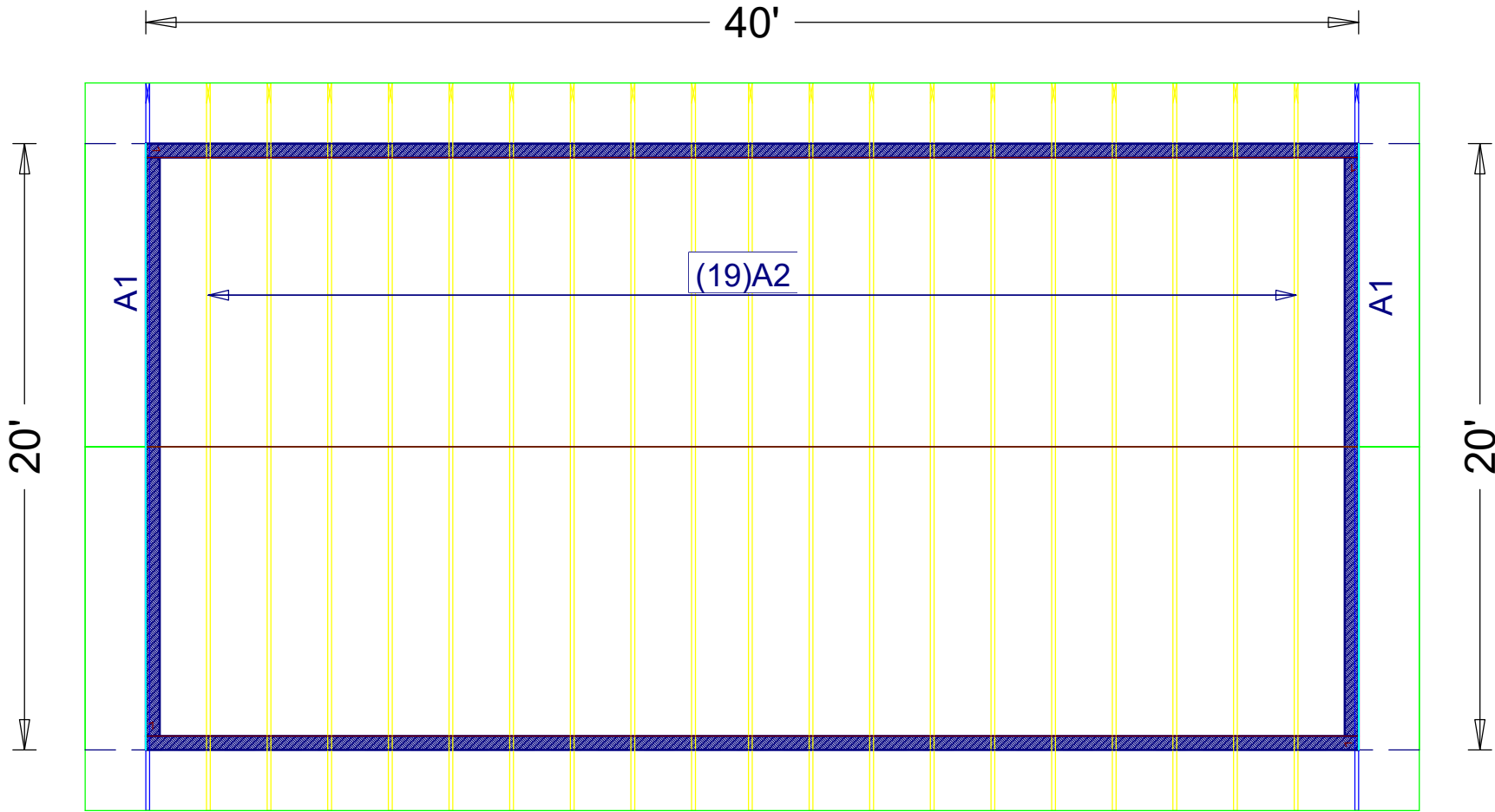


These documents shall remain with the approved set of plans and documents



NO EXCEPTIONS NOTED

Review of these shop drawings does not relieve contractor from compliance with requirements of the drawings and specifications. This review is only for general conformance with the design concept of the project and general compliance with the information given in the contract documents. The contractor is responsible for confirming and correlating all quantities and dimensions, selecting fabrication processes and techniques of construction, coordinating his work with that of all other trades and performing his work in a safe and satisfactory manner.

MKM & ASSOCIATES

Date: 3/21/25 By: Erin L. Dupree

Description: TIMBER COVE GARAGE
Address: 22090 TIMBER COVE ROAD
City: TIMBER COVE

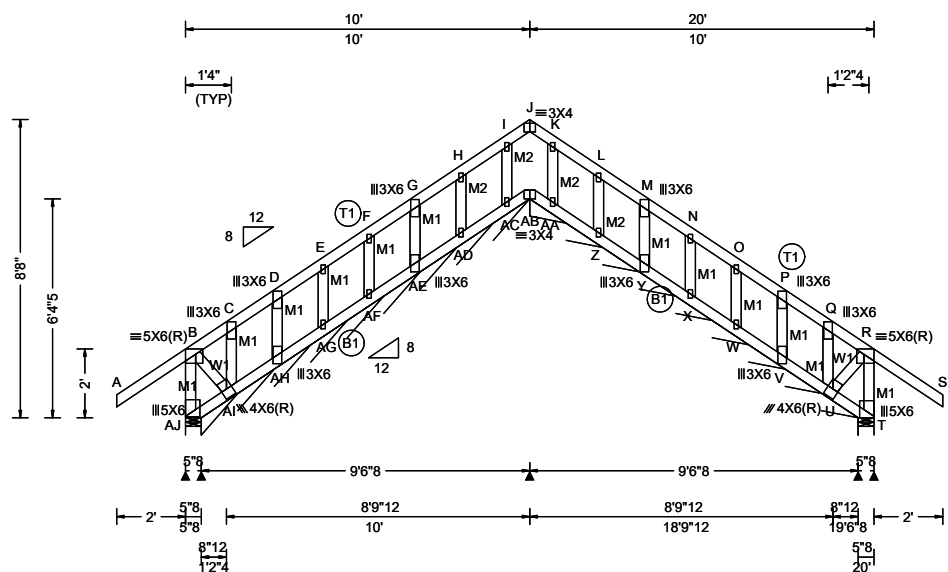
Design: BRR
Designer: BRR

Plan Examiner: JANEEN CASEBEER

REVIEWED FOR CODE COMPLIANCE - PERMIT SONOMA - BUILDING PLAN CHECK

JOB NO:
25-238

PAGE NO:
1 OF 1



Loading Criteria (psf) TCLL: 20.00 TCDL: 5.00 BCLL: 0.00 BCDL: 7.00 Des Ld: 32.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 110 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 3.0 psf BCDL: 4.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.25	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: CBC 2022 Res TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.007 R 999 360 VERT(CL): 0.007 R 999 240 HORZ(LL): -0.031 J - - HORZ(TL): 0.034 J - - Creep Factor: 2.0 Max TC CSI: 0.420 Max BC CSI: 0.198 Max Web CSI: 0.697 Mfg Specified Camber: VIEW Ver: 23.01.02.0731.17	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL					
				AJ 604 /- /- /2313 /2136 /448 AJ* 88 /- /- /261 /213 /- AB* 98 /- /- /262 /212 /- T 604 /- /- /2313 /2136 /- AI /-2180 U /-2180 Wind reactions based on MWFRS AJ Brg Wid = 5.5 Min Req = 1.9 (Truss) AJ Brg Wid = 114 Min Req = - AB Brg Wid = 114 Min Req = - T Brg Wid = 5.5 Min Req = 1.9 (Truss) Bearings AJ, AJ, AB, & T are a rigid surface.					

Lumber Top chord: 2x4 DF-L #1&Bet.(g); Bot chord: 2x4 DF-L #2(g); Webs: 2x4 DF-L Standard(g); M2 2x4 DF-L #1&Bet.(g); Plating Notes Connectors in green lumber (g) designed using NDS/TPI reduction factors. All plates are 1.5X3 except as noted. Loading Truss transfers a maximum horizontal load of 2500 # (125.01 plf) along top chord, from either direction, to supports where indicated. Diaphragm and connections are to be designed by Engineer of Record. Drag Loads: Force(#) PLF Mbr Start End Case 1: 2500 125.01 TC 0.00 20.00 2500 131.00 BC 0.46 19.54 Truss designed to support 2-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise. Bottom chord checked for 10.00 psf non-concurrent live load. Truss designed for unbalanced load using 0.00/1.00 windward/leeward factors.	Wind Wind loads based on MWFRS with additional C&C member design. End verticals exposed to wind pressure. Deflection meets L/240. Wind loading based on both gable and hip roof types. Additional Notes Negative reaction(s) of -2136# MAX. Requires uplift connection. See Maximum Reactions. Lumber shall be dried to a maximum moisture content of 19% prior to installation. See DWGS A11515ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements. Shim all supports to solid bearing.	Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 134 -13 J - K 269 -91 B - C 1419 -1466 K - L 324 -314 C - D 1325 -1331 L - M 511 -506 D - E 1105 -1108 M - N 711 -709 E - F 910 -911 N - O 910 -911 F - G 711 -709 O - P 1105 -1108 G - H 511 -506 P - Q 1325 -1331 H - I 329 -314 Q - R 1419 -1466 I - J 270 -91 R - S 134 -13 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. AJ-AI 523 -528 AB-AA 454 -257 AI-AH 1357 -1373 AA- Z 471 -312 AH-AG 1131 -1149 Z - Y 495 -522 AG-AF 918 -939 Y - X 707 -731 AF-AE 707 -731 X - W 918 -939 AE-AD 495 -522 W - V 1131 -1149 AD-AC 473 -312 V - U 1357 -1373 AC-AB 454 -257 U - T 110 -98 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. B -AI 1896 -1898 U - R 1896 -1898 Maximum Gable Forces Per Ply (lbs) Gables Tens.Comp. Gables Tens. Comp.
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****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**
 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.
 Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
 For more information see these web sites: Alpine: alpineitw.com; TPI: tpinet.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org



AJ- B	2157 -2320	AA- K	29 -213
C -AI	197 -228	Z - L	186 -139
D -AH	125 -150	Y - M	109 -147
E -AG	112 -148	X - N	111 -146
F -AF	111 -146	W - O	111 -148
G -AE	109 -147	V - P	124 -150
H -AD	179 -139	U - Q	191 -229
I -AC	65 -213	R - T	2157 -2320

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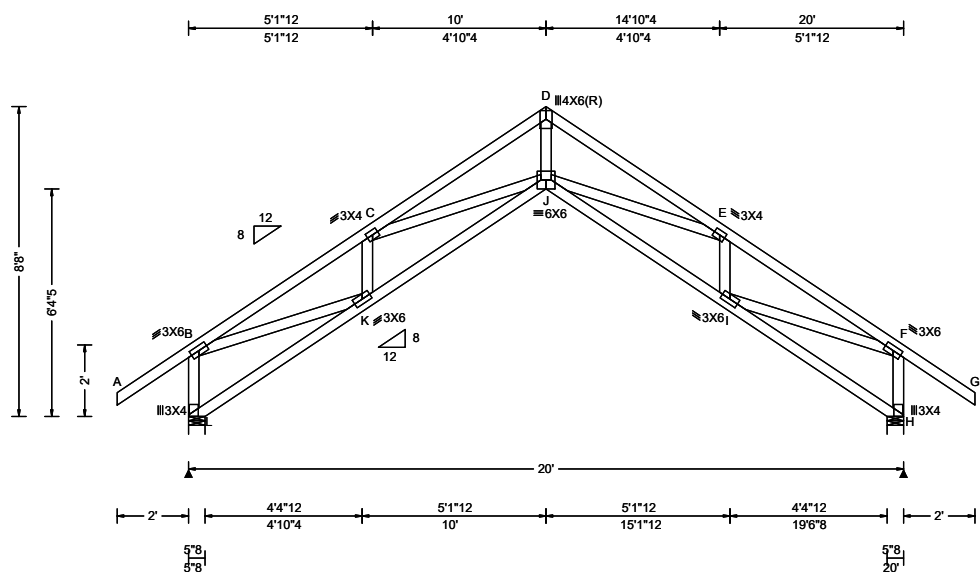
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

SEQN: 779564 / T1 / COMN
FROM:

Ply: 1
Qty: 19
Wgt: 0.0 lbs

Job Number: 25-238
TIMBER COVE GARAGE
Truss Label: A2

DRW: ... / ...
01/10/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	5.00
BCLL:	0.00
BCDL:	7.00
Des Ld:	32.00
NCBCLL:	10.00
Soffit:	2.00
Load Duration:	1.25
Spacing:	24.0 "

Wind Criteria

Wind Std: ASCE 7-16
Speed: 110 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 3.0 psf
BCDL: 4.2 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.25

Snow Criteria (Pg,Pf in PSF)

Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
CBC 2022 Res
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in loc L/defl L/#
VERT(LL): 0.145 J 999 360
VERT(CL): 0.242 J 972 240
HORZ(LL): 0.211 H - -
HORZ(TL): 0.352 H - -
Creep Factor: 2.0
Max TC CSI: 0.212
Max BC CSI: 0.328
Max Web CSI: 0.798
Mfg Specified Camber:
VIEW Ver: 23.01.02.0731.17

▲ Maximum Reactions (lbs)

Loc	Gravity			Non-Gravity		
	R+	R-	Rh	Rw	U	RL
L 802	-	-		/377	/110	/266
H 802	-	-		/377	/110	-

Wind reactions based on MWFRS
L Brg Wid = 5.5 Min Req = 1.5 (Truss)
H Brg Wid = 5.5 Min Req = 1.5 (Truss)
Bearings L & H are a rigid surface.

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - B	63 -22	D - E	256 -2069
B - C	228 -1543	E - F	178 -1543
C - D	223 -2069	F - G	63 -22

Lumber
Top chord: 2x4 DF-L #2(g);
Bot chord: 2x4 DF-L #2(g);
Webs: 2x4 DF-L Standard(g);

Plating Notes
Connectors in green lumber (g) designed using NDS/TPI reduction factors.

Loading
Bottom chord checked for 10.00 psf non-concurrent live load.
Truss designed for unbalanced load using 0.00/1.00 windward/leeward factors.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals exposed to wind pressure. Deflection meets L/240.
Wind loading based on both gable and hip roof types.

Additional Notes
Lumber shall be dried to a maximum moisture content of 19% prior to installation.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
L - K	278 -331	J - I	1521 -143
K - J	1521 -411	I - H	71 -105

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - L	197 -722	J - E	573 -317
B - K	1337 -89	E - I	103 -440
K - C	94 -440	I - F	1337 -106
C - J	442 -25	H - F	189 -722
D - J	1964 -253		

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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org





Alpine, an ITW Company
 155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025
 Phone: (800)877-3678 (916)387-0116
 Fax: (916)387-1110
 www.alpineitw.com

Site Information:		Page 1:	
<i>Customer:</i> American Truss Company, Inc.		<i>Job Number:</i> 25-238	
<i>Job Description:</i> TIMBER COVE GARAGE			
<i>Address:</i> 22090 TIMBER COVE ROAD, TIMBER COVE, CA 95450			

Job Engineering Criteria:			
<i>Design Code:</i> CBC 2022 Res		<i>IntelliVIEW Version:</i> 23.01.02	
		<i>JRef #:</i> 1Y8H8800009	
<i>Wind Standard:</i> ASCE 7-16	<i>Wind Speed (mph):</i> 110	<i>Design Loading (psf):</i> 32.00	
<i>Building Type:</i> Closed			

This package contains general notes pages, 2 truss drawing(s) and 3 detail(s).

Item	Drawing Number	Truss
1	083.25.1755.51633	A1
3	A11515ENC160118	
5	GBLLETIN0118	

Item	Drawing Number	Truss
2	083.25.1755.59667	A2
4	GABRST160118	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high-quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Bearing Information:

The bearing area factor, C_b , is considered for the allowable capacity of solid sawn wood bearings supporting trusses that are located a minimum of 3" from the end of the lumber piece.

General Notes (continued)

Coated Lumber:

Coated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Coated lumber has no adjustments to lumber properties. Coated lumber may be more brittle than uncoated lumber. Special handling care must be taken to prevent breakage during all handling activities. Refer to manufacturer literature, specifications, and code evaluation reports for restrictions, details, and requirements.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

C = Coated lumber.

C-AT = AtTEK coated lumber.

C-FX = FX Lumber Guard coated lumber.

C -TE = TechWood 4400 coated lumber.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-BF = Boraflame Fire Retardant Treated lumber

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-ON = OnWood Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

FRT-PR = ProWood Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

General Notes (continued)

Key to Terms (continued):

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

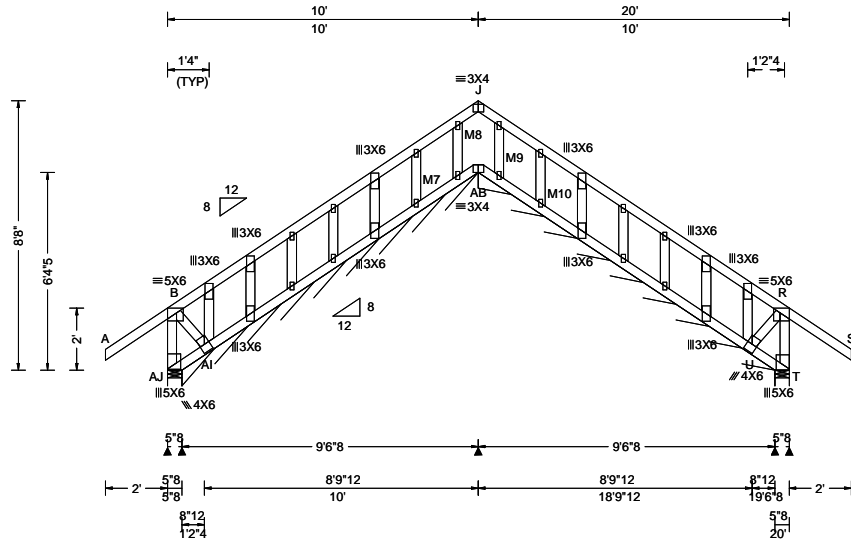
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoclin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com



Loading Criteria (psf) TCLL: 20.00 TCDL: 5.00 BCLL: 0.00 BCDL: 7.00 Des Ld: 32.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 110 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 3.0 psf BCDL: 4.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.25	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: CBC 2022 Res TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.007 R 999 360 VERT(CL): 0.007 R 999 240 HORZ(LL): -0.031 J - - HORZ(TL): 0.034 J - - Creep Factor: 2.0 Max TC CSI: 0.420 Max BC CSI: 0.198 Max Web CSI: 0.697 VIEW Ver: 23.01.02.0731.17	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL AJ 604 /- /- /2313 /2136 /448 AJ* 88 /- /- /261 /213 /- AB*98 /- /- /262 /212 /- T 604 /- /- /2313 /2136 /- AI /-2180 U /-2180 Wind reactions based on MWFRS AJ Brg Wid = 5.5 Min Req = 1.9 (Truss) AJ Brg Wid = 114 Min Req = - AB Brg Wid = 114 Min Req = - T Brg Wid = 5.5 Min Req = 1.9 (Truss) Bearings AJ, AJ, AB, & T are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - J 1419 - 1466 J - R 1419 - 1466 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. AJ-AI 523 -528 AB- U 1357 - 1373 AI-AB 1357 - 1373 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. B -AI 1896 - 1898 U - R 1896 - 1898 Maximum Gable Forces Per Ply (lbs) Gables Tens.Comp. Gables Tens. Comp. AJ- B 2157 - 2320 R - T 2157 - 2320
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Lumber
Top chord: 2x4 DF-L #1&Bet.(g);
Bot chord: 2x4 DF-L #2(g);
Webs: 2x4 DF-L Standard(g); M7,M8,M9,
M10 2x4 DF-L #1&Bet.(g);

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals exposed to wind pressure. Deflection meets L/240.
Wind loading based on both gable and hip roof types.

Plating Notes
Connectors in green lumber (g) designed using NDS/TPI reduction factors.
All plates are 1.5X3 except as noted.

Loading
Truss transfers a maximum horizontal load of 2500 # (125.01 plf) along top chord, from either direction, to supports where indicated. Diaphragm and connections are to be designed by Engineer of Record.
Drag Loads: Force(#) (PLF) Mbr Start End
Case 1: 2500 125.01 TC 0.00 20.00
2500 131.00 BC 0.46 19.54

Truss designed to support 2-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.
Bottom chord checked for 10.00 psf non-concurrent live load.
Truss designed for unbalanced load using 0.00/1.00 windward/leeward factors.
Top chord may be notched 1.5" deep X 3.5" periodically along top edge. DO NOT OVERCUT. No knots or other lumber defects allowed within 12" of notches. Do not notch in overhang or heel panel, unless specified otherwise.



SEQN: 779568	GABL	Ply: 1	Job Number: 25-238	Cust: R 880 JRef: 1Y8H8800009 T2
FROM:		Qty: 2	TIMBER COVE GARAGE	DrwNo: 083.25.1755.51633
Page 2 of 2			Truss Label: A1	/ JAK 03/24/2025

Additional Notes

Negative reaction(s) of -2136# MAX. Requires uplift connection. See Maximum Reactions.

Lumber shall be dried to a maximum moisture content of 19% prior to installation.

See DWGS A11515ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

Shim all supports to solid bearing.

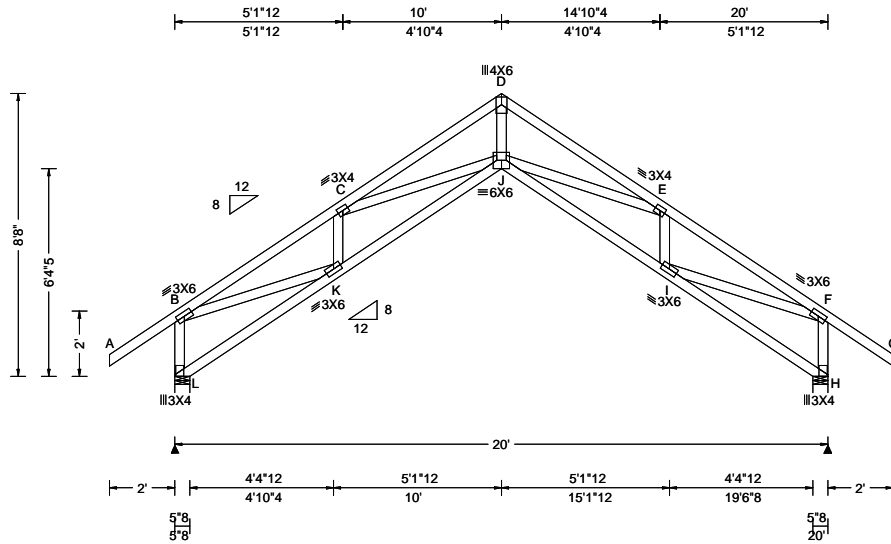


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American Truss Company, Inc.
 4550 Spring Hill Road
 PETALUMA CA 94952
 (707)763-8713



155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025



Loading Criteria (psf) TCLL: 20.00 TCDL: 5.00 BCLL: 0.00 BCDL: 7.00 Des Ld: 32.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 110 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 3.0 psf BCDL: 4.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.25	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: CBC 2022 Res TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.145 J 999 360 VERT(CL): 0.242 J 972 240 HORZ(LL): 0.211 H - - HORZ(TL): 0.352 H - - Creep Factor: 2.0 Max TC CSI: 0.212 Max BC CSI: 0.328 Max Web CSI: 0.798 VIEW Ver: 23.01.02.0731.17	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL L 802 - / - / 377 / 110 / 266 H 802 - / - / 377 / 110 / - Wind reactions based on MWFRS L Brg Wid = 5.5 Min Req = 1.5 (Truss) H Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings L & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 228 - 1543 D - E 256 - 2069 C - D 223 - 2069 E - F 178 - 1543 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. K - J 1521 - 411 J - I 1521 - 143 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. B - L 197 - 722 J - E 573 - 317 B - K 1337 - 89 E - I 103 - 440 K - C 94 - 440 I - F 1337 - 106 C - J 442 - 25 H - F 189 - 722 D - J 1964 - 253
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Lumber

Top chord: 2x4 DF-L #2(g);
Bot chord: 2x4 DF-L #2(g);
Webs: 2x4 DF-L Standard(g);

Plating Notes

Connectors in green lumber (g) designed using NDS/TPI reduction factors.

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.
Truss designed for unbalanced load using 0.00/1.00 windward/leeward factors.

Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals exposed to wind pressure. Deflection meets L/240.
Wind loading based on both gable and hip roof types.

Additional Notes

Lumber shall be dried to a maximum moisture content of 19% prior to installation.



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Alpine Truss Company, Inc.
4550 Spring Hill Road
Petaluma CA 94952
(707)763-8713



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

Gable Stud Reinforcement Detail

ASCE 7-16: 115 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Max Gable Vertical Length	2x4 Gable Vertical		Brace Grade	No Braces	(1) 1x4 'L' Brace *		(1) 2x4 'L' Brace *		(2) 2x4 'L' Brace **		(1) 2x6 'L' Brace *		(2) 2x6 'L' Brace **	
	Spacing	Species			Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
	24" o.c.	SPF	#1 / #2	#1 / #2	4' 11"	8' 5"	8' 9"	9' 11"	10' 4"	11' 10"	12' 4"	14' 0"	14' 0"	14' 0"
#3				4' 8"	8' 1"	8' 7"	9' 10"	10' 3"	11' 9"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
Stud				4' 8"	8' 0"	8' 6"	9' 10"	10' 3"	11' 9"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
HF			#1	4' 8"	6' 11"	7' 4"	9' 2"	9' 10"	11' 9"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 2"	8' 7"	8' 10"	10' 1"	10' 6"	12' 0"	12' 5"	14' 0"	14' 0"	14' 0"	14' 0"
			Standard	4' 8"	8' 0"	8' 6"	9' 10"	10' 3"	11' 9"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
SP		#1	#1	4' 11"	8' 5"	8' 9"	9' 11"	10' 4"	11' 10"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	4' 11"	8' 5"	8' 9"	9' 11"	10' 4"	11' 10"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"
			Standard	4' 10"	7' 3"	7' 9"	9' 8"	10' 3"	11' 9"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		DFL	#1	4' 10"	7' 3"	7' 9"	9' 8"	10' 3"	11' 9"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	4' 10"	7' 3"	7' 9"	9' 8"	10' 3"	11' 9"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"
			Standard	4' 8"	6' 5"	6' 10"	8' 7"	9' 2"	11' 7"	12' 2"	13' 5"	14' 0"	14' 0"	14' 0"
16" o.c.	SPF	#1 / #2	#1 / #2	5' 8"	9' 8"	10' 0"	11' 5"	11' 10"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"	
			#3	5' 5"	9' 6"	10' 0"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	
			Stud	5' 5"	9' 6"	9' 10"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	
		HF	#1	5' 5"	9' 6"	9' 10"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	
			#2	5' 11"	9' 9"	10' 2"	11' 6"	12' 0"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	
			Standard	5' 5"	8' 5"	9' 0"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	
	SP	#1	#1	5' 11"	9' 9"	10' 2"	11' 6"	12' 0"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	
			#2	5' 8"	9' 8"	10' 0"	11' 5"	11' 10"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"	
			Standard	5' 6"	8' 11"	9' 6"	11' 4"	11' 9"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	
		DFL	#1	5' 6"	8' 11"	9' 6"	11' 4"	11' 9"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	
			#2	5' 6"	8' 11"	9' 6"	11' 4"	11' 9"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	
			Standard	5' 5"	7' 10"	8' 4"	10' 6"	11' 2"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	
12" o.c.	SPF	#1 / #2	#1 / #2	6' 3"	10' 7"	11' 0"	12' 7"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"	
			#3	5' 11"	10' 6"	10' 10"	12' 5"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
			Stud	5' 11"	10' 6"	10' 10"	12' 5"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		HF	#1	5' 11"	9' 9"	10' 5"	12' 5"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
			#2	6' 6"	10' 9"	11' 12"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
			Standard	5' 11"	9' 9"	10' 5"	12' 5"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
	SP	#1	#1	6' 6"	10' 9"	11' 12"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
			#2	6' 3"	10' 7"	11' 0"	12' 7"	13' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
			Standard	6' 1"	10' 3"	10' 11"	12' 6"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		DFL	#1	6' 1"	10' 3"	10' 11"	12' 6"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
			#2	6' 1"	10' 3"	10' 11"	12' 6"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
			Standard	5' 11"	9' 1"	9' 8"	12' 1"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	

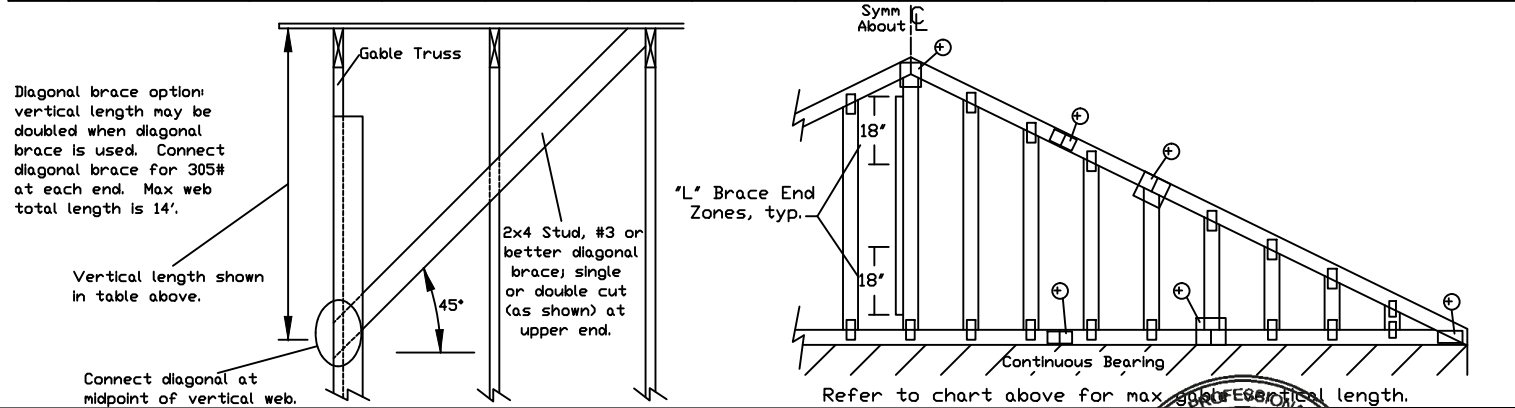
Bracing Group Species and Grades:

Group A:			
Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard
Douglas Fir-Larch		Southern Pine***	
#3		#3	
Stud		Stud	
Standard		Standard	

Group B:	
Hem-Fir	
#1 & Btr	
#1	
Douglas Fir-Larch	
#1	
#2	
Southern Pine***	
#1	
#2	

1x4 Braces shall be SRB (Stress-Rated Board).
 ***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:
 Wind Load deflection criterion is L/240.
 Provide uplift connections for 30 plf over continuous bearing (5 psf TC Dead Load).
 Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12' plywood overhang.



Attach 'L' braces with 10d (0.128"x3.0" min) nails.
 * For (1) 'L' brace: space nails at 2' o.c. in 18' end zones and 4' o.c. between zones.
 ** For (2) 'L' braces: space nails at 3' o.c. in 18' end zones and 6' o.c. between zones.
 'L' bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0", but less than 11' 6"	2X4
Greater than 11' 6"	3X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

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MAX. TOT. LD. 60 PSF

03/24/2025

MAX. SPACING 24.0"

REF	ASCE7-16-GAB11515
DATE	01/26/2018
DRWG	A11515ENC160118

ASCE 7-16: 120 mph, 30' Mean Height, Closed, Exposure C Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30ft. Mean Hgt, ASCE 7-16, Enclosed, Exp C, or
100 mph, 30ft. Mean Hgt, ASCE 7-16, Enclosed, Exp D, or
100 mph, 30ft. Mean Hgt, ASCE 7-16, Part. Enclosed, Exp C,
Kzt = 1.00, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf.

Lateral chord bracing requirements
Top: Continuous roof sheathing
Bot: Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail for lumber, plates, and other information not shown on this detail.

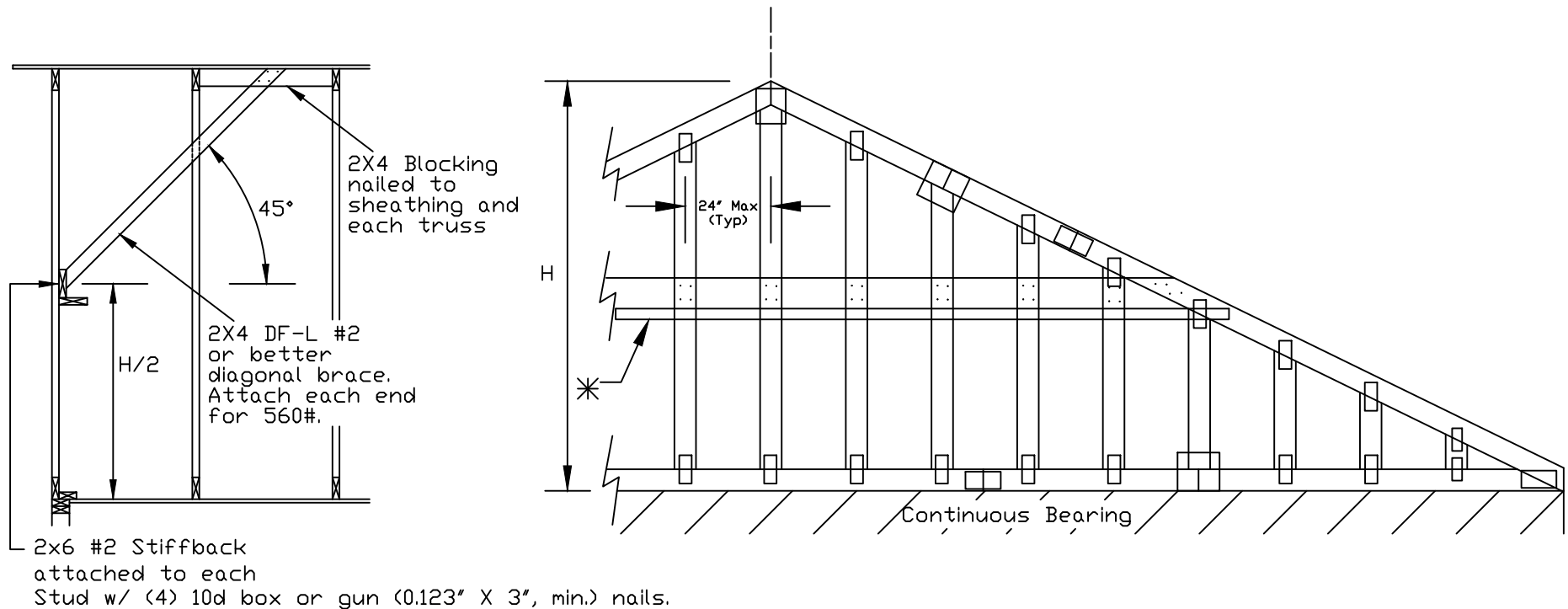
Nails: 10d box or gun (0.128"x3",min) nails.

H Less than 4'6" - no stud bracing required

H Greater than 4'6" to 7'6" in length
provide a 2x6 stiffback at mid-height and brace stiffback to roof diaphragm every 6'0" (see detail below or refer to DRWG A12030ENC160118).

H Greater than 7'6" to 12'0" max:
provide a 2x6 stiffback at mid-height and brace to roof diaphragm every 4'0" (see detail below or refer to DRWG A12030ENC160118).

* Optional 2x L-reinforcement attached to stiffback with 10d box or gun (0.128" x 3", min.) nails @ 6" o.c.



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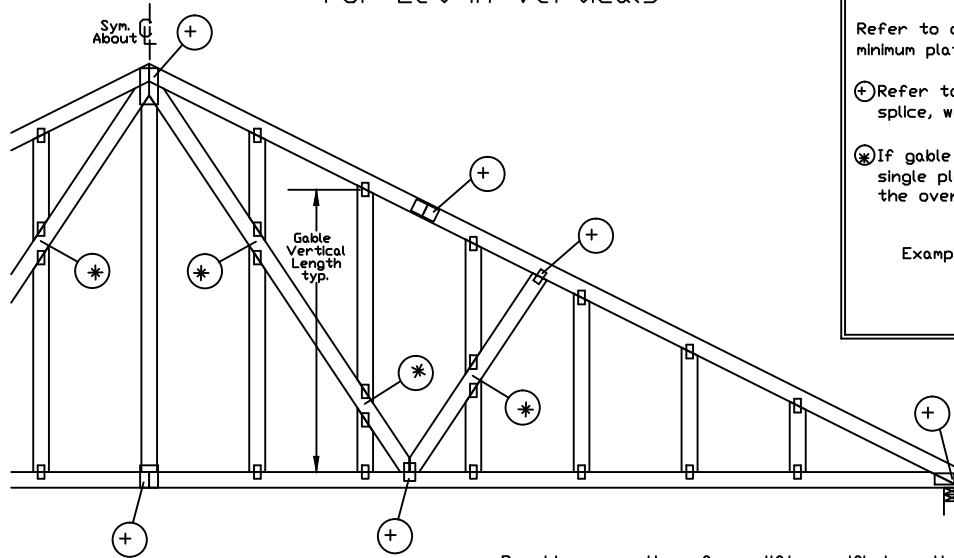
MAX. TOT. LD. 60 PSF

MAX. SPACING

REF	GE WHALER
DATE	01/02/2018
DRWG	GABRST160118

03/24/2028

Gable Detail For Let-in Verticals

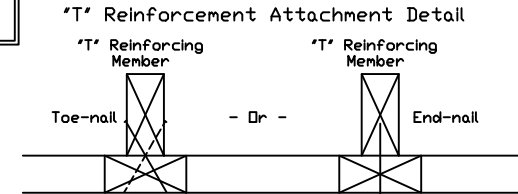


Gable Truss Plate Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

- ⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.
- ⊗ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



Provide connections for uplift specified on the engineered truss design.

Attach each 'T' reinforcing member with
End Driven Nails:
10d Common (0.148"x3",min) Nails at 4' o.c. plus
(4) nails in the top and bottom chords.

Toenailed Nails:
10d Common (0.148"x3",min) Toenails at 4' o.c. plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

- ASCE 7-05 Gable Detail Drawings
A13015051014, A12015051014, A11015051014, A10015051014, A14015051014,
A13030051014, A12030051014, A11030051014, A10030051014, A14030051014
- ASCE 7-10 & ASCE 7-16 Gable Detail Drawings
A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118,
A18015ENC100118, A20015ENC100118, A20015END100118, A20015PED100118,
A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118,
A18030ENC100118, A20030ENC100118, A20030END100118, A20030PED100118,
S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118,
S18015ENC100118, S20015ENC100118, S20015END100118, S20015PED100118,
S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118,
S18030ENC100118, S20030ENC100118, S20030END100118, S20030PED100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.

To convert from 'L' to 'T' reinforcing members, multiply 'T' increase by length (based on appropriate Alpine gable detail).

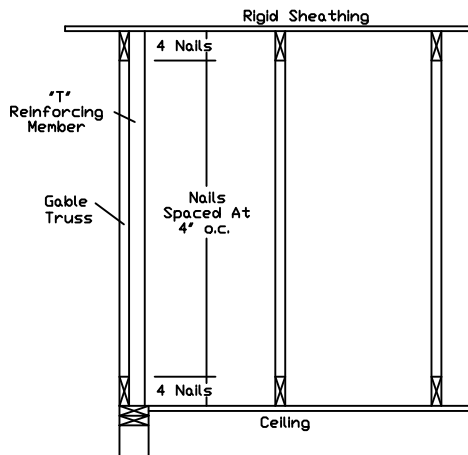
Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord.

'T' reinforcing member material must match size, specie, and grade of the 'L' reinforcing member.

Web Length Increase w/ 'T' Brace

'T' Reinf. Mbr. Size	'T' Increase
2x4	30 %
2x6	20 %

Example:
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft, Kzt = 1.00
Gable Vertical = 24' o.c. SP #3
'T' Reinforcing Member Size = 2x4
'T' Brace Increase (From Above) = 30% = 1.30
(1) 2x4 'L' Brace Length = 8' 7"
Maximum 'T' Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"



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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



03/24/2025

REF LET-IN VERT
DATE 01/02/2018
DRWG GBLLETIN0118

MAX. TOT. LD. 60 PSF
DUR. FAC. ANY
MAX. SPACING 24.0"