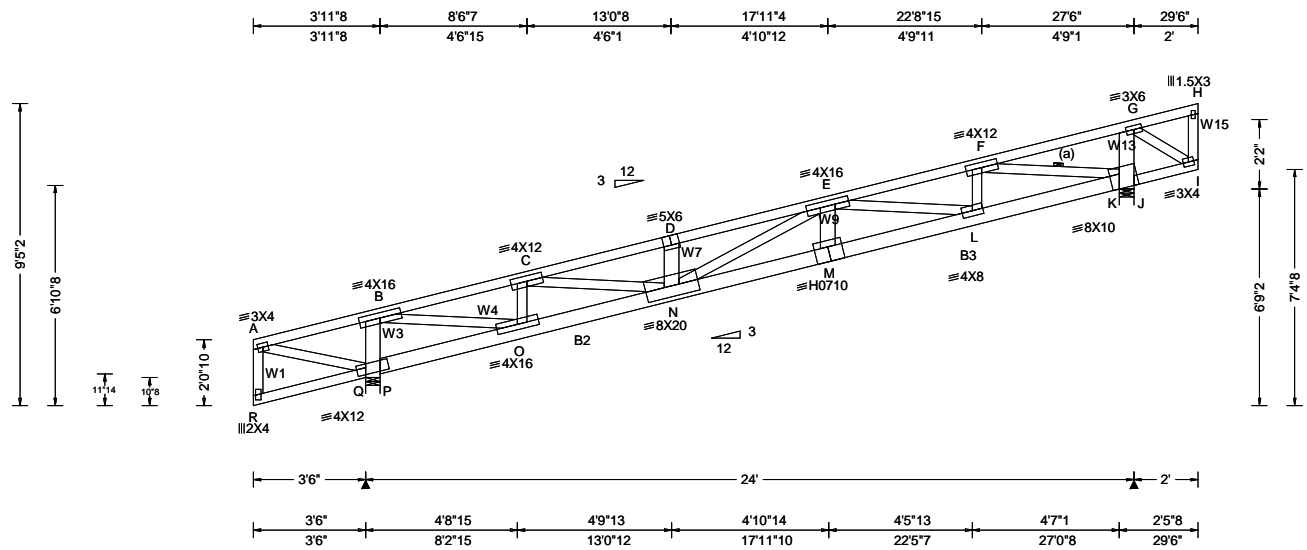
Webs	Tens.Comp.	Webs	Tens. Comp.
M - B	187 -1113	D - J	161 -748
B - L	2342 -199	J - E	412 0
L - C	149 -621	E - I	322 -2401
C - K	798 -58		

****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org

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Loading Criteria (psf) TCLL: 20.00 TC DL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 110 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TC DL: 6.0 psf BCDL: 6.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.25	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: CBC 2019 Res TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.291 N 969 360 VERT(CL): 0.599 N 471 240 HORZ(LL): 0.093 D - - HORZ(TL): 0.192 D - - Creep Factor: 2.0 Max TC CSI: 0.792 Max BC CSI: 0.915 Max Web CSI: 0.908 VIEW Ver: 22.02.00.0914.11	▲ Maximum Reactions (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>Q</td> <td>1723</td> <td>-</td> <td>-</td> <td>700</td> <td>17</td> <td>180</td> </tr> <tr> <td>K</td> <td>1587</td> <td>-</td> <td>-</td> <td>562</td> <td>40</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS Q Brg Wid = 5.5 Min Req = 1.5 (Support) K Brg Wid = 5.5 Min Req = 1.5 (Support) Bearings Q & K are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)</p> <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>220 - 3316</td> <td>D - E</td> <td>363 - 5668</td> </tr> <tr> <td>C - D</td> <td>330 - 5686</td> <td>E - F</td> <td>186 - 3478</td> </tr> </tbody> </table> <p>Maximum Bot Chord Forces Per Ply (lbs)</p> <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>O - N</td> <td>3415 - 437</td> <td>M - L</td> <td>5838 - 447</td> </tr> <tr> <td>N - M</td> <td>5925 - 452</td> <td>L - J</td> <td>3400 - 273</td> </tr> </tbody> </table> <p>Maximum Web Forces Per Ply (lbs)</p> <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Webs</th> <th>Tens.Comp.</th> <th>Webs</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>P - Q</td> <td>117 - 1649</td> <td>E - M</td> <td>581 0</td> </tr> <tr> <td>P - B</td> <td>176 - 1556</td> <td>E - L</td> <td>178 - 2343</td> </tr> <tr> <td>B - O</td> <td>3167 - 171</td> <td>L - F</td> <td>815 0</td> </tr> <tr> <td>O - C</td> <td>133 - 1111</td> <td>F - J</td> <td>280 - 3278</td> </tr> <tr> <td>C - N</td> <td>2234 - 95</td> <td></td> <td></td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	Q	1723	-	-	700	17	180	K	1587	-	-	562	40	-	Chords	Tens.Comp.	Chords	Tens. Comp.	B - C	220 - 3316	D - E	363 - 5668	C - D	330 - 5686	E - F	186 - 3478	Chords	Tens.Comp.	Chords	Tens. Comp.	O - N	3415 - 437	M - L	5838 - 447	N - M	5925 - 452	L - J	3400 - 273	Webs	Tens.Comp.	Webs	Tens. Comp.	P - Q	117 - 1649	E - M	581 0	P - B	176 - 1556	E - L	178 - 2343	B - O	3167 - 171	L - F	815 0	O - C	133 - 1111	F - J	280 - 3278	C - N	2234 - 95		
Loc	Gravity			Non-Gravity																																																																											
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Q	1723	-	-	700	17	180																																																																									
K	1587	-	-	562	40	-																																																																									
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N - M	5925 - 452	L - J	3400 - 273																																																																												
Webs	Tens.Comp.	Webs	Tens. Comp.																																																																												
P - Q	117 - 1649	E - M	581 0																																																																												
P - B	176 - 1556	E - L	178 - 2343																																																																												
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O - C	133 - 1111	F - J	280 - 3278																																																																												
C - N	2234 - 95																																																																														

Lumber
 Top chord: 1.5"x3.562" DF-L #2(g);
 Bot chord: 1.5"x3.562" DF-L #2(g); B2 2x6 DF-L SS(g);
 B3 2x6 DF-L #2(g);
 Webs: 2x4 DF-L Standard(g); W1,
 W15 1.5"x3.562" DF-L Standard(g); W3,W7,W9,
 W13 2x6 DF-L #2(g); W4 2x4 DF-L #2(g);

Wind
 Wind loads based on MWFRS with additional C&C member design.
 End verticals not exposed to wind pressure.
 Left and right cantilevers are exposed to wind
 Wind loading based on both gable and hip roof types.

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Special Loads
 -----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 61 plf at 0.00 to 61 plf at 29.50
 BC: From 21 plf at 0.00 to 21 plf at 29.50
 BC: 450 lb Conc. Load at 13.29,17.94

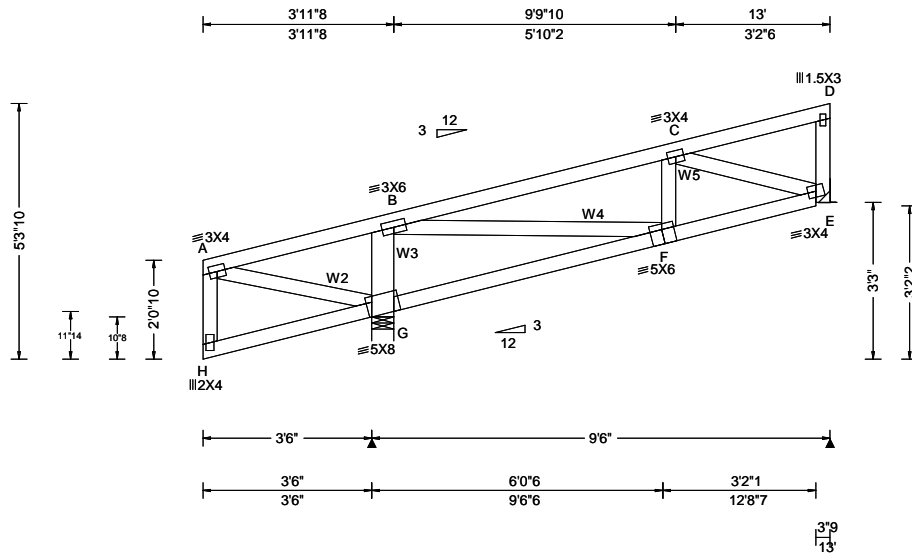
Additional Notes
 Lumber shall be dried to a maximum moisture content of 19% prior to installation.
 Shim all supports to solid bearing.

Plating Notes
 Connectors in green lumber (g) designed using NDS/TPI reduction factors.
 (**) 4 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading
 Bottom chord checked for 10.00 psf non-concurrent live load.
 Truss designed for unbalanced load using 0.00/1.00 windward/leeward factors.



SEQN: 2612 FROM:	FLAT Ply: 1 Qty: 1	Job Number: 0954 WIGHT ART STUDIO Truss Label: A1A	Cust: R 880 JRef: 1XJQ8800002 T4 DrwNo: 286.22.1250.07147 / RTT 10/13/2022
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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 110 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 6.0 psf BCDL: 6.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.25	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: CBC 2019 Res TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.009 F 999 360 VERT(CL): 0.017 H 999 240 HORZ(LL): -0.003 D - - HORZ(TL): 0.006 D - - Creep Factor: 2.0 Max TC CSI: 0.312 Max BC CSI: 0.224 Max Web CSI: 0.249 VIEW Ver: 21.02.01.1216.14	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>G</td> <td>740</td> <td>-</td> <td>-</td> <td>/429</td> <td>/16</td> <td>/82</td> </tr> <tr> <td>E</td> <td>340</td> <td>-</td> <td>-</td> <td>/161</td> <td>-</td> <td>-</td> </tr> </tbody> </table>						Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	G	740	-	-	/429	/16	/82	E	340	-	-	/161	-	-
				Loc	Gravity			Non-Gravity																												
R+	/R-	/Rh	/Rw		/U	/RL																														
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				Wind reactions based on MWFRS G Brg Wid = 5.5 Min Req = 1.5 (Support) E Brg Wid = - Min Req = - Bearing G is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th colspan="2"></th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>89</td> <td>-</td> <td>547</td> </tr> </tbody> </table>						Chords	Tens.Comp.			B - C	89	-	547																			
Chords	Tens.Comp.																																			
B - C	89	-	547																																	

Lumber
 Top chord: 1.5"x3.562" DF-L #2(g);
 Bot chord: 1.5"x3.562" DF-L #2(g);
 Webs: 1.5"x3.562" DF-L Standard(g); W2,W4,
 W5 2x4 DF-L Standard(g); W3 2x6 DF-L #2(g);

Plating Notes
 Connectors in green lumber (g) designed using
 NDS/TPI reduction factors.

Loading
 Bottom chord checked for 10.00 psf non-concurrent
 live load.
 Truss designed for unbalanced load using 0.00/1.00
 windward/leeward factors.

Wind
 Wind loads based on MWFRS with additional C&C
 member design.
 End verticals not exposed to wind pressure.
 Left cantilever is exposed to wind
 Wind loading based on both gable and hip roof types.

Additional Notes
 Lumber shall be dried to a maximum moisture content
 of 19% prior to installation.
 Shim all supports to solid bearing.



Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.		
F - E	514	-	162

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.	
G - B	197	-556	C - E	155 -513
B - F	612	-73		

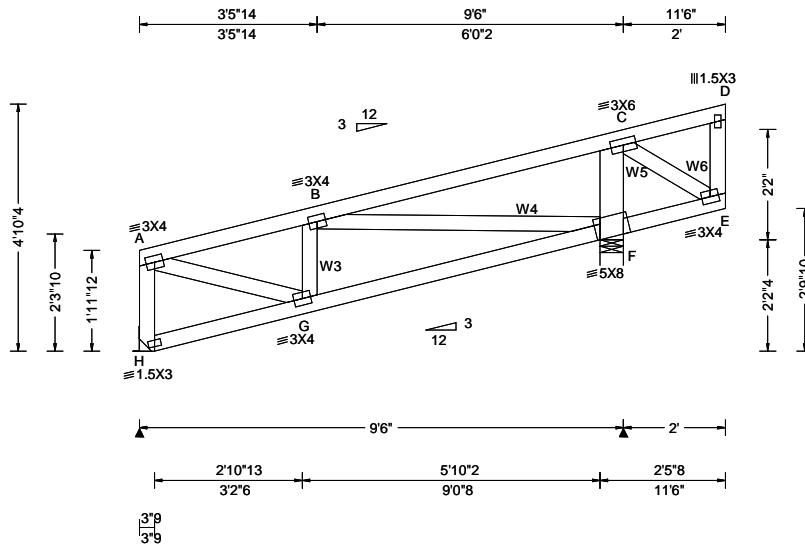
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SEQN: 2614 FROM:	FLAT Ply: 1 Qty: 1	Job Number: 0954 WIGHT ART STUDIO Truss Label: A1B	Cust: R 880 JRef: 1XJQ8800002 T8 DrwNo: 286.22.1250.09927 / RTT 10/13/2022
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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 110 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.32 ft TCCL: 6.0 psf BCDL: 6.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.25	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: CBC 2019 Res TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.010 B 999 360 VERT(CL): 0.021 B 999 240 HORZ(LL): 0.005 B - - HORZ(TL): 0.009 B - - Creep Factor: 2.0 Max TC CSI: 0.265 Max BC CSI: 0.303 Max Web CSI: 0.337 VIEW Ver: 21.02.01.1216.14	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>H</td> <td>359</td> <td>-</td> <td>-</td> <td>/178</td> <td>-</td> <td>/74</td> </tr> <tr> <td>F</td> <td>570</td> <td>-</td> <td>-</td> <td>/279</td> <td>/34</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS H Brg Wid = - Min Req = - F Brg Wid = 5.5 Min Req = 1.5 (Support) Bearing F is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> </tr> </thead> <tbody> <tr> <td>A - B</td> <td>108 -558</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	H	359	-	-	/178	-	/74	F	570	-	-	/279	/34	-	Chords	Tens.Comp.	A - B	108 -558
Loc	Gravity			Non-Gravity																															
	R+	/R-	/Rh	/Rw	/U	/RL																													
H	359	-	-	/178	-	/74																													
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Chords	Tens.Comp.																																		
A - B	108 -558																																		

Lumber
 Top chord: 1.5"x3.562" DF-L #2(g);
 Bot chord: 1.5"x3.562" DF-L #2(g);
 Webs: 1.5"x3.562" DF-L Standard(g); W3,W4,
 W6 2x4 DF-L Standard(g); W5 2x6 DF-L #2(g);

Plating Notes
 Connectors in green lumber (g) designed using
 NDS/TPI reduction factors.

Loading
 Bottom chord checked for 10.00 psf non-concurrent
 live load.
 Truss designed for unbalanced load using 0.00/1.00
 windward/leeward factors.

Wind
 Wind loads based on MWFRS with additional C&C
 member design.
 End verticals not exposed to wind pressure.
 Right cantilever is exposed to wind
 Wind loading based on both gable and hip roof types.

Additional Notes
 Lumber shall be dried to a maximum moisture content
 of 19% prior to installation.
 Shim all supports to solid bearing.



Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.
G - F	580 -245

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
A - G	545	-88	B - F 235 -562

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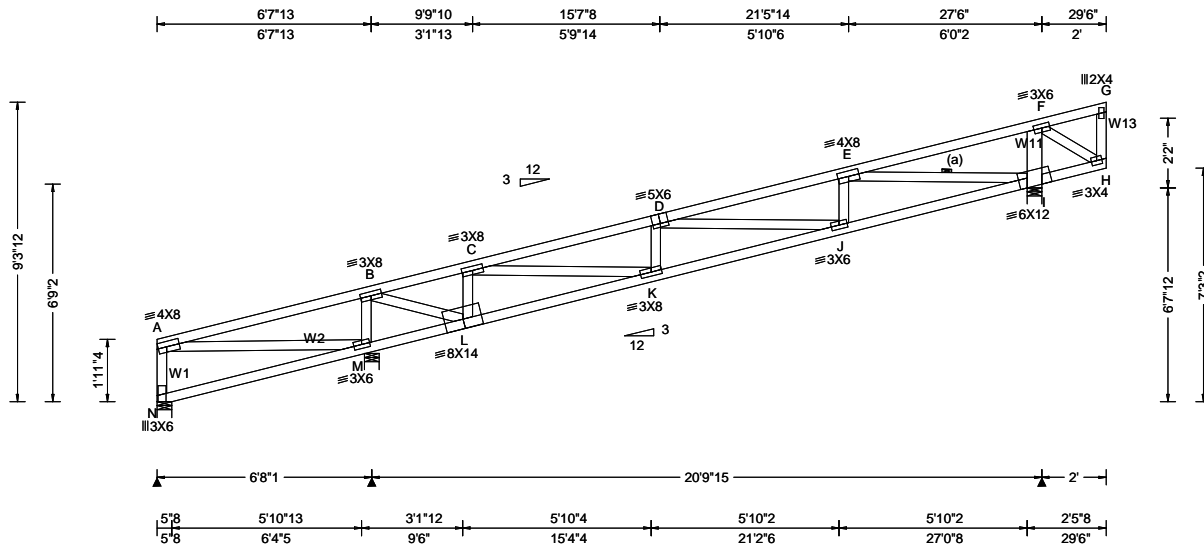
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 Glenview, IL 60025

SEQN: 3061 FROM:	FLAT Ply: 1 Qty: 1	Job Number: 0954 WIGHT ART STUDIO Truss Label: A4	Cust: R 880 JRRef: 1XJQ880002 T5 DrwNo: 286.22.1315.29000 / RTT 10/13/2022
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 110 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 6.0 psf BCDL: 6.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.25	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: CBC 2019 Res TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.101 D 999 360 VERT(CL): 0.207 D 999 240 HORZ(LL): 0.030 J - - HORZ(TL): 0.061 J - - Creep Factor: 2.0 Max TC CSI: 0.627 Max BC CSI: 0.444 Max Web CSI: 0.861 VIEW Ver: 21.02.01.1216.14	▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL N 63 /-228 /- /65 /67 /184 M 1586 /- /- /818 /55 /- I 928 /- /- /458 /29 /- Wind reactions based on MWFRS N Brg Wid = 5.5 Min Req = 1.5 (Truss) M Brg Wid = 5.5 Min Req = 1.5 (Truss) I Brg Wid = 5.5 Min Req = 1.5 (Support) Bearings N, M, & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
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Lumber
Top chord: 1.5"x3.562" DF-L #2(g);
Bot chord: 1.5"x3.562" DF-L #2(g);
Webs: 2x4 DF-L Standard(g); W1,W2,
W13 1.5"x3.562" DF-L Standard(g);
W11 2x6 DF-L #2(g);

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
Connectors in green lumber (g) designed using NDS/TPI reduction factors.

Loading
Bottom chord checked for 10.00 psf non-concurrent live load.
Truss designed for unbalanced load using 0.00/1.00 windward/leeward factors.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Right cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

Additional Notes
Negative reaction(s) of -228# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.
Lumber shall be dried to a maximum moisture content of 19% prior to installation.
Shim all supports to solid bearing.

Maximum Bot Chord Forces Per Ply (lbs)			
Chords	Tens.Comp.	Chords	Tens. Comp.
M - L	0 - 1131	J - I	1797 - 247
K - J	1880 - 296		
Maximum Web Forces Per Ply (lbs)			
Webs	Tens.Comp.	Webs	Tens. Comp.
A - M	231 - 1215	L - C	115 - 747
M - B	203 - 1165	C - K	1475 - 116
B - L	1405 - 112	E - I	230 - 1725



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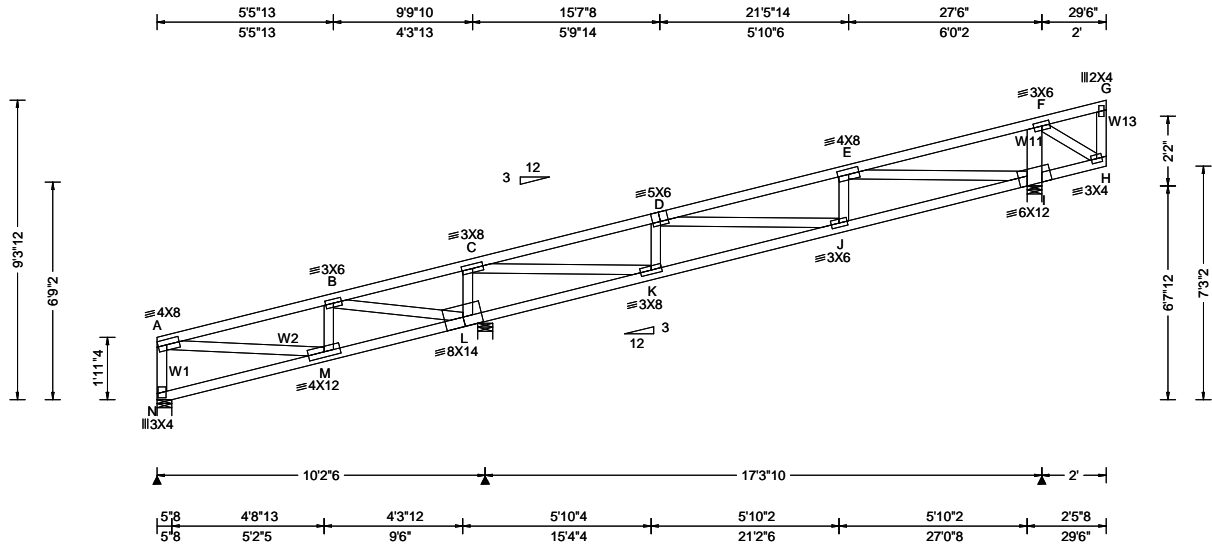
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SEQN: 3063 FROM:	FLAT Ply: 1 Qty: 1	Job Number: 0954 WIGHT ART STUDIO Truss Label: A4A	Cust: R 880 JRef: 1XJQ8800002 T6 DrwNo: 286.22.1315.31500 / RTT 10/13/2022
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Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 110 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 6.0 psf BCDL: 6.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.25	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: CBC 2019 Res TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.085 D 999 360 VERT(CL): 0.174 D 999 240 HORZ(LL): 0.026 J - - HORZ(TL): 0.053 J - - Creep Factor: 2.0 Max TC CSI: 0.310 Max BC CSI: 0.645 Max Web CSI: 0.878 VIEW Ver: 21.02.01.1216.14	▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL N 375 - / - / 163 - / 184 L 1216 - / - / 632 /38 - / I 843 - / - / 414 /28 - / Wind reactions based on MWFRS N Brg Wid = 5.5 Min Req = 1.5 (Truss) L Brg Wid = 5.5 Min Req = 1.5 (Truss) I Brg Wid = 5.5 Min Req = 1.5 (Support) Bearings N, L, & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
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Lumber
Top chord: 1.5"x3.562" DF-L #2(g);
Bot chord: 1.5"x3.562" DF-L #2(g);
Webs: 2x4 DF-L Standard(g); W1,
W13 1.5"x3.562" DF-L Standard(g);
W2 2x4 DF-L #2(g); W11 2x6 DF-L #2(g);

Plating Notes
Connectors in green lumber (g) designed using NDS/TPI reduction factors.

Loading
Bottom chord checked for 10.00 psf non-concurrent live load.
Truss designed for unbalanced load using 0.00/1.00 windward/leeward factors.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Right cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

Additional Notes
Lumber shall be dried to a maximum moisture content of 19% prior to installation.
Shim all supports to solid bearing.

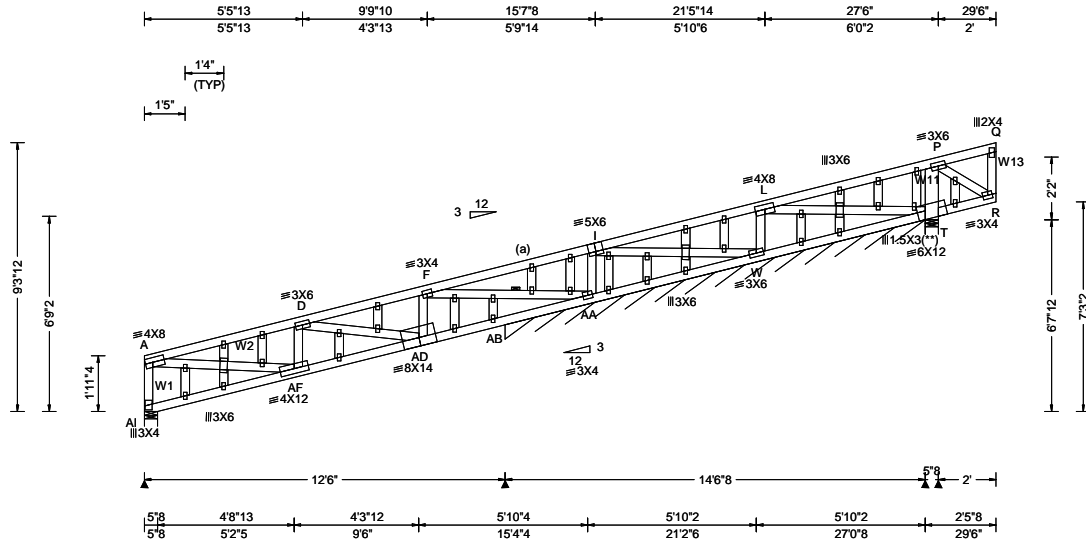


A - B	27	-648	C - D	81	-1216
B - C	413	-100	D - E	121	-1560
Maximum Bot Chord Forces Per Ply (lbs)					
M - L	613	-255	K - J	1251	-232
L - K	81	-497	J - I	1530	-221
Maximum Web Forces Per Ply (lbs)					
A - M	580	0	C - K	1436	-101
B - L	155	-847	K - D	123	-455
L - C	141	-771	E - I	206	-1465

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Glenview, IL 60025

SEQN: 2632 FROM:	GABL Ply: 1 Qty: 1	Job Number: 0954 WIGHT ART STUDIO Truss Label: A5	Cust: R 880 JRef: 1XJQ8800002 T3 DrwNo: 286.22.1315.34950 / RTT 10/13/2022
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 110 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 6.0 psf BCDL: 6.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.25	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: CBC 2019 Res TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.040 E 999 360 VERT(CL): 0.081 E 999 240 HORZ(LL): 0.015 B - - HORZ(TL): 0.030 B - - Creep Factor: 2.0 Max TC CSI: 0.176 Max BC CSI: 0.288 Max Web CSI: 0.684 VIEW Ver: 21.02.01.1216.14	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL AI 514 - / - / 254 - / 184 AB*111 - / - / 157 / 3 - T 306 - / - / 145 / 13 - Wind reactions based on MWFRS AI Brg Wid = 5.5 Min Req = 1.5 (Truss) AB Brg Wid = 174 Min Req = - T Brg Wid = 5.5 Min Req = 1.5 (Support) Bearings AI, AB, & T are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
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Lumber
Top chord: 1.5"x3.562" DF-L #2(g);
Bot chord: 1.5"x3.562" DF-L #2(g);
Webs: 2x4 DF-L Standard(g); W1,
W13 1.5"x3.562" DF-L Standard(g);
W2 2x4 DF-L #2(g); W11 2x6 DF-L #2(g);

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
Connectors in green lumber (g) designed using NDS/TPI reduction factors.
All plates are 1.5X3 except as noted.
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading
Truss designed to support 0-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.
Bottom chord checked for 10.00 psf non-concurrent live load.
Truss designed for unbalanced load using 0.00/1.00 windward/leeward factors.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Right cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

Additional Notes
Lumber shall be dried to a maximum moisture content of 19% prior to installation.
See DWGS A11515ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.
Shim all supports to solid bearing.

A - D	90	- 1031	F - I	472	- 163
D - F	38	- 734			
Maximum Bot Chord Forces Per Ply (lbs)					
AF-AD	1023	- 302	AA- W	4	- 427
AD-AA	1415	- 431			
Maximum Web Forces Per Ply (lbs)					
Webs	Tens.Comp.	Webs	Tens. Comp.		
A - AI	50	- 463	F - AA	205	- 1101
A - AF	966	- 52			



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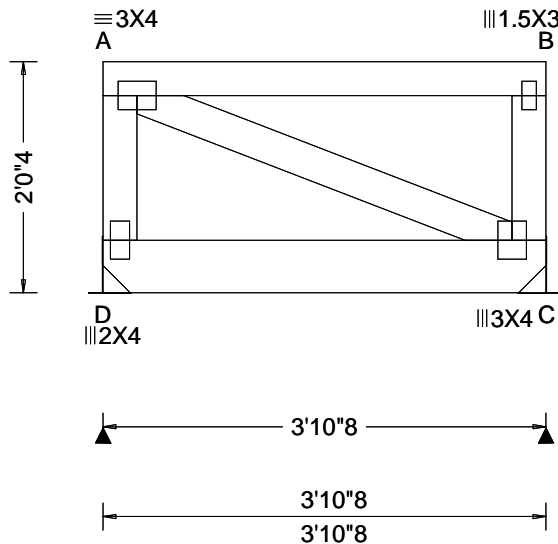
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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

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ALPINE
AN ITW COMPANY
155 Harlem Ave
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Glenview, IL 60025

SEQN: 2620 FROM:	FLAT Ply: 1 Qty: 1	Job Number: 0954 WIGHT ART STUDIO Truss Label: F1	Cust: R 880 JRef: 1XJQ8800002 T9 DrwNo: 286.22.1315.38863 / RTT 10/13/2022
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Loading Criteria (psf) TCLL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 110 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 6.0 psf BCDL: 6.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 10.50 ft GCp: 0.18 Wind Duration: 1.25	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: CBC 2019 Res TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.000 B 999 360 VERT(CL): 0.001 B 999 240 HORZ(LL): -0.000 B - - HORZ(TL): 0.000 B - - Creep Factor: 2.0 Max TC CSI: 0.429 Max BC CSI: 0.334 Max Web CSI: 0.101 VIEW Ver: 21.02.01.1216.14	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL D 427 /- /- /17 /2 /- C 441 /- /- /20 /2 /- Wind reactions based on MWFRS D Brg Wid = - Min Req = - C Brg Wid = - Min Req = - Members not listed have forces less than 375#
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Lumber
 Top chord: 1.5"x3.562" DF-L #2(g);
 Bot chord: 2x6 DF-L #2(g);
 Webs: 1.5"x3.562" DF-L Standard(g);

Special Loads
 ----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 60 plf at 0.00 to 60 plf at 0.25
 TC: From 120 plf at 0.25 to 120 plf at 3.88
 BC: From 20 plf at 0.00 to 20 plf at 3.88
 BC: 340 lb Conc. Load at 1.94

Plating Notes
 Connectors in green lumber (g) designed using NDS/TPI reduction factors.

Loading
 Bottom chord checked for 10.00 psf non-concurrent live load.

Purlins
 The TC of this truss shall be braced with attached spans at 24" oc in lieu of structural sheathing.

Wind
 Wind loads and reactions based on MWFRS.
 End verticals not exposed to wind pressure.

Additional Notes
 Lumber shall be dried to a maximum moisture content of 19% prior to installation.
 Truss must be installed as shown with top chord up.



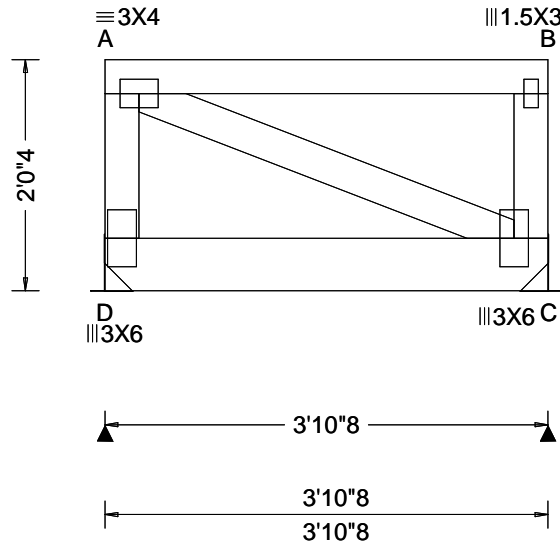
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ALPINE
 AN ITW COMPANY
 155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025

SEQN: 2618 FROM:	FLAT Ply: 1 Qty: 1	Job Number: 0954 WIGHT ART STUDIO Truss Label: F2	Cust: R 880 JRef: 1XJQ8800002 T11 DrwNo: 286.22.1315.53330 / RTT 10/13/2022
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 110 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 6.0 psf BCDL: 6.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 10.50 ft GCp: 0.18 Wind Duration: 1.25	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: CBC 2019 Res TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.000 B 999 360 VERT(CL): 0.001 B 999 240 HORZ(LL): -0.000 B - - HORZ(TL): 0.000 B - - Creep Factor: 2.0 Max TC CSI: 0.431 Max BC CSI: 0.343 Max Web CSI: 0.102 VIEW Ver: 21.02.01.1216.14	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL D 446 /- /- /79 /3 /- C 446 /- /- /79 /3 /- Wind reactions based on MWFRS D Brg Wid = - Min Req = - C Brg Wid = - Min Req = - Members not listed have forces less than 375#
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Lumber

Top chord: 1.5"x3.562" DF-L #2(g);
Bot chord: 2x6 DF-L #2(g);
Webs: 1.5"x3.562" DF-L Standard(g);

Additional Notes

Lumber shall be dried to a maximum moisture content of 19% prior to installation.
Truss must be installed as shown with top chord up.

Special Loads

----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 120 plf at 0.00 to 120 plf at 3.88
BC: From 20 plf at 0.00 to 20 plf at 3.88
BC: 350 lb Conc. Load at 1.94

Plating Notes

Connectors in green lumber (g) designed using NDS/TPI reduction factors.

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.
Truss designed for unbalanced load using 0.00/1.00 windward/leeward factors.

Purlins

The TC of this truss shall be braced with attached spans at 24" oc in lieu of structural sheathing.

Wind

Wind loads based on MWFRS.
End verticals not exposed to wind pressure.



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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

Gable Stud Reinforcement Detail
 ASCE 7-16: 115 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, $K_{zt} = 1.00$

4' 11"	8' 5"	8' 9"	9' 11"	10' 4"	11' 10"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"
4' 8"	8' 1"	8' 7"	9' 10"	10' 3"	11' 9"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
4' 8"	8' 0"	8' 6"	9' 10"	10' 3"	11' 9"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
4' 8"	6' 11"	7' 4"	9' 2"	9' 10"	11' 9"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
5' 2"	8' 7"	8' 10"	10' 1"	10' 6"	12' 0"	12' 5"	14' 0"	14' 0"	14' 0"	14' 0"
4' 11"	8' 5"	8' 9"	9' 11"	10' 4"	11' 10"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"
4' 10"	7' 3"	7' 9"	9' 8"	10' 3"	11' 9"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"
4' 10"	7' 3"	7' 9"	9' 8"	10' 3"	11' 9"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"
4' 8"	6' 5"	6' 10"	8' 7"	9' 2"	11' 7"	12' 2"	13' 5"	14' 0"	14' 0"	14' 0"
5' 8"	9' 8"	10' 0"	11' 5"	11' 10"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
5' 5"	9' 6"	10' 0"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
5' 5"	9' 6"	9' 10"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
5' 5"	8' 5"	9' 0"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
5' 11"	9' 9"	10' 2"	11' 6"	12' 0"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
5' 8"	9' 8"	10' 0"	11' 5"	11' 10"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
5' 6"	8' 11"	9' 6"	11' 4"	11' 9"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
5' 6"	8' 11"	9' 6"	11' 4"	11' 9"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
5' 5"	7' 10"	8' 4"	10' 6"	11' 2"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
6' 3"	10' 7"	11' 0"	12' 7"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
5' 11"	10' 6"	10' 10"	12' 5"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
5' 11"	10' 6"	10' 10"	12' 5"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
5' 11"	9' 9"	10' 5"	12' 5"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
6' 6"	10' 9"	11' 12"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
6' 3"	10' 7"	11' 0"	12' 7"	13' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
6' 1"	10' 3"	10' 11"	12' 6"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
6' 1"	10' 3"	10' 11"	12' 6"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
5' 11"	9' 1"	9' 8"	12' 1"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:	
Group A:	
Spruce-Pine-Fir Standard Stud	Heu-Fir Stud Standard
Douglas Fir-Larch Stud Standard	Southern Pine Stud Standard
Group B:	
Heu-Fir	
Douglas Fir-Larch	Southern Pine

1x4 Braces shall be S2P (Stress-Rated Board).
 For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:
 Wind Load deflection criterion is L/240.
 Provide uplift connections for 30 psf over continuous bearing (5 psf IC Dead Load).
 Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12' plywood overhang.

Attach 'L' braces with 10d (0.128" x 3.0" min) nails.
 For (1) 'L' brace: space nails at 2' o.c. in 18' end zones and 4' o.c. between zones.
 For (2) 'L' braces: space nails at 3' o.c. in 18' end zones and 6' o.c. between zones.
 'L' bracing must be a minimum of 80% of web member length.



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 For more information see this job's general notes page and these web sites:
 ALPINE: www.alpineitw.com; IPI: www.ipi.org; SPCA: www.spcainst.org; ICC: www.iccsafe.org



MAN. TOT. L.D. 60 PSF
 MAN. SPACING 24.0'

REF	ASCE7-16-GAR11515
DATE	01/26/2018
DRWG	A11515ENC160118

Refer to the Building Designer for conditions not addressed by this detail.

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

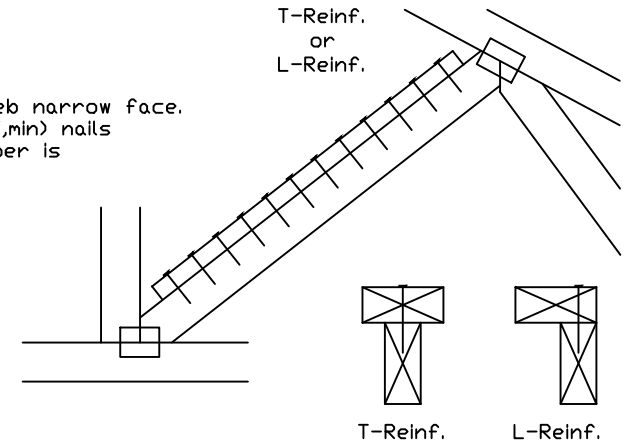
Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6 or 2x4	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

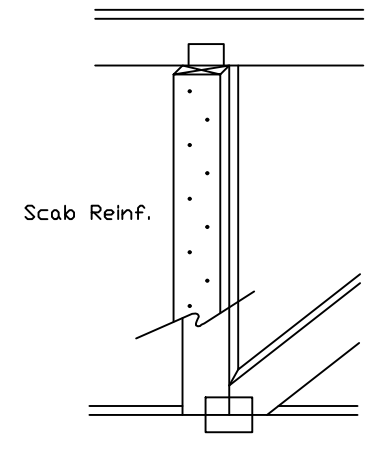
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



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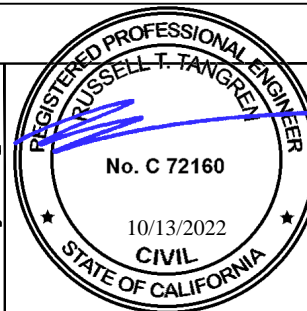
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For more information see this Job's general notes page and these web sites:
ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org



TC LL	PSF	REF CLR Subst.
TC DL	PSF	DATE 01/02/19
BC DL	PSF	DRWG BRCLBSUB0119
BC LL	PSF	
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

ASCE 7-16: 120 mph, 30' Mean Height, Closed, Exposure C Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30ft. Mean Hgt, ASCE 7-16, Enclosed, Exp C, or
100 mph, 30ft. Mean Hgt, ASCE 7-16, Enclosed, Exp D, or
100 mph, 30ft. Mean Hgt, ASCE 7-16, Part. Enclosed, Exp C,
Kzt = 1.00, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf.

Lateral chord bracing requirements
Top: Continuous roof sheathing
Bot: Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail for lumber, plates, and other information not shown on this detail.

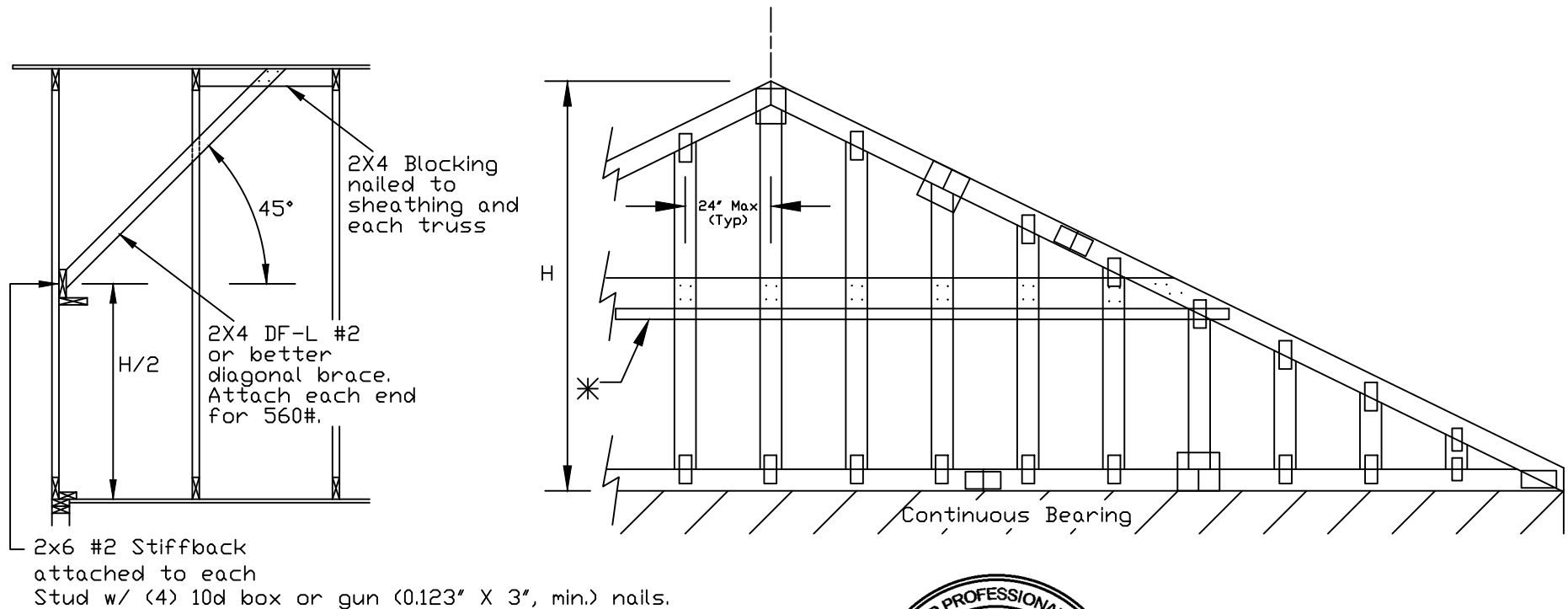
Nails: 10d box or gun (0.128"x3",min) nails.

H Less than 4'6" - no stud bracing required

H Greater than 4'6" to 7'6" in length
provide a 2x6 stiffback at mid-height and brace stiffback to roof diaphragm every 6'0" (see detail below or refer to DRWG A12030ENC160118).

H Greater than 7'6" to 12'0" max:
provide a 2x6 stiffback at mid-height and brace to roof diaphragm every 4'0" (see detail below or refer to DRWG A12030ENC160118).

* Optional 2x L-reinforcement attached to stiffback with 10d box or gun (0.128" x 3", min.) nails @ 6" o.c.



WARNING: READ AND FOLLOW ALL NOTES ON THIS DRAWING
IMPORTANT: FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites:
ALPINE: www.alpineitw.com; TPI: www.tpinet.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

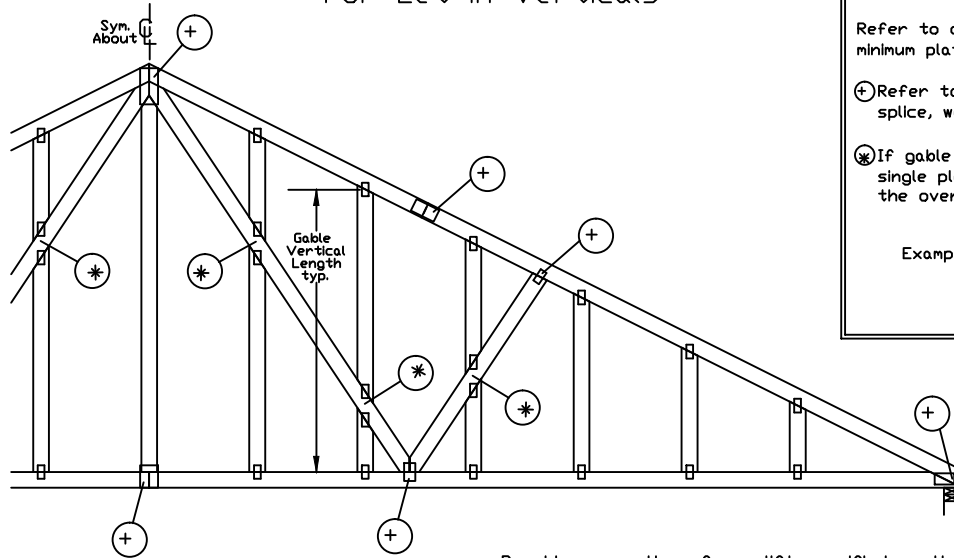


MAX. TOT. LD. 60 PSF

MAX. SPACING

REF	GE WHALER
DATE	01/02/2018
DRWG	GABRST160118

Gable Detail For Let-in Verticals

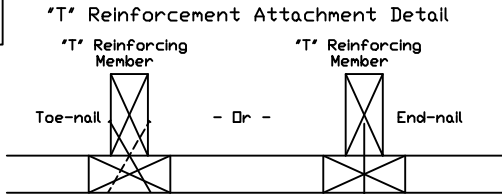


Gable Truss Plate Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

- ⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.
- ⊗ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



Provide connections for uplift specified on the engineered truss design.

Attach each 'T' reinforcing member with
End Driven Nails:
10d Common (0.148"x3",min) Nails at 4' o.c. plus
(4) nails in the top and bottom chords.

Toenailed Nails:
10d Common (0.148"x3",min) Toenails at 4' o.c. plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

- ASCE 7-05 Gable Detail Drawings
A13015051014, A12015051014, A11015051014, A10015051014, A14015051014,
A13030051014, A12030051014, A11030051014, A10030051014, A14030051014
- ASCE 7-10 & ASCE 7-16 Gable Detail Drawings
A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118,
A18015ENC100118, A20015ENC100118, A20015END100118, A20015PED100118,
A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118,
A18030ENC100118, A20030ENC100118, A20030END100118, A20030PED100118,
S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118,
S18015ENC100118, S20015ENC100118, S20015END100118, S20015PED100118,
S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118,
S18030ENC100118, S20030ENC100118, S20030END100118, S20030PED100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.

To convert from 'L' to 'T' reinforcing members, multiply 'T' increase by length (based on appropriate Alpine gable detail).

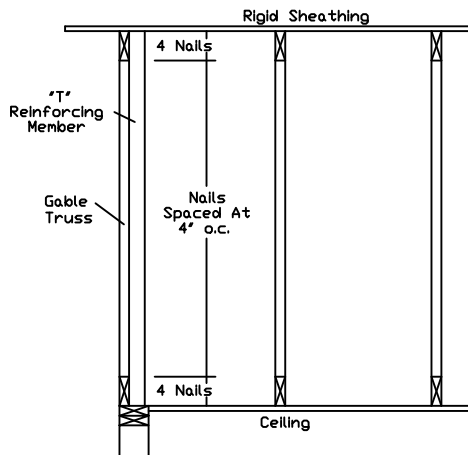
Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord.

'T' reinforcing member material must match size, specie, and grade of the 'L' reinforcing member.

Web Length Increase w/ 'T' Brace

'T' Reinf. Mbr. Size	'T' Increase
2x4	30 %
2x6	20 %

Example:
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft, Kzt = 1.00
Gable Vertical = 24' o.c. SP #3
'T' Reinforcing Member Size = 2x4
'T' Brace Increase (From Above) = 30% = 1.30
(1) 2x4 'L' Brace Length = 8' 7"
Maximum 'T' Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"



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For more information see this Job's general notes page and these web sites:
ALPINE: www.alpneitw.com; TPI: www.tpinstr.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



REF LET-IN VERT
DATE 01/02/2018
DRWG GBLLETIN0118

MAX. TOT. LD. 60 PSF
DUR. FAC. ANY
MAX. SPACING 24.0"