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June 1, 2026

To: Interested Agencies

The following application has been filed with the Sonoma County Permit and Resource Management Department.

File Number:	DRH26-0002
Applicant Name:	Martin Dreiling for Dreiling Terrones Architecture
Owner Name:	Northern Sonoma County Fire Protection District
Site Address:	20975 Geyserville Ave., Geyserville
APN:	140-100-070
Zoning:	K, PF, NONE

Project Description: The proposed project at Geyserville Station 1 includes two efforts: 1) modernization of the existing administrative and management areas with minor additions; 2) construction of a new building at the rear of the property. Original facility approved under PLP03-0051/UPE03-0061/DRH03-0006.

We are submitting the above application for your review and recommendation. Additional information is on file in this office.

Responses to referrals should include a combination of any or all of the following details:

- (1) Statement of any environmental concerns or uncertainties your agency may have with the project.
- (2) Comments you wish to make regarding the merits of the project.
- (3) Identification of any missing information or application submittals that will preclude you from providing conditions and mitigations for this project in the future.
- (4) Your proposed conditions of approval and/or mitigations for this project.

After reviewing this application, please respond to the planner with your *marked* response below:

- ☐ Conditions will be provided and no further information is necessary.
- ☐ Conditions will be provided and additional information is necessary.
- ☐ Comments and/or concerns.
- ☐ No comments or conditions.

Responsible agencies under CEQA are requested to indicate whether permits will be required for this project.

Your comments will be appreciated by June 15, 2026, and should be sent to the attention of:

DRH26-0002, Levan King Cranston (Levan.KingCranston@sonomacounty.gov). The Project Planner can also be reached at (707) 565-2592. **If no response is received by June 15, 2026, it will be assumed that no comments or conditions will be provided.**

Please send a copy of your comments to the applicant(s) or their representatives as indicated on the attached Planning Application.

- | | |
|--|--|
| ☒ Sanitation | ☒ Local Fire District – Northern Sonoma County FPD |
| ☒ Fire Prevention | ☒ Water District - Geyserville |
| ☒ Grading and Storm Water | ☒ Recology Sonoma Marin (Disposal) |
| ☒ Management Group | ☒ Caltrans-State Dept of Transportation |
| ☒ Natural Resources | ☒ Regional Water QCB: North Coast |
| ☒ Road Naming | ☒ Air Pollution Control: No. So. County / Bay Area AQM |
| ☒ Assessor | ☒ Tribal Notification |
| ☒ Dist. 4 Director and Commissioners | ☒ SMART |
| ☒ SPI (DTPW), Land Development | ☒ Geyserville Planning Advisory Board |
| ☒ Treasurer/Special Assessment | ☒ Alexander Valley Association |

Planning Application

PJR-001

Application Type(s):

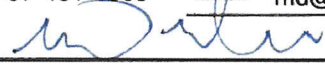
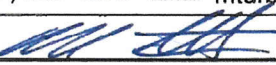
- | | |
|--|--|
| ☐ Admin Cert. Compliance | ☒ Design Review Admin. |
| ☐ Ag. or Timber Preserve/Contract | ☐ Design Review Full |
| ☐ Conditional Cert. of Compliance | ☐ General Plan Amendment |
| ☐ Cert. of Modification | ☐ Lot Line Adjustment |
| ☐ Coastal Permit | ☐ Major Subdivision |
| ☐ Zoning Permit for: _____ | |

File # _____

- | | |
|---|--|
| ☐ Minor Subdivision | ☒ Use Permit |
| ☐ Voluntary Merger | ☐ Variance |
| ☐ Ordinance Interpretation | ☐ Zone Change |
| ☐ Second Unit Permit | ☐ Other: _____ |
| ☐ Specific/Area Plan Amendment | |

By placing my contact information (name, address, phone number, email address, etc.) on this application form and submitting it to Sonoma County PRMD, I understand and authorize PRMD to post this application to the internet for public information purposes, including my contact information.

PRINT CLEARLY

APPLICANT				OWNER (IF OTHER THAN APPLICANT)			
Name Martin Dreiling				Name Northern Sonoma County Fire Protection District			
Mailing Address 314 Center Street				Mailing Address 20975 Geyserville Ave.			
City Healdsburg	State CA	Zip 95448		City Geyserville	State CA	Zip 95441	
Day Ph 707-431-1305		Email md@dtbarch.com		Day Ph (707) 857-4373		Email mturbeville@nosocofire.com	
Signature 		Date 4-20-26		Signature 		Date 4-20-26	
Billing Responsible Party (At-Cost Only) ☐ Applicant ☒ Owner ☐ Other: _____							
OTHER PERSONS TO RECEIVE CORRESPONDENCE							
Name/Title Kim Garibaldi				Name/Title _____			
Mailing Address 1103 Juanita Ave.				Mailing Address _____			
City Burlingame	State CA	Zip 94010		City _____	State _____	Zip _____	
Day Ph 650-696-1200		Email kg@dtbarch.com		Day Ph () _____		Email _____	
PROJECT INFORMATION							
Address(es) 20975 Geyserville Ave.						City Geyserville	
Assessor's Parcel Number(s) 140-100-070							
Project Description The proposed project includes two efforts: 1) Modernization of the existing administrative and management areas with minor additions. 2) Construction of a new building at the rear of the property.							
Acreage 1.02 acres				Number of new lots proposed none			
Site Served by Public Water? ☒ Yes ☐ No				Site Served by Public Sewer? ☒ Yes ☐ No			

TO BE COMPLETED BY PRMD STAFF

Planning Area	Supervisory District	☐ Critical Habitat	☐ Urban Service	Groundwater	☐ 1 / 2
Current Zoning		☐ NPDES	☐ Williamson Act	Availability	☐ 3 / 4
		Specific/Area Plan		Subject to	☐ EX
General Plan Land Use		Parcel Specific Policy		CEQA	☐ YES
Application resolve planning violation? ☐ Yes ☐ No		Violation? ☐ Yes ☐ No		File No. _____	
Previous Files		Penalty application? ☐ Yes ☐ No			
Application accepted by		Date			
Approved by		Date			

Sonoma County Permit and Resource Management Department
 2550 Ventura Avenue, Santa Rosa, CA, 95403-2829, (707) 565-1900

09/13/2018

Project	<u>NSCFPD Station 1 Modernization</u>
Address	<u>20975 Geyserville Ave., Geyserville</u>
DTA Project #	<u>2525</u>
Date:	4/20/26
Written By:	md

Project Narrative: Combined

Constraints Analysis

Project Site	Project site is an existing, fully developed urban site located in downtown Geyserville. The present site houses Station 1 of the Northern Sonoma County Fire Protection District. The Station was originally constructed in 2004. All parcel and building data, both existing and proposed, are listed in the attached "Project Data" form.
Site Information	Original project included a full survey and map recording, reflecting parcel combinations that were part of the project. The proposed project includes a new topo and boundary survey for the portion of the site affected by the new building. Survey includes full topo, property corners, utility identification, boundary identification and physical staking of all corners. Site utilities have been fully investigated via USA and by project engineers.
Existing Land Use on Site	Urban Fire Station. Includes: • Four enclosed apparatus bays • Support / storage / maintenance facilities • Various Operations Management areas • Emergency medical and operational supply stockpiles • Capacity for expanded function during emergencies • Staffed 24/7, and includes sleeping, cooking and recreational areas for full-time firefighters • Training tower and areas for selected training on site • Fire District Administration • Site circulation and parking • Additional vehicle parking in covered car port • Multiple storage containers for emergency supplies and deep storage A small portion of the site at the rear property line is undeveloped and was originally intended for facility expansion, as is presently proposed.
Surrounding Uses	North: • Geyserville main commercial district • Several residences are located at the rear of commercial uses • A single residence is located on the adjacent property. East: • Geyserville Avenue Frontage • Geyserville Avenue commercial uses across roadway • Single family residence across roadway South and West: Isis Oasis: Residential / recreational facility
Easements	Existing 15' public utility easement running along the north property line. It is held by the Cal American Water Company for the public water main that passes through the site.

Submittal Outline

continued

Project **Geyserville Station Mod - 2525**
Address **20975 Geyserville Ave., Geyserville 95441**
Date **04/20/26**

	Included in the project is the need to modify a short portion of that easement and relocate a short portion of the water main to clear the new building. Applicant is currently working with the water company on that design. Proposed modifications are shown on the Site Development plan.
Utilities	Water: Public water system - Cal American Water Company Sewer: Geyserville municipal sewer system Power: PG&E via overhead lines, underground on site, and on-site transformer. Gas: PG&E via existing underground service
Covenants and Restrictions	Building envelope is defined only by Zoning Ordinance for lot coverage, setbacks and height. PF Zone does not require Building Intensity compliance (however proposed project is compliant). The project is compliant with all limitations except for the rear property line. There are two zones listed for the property and the proposed rear setback complies with Zone K but not Zone PF. We are seeking waiver of the requirement to achieve a 10' rear yard setback. Administrative waiver is allowed by the Zoning Ordinance. No other limits are present.
Farming	No farming operations near project site.
Waterways and Drainage	No waterways on or adjacent to site. Adjacent properties either drain away from site, are neutral for drainage, or include managed drainage.
Vegetation	Vegetation on site consists almost entirely of landscaping installed in the original construction per the original permits. Landscaping is generally healthy. A small portion of the site at the rear of the property is not landscaped. This is where the new building will be placed.
Tree Protection	All trees on site are landscape trees. No "natural" or native trees occur on site. Tree protection is required at rear parking areas and at southeast corner of building. Subject trees are non-specimen trees and require barriers only. One tree is proposed for removal near the addition to the existing building. It is a non-specimen tree planted by the firefighters. It will be replicated in another location at the building frontage. Three additional trees are proposed for removal at the rear of the site. These trees conflict with a very limited building envelope at the rear of the property. They also conflict with WUI construction standards and Zone Zero standards for the proposed project. Refer to Site Development Plans / Landscape plan for details.
Noise	Existing ambient noise is driven by the presence of an active commercial district. Existing noise generation from the site is normal for a functioning Fire Station. Existing impacts have been present for 20 years with no record of complaints. No change in site generated noise is proposed. Fire Station operations are not being expanded by this project. Noise transmission to adjacent sites will be reduced by the placement of the new building and by housing two vehicles in the new building that are currently housed in an open carport.
Hazards	No natural hazards are present on site. Existing hazards include the station fueling bay, constructed under the original permit and which was fully mitigated at that time.

No changes are proposed to fueling operations and no increase in use is proposed.

Proposed Project

Proposed Land Use

Proposed land use remains a public Fire Station. No change in land use is proposed.

Proposed Project

Not Growth

This project does not increase operational quantities or impacts. It is NOT a growth project.

The primary purpose of the project is to correct organizational and territorial deficiencies, make industry-standard improvements to the living quarters for the firefighters, address storage needs more effectively, and clean up the rear portion of the site.

The proposed project includes two tasks:

- Modernization of the existing administrative and management areas with minor additions.
- Construction of a new building at the rear of the property.

Refer to attached Project Data sheet for all site and building data.

Modernization

The **Modernization** project includes interior reorganization of administrative areas to address deficiencies in the original design, territorial conflicts and various systems deficiencies. It is primarily a “tenant improvement” project.

The project also addresses deferred maintenance items such as roof replacement, aging HVAC system and aging interior lighting.

It does include several additions to the existing building:

1. Two-story addition adjacent to existing kitchen to provide a proper day room and evening room for firefighting staff.
2. Second floor addition above the present “shop wing” to provide deep storage of personnel equipment, overflow medical supplies, excess uniforms and items appurtenant to emergency response.

New Construction

The **New Construction** project includes construction of a new building at the rear of the property to accommodate territorial issues in the existing building and resolve existing site storage issues. It will include the following areas:

1. New exercise facility to replace the undersized room in the existing building. This is a key need to allow staff to exercise together as a unit, while also making important space in the main building available.
2. Enclosed parking for two vehicles. These are currently housed in an open carport on-site and require enclosure?
3. Storage room for deep storage overflow from the main building, for long term emergency supplies currently stored in used shipping containers and deep records storage for the admin operation.

Future Use

This project substantially builds out the site.

Remaining developable area is the existing exposed carport (which will be removed). This area will be used for overflow parking. A permanent covered carport may be constructed in the future. Such expansion will likely consist of a roof extension of the new building so that the architecture is consistent and harmonized.

No firm plans exist for this.

Setbacks

Setbacks are listed in the attached Project Data sheet and shown on the site plan.

Submittal Outline

continued

Project **Geyserville Station Mod - 2525**
Address **20975 Geyserville Ave., Geyserville 95441**
Date **04/20/26**

All setbacks are in compliance with Zoning Ordinance with the exception of the rear property line. We are seeking administrative approval to use the K-zone setback in lieu of the PF zone setback in that location.

Heights Existing and new buildings are complaint with Zoning Ordinance height limitations. Actual heights are show in the attached Project Data sheet and in the plans.

Lot Coverage Existing and new buildings are fully complaint with Zoning Ordinance Lot Coverage limits. Coverage is shown in the attached Project Data sheet.

Hours of Operation Administrative functions M-F: 9am – 5pm

Firefighters 24/7

Firefighters are present for 48-hour shifts. There are three shifts that rotate on a 6-day cycle.

Employees Admin Staff (2) - full time, Mon-Fri

Firefighters (4) - full time, 24/7

Chief (1) - on-site part time

Inspector (1) - part time

Storage Area Existing storage occurs throughout the facility in various closets, rooms, portable cabinets and other fixtures.

Additionally (5) shipping containers are placed at the rear of the site for emergency supplies and deep storage. These will be removed and replaced by the storage portion of the new building.

No hazardous materials are stored.

New Storage: 2nd Floor at Main Building: **1,650 sf**

- surplus clothing and protective gear
- excess gear for volunteers
- surplus medical supplies
- surplus custodial supplies
- other light weight supplies requiring semi-deep storage.

Storage Room at New Building: **1,000 sf**

- surplus parts
- redundant and specialized emergency equipment
- various volunteer and foundation supplies
- surplus veg management tools

Outdoor storage areas None proposed

Visitors No change in existing visitation proposed or expected.

Marketing/Promotional No event capability is proposed.

Food Service No public food service is proposed.

Housing Plan Not applicable

Circulation Plan

Site Access and Circulation No changes are proposed to site access and internal circulation.
Project does not increase staffing or change any demands for parking.

Present access and circulation have generated no impacts.

Traffic Generation Project will generate no changes to present trip generation.

Existing trips include:

Submittal Outline

continued

Project **Geyserville Station Mod - 2525**
Address **20975 Geyserville Ave., Geyserville 95441**
Date **04/20/26**

	• staff arrival and departure • insubstantial visitation by other firefighters, occasional public • emergency response vehicle trips
Parking	No changes are proposed to existing parking. Existing parking is shown on plans. No public parking is allowed on site. All public parking occurs in public spaces on Geyserville Ave.
Loading Areas	Not applicable
Transit	Not applicable

Utilities and Services

Water Supply	Public water company: Cal American Water Company. Project is adding one domestic bathroom and one kitchen sink. No capacity increase is required. Project will connect to existing building water with no changes required to existing service, meter or backflow.
Water Conservation Plan	Existing building currently utilizes standard features for reduced water use as required by code. New building will meet the same standards. Due to Essential Services use, other features (graywater / rainwater collection) are not applicable.
Sewage Disposal System	Geyserville municipal sewer system.
Storm Water Management Plan	Project affects less than 10,000 sf of site area and is presumed exempt from onsite treatment. Existing site is assumed to be complaint based on 2004 permit. Consideration is only made for new construction area at rear of site. At rear of site: • Existing impervious features will be removed: carport, containers. • Existing impervious paving will remain. • 85% of new paving will be pervious.
Solid Waste Disposal	Facility currently served by residential trash, recycling and green waste bins. No change is proposed. Trash bin location will move to new gated trash enclosure as shown on plans.
Fire and Emergency Services	Conveniently there is a Fire Station located on our site. The site is highly accessible by emergency vehicles. Site is located in the Local Responsibility Area, is listed as a non-wildland area and is listed as an Extreme Fire Severity zone. Buildings will be constructed using WUI standards. Buildings will adopt proposed Zone 0 for exterior landscaping and treatments. Building finishes are non-combustible and building details prevent fire penetration.
Energy Conservation Plan	No formal energy management plan is included. Project generates less than 10% increase in treated space. No increase in proposed staffing.

Key energy management tasks include:

- Elimination of gas fired heating. Replaced with elec. heat pumps.
- Omission of air handlers. Replaced by fan-coil units with fans at individual rooms.
- Omission of general area thermostats. Replaced by individual room control and automatic system cutbacks when there are no occupants.
- Removal of fluorescent lighting. Replaced with 100% LED.
- All lighting motion controlled or controlled by central lighting controller.

Proposed Design

Architectural Design Style

The design of the proposed building is intended to match or fully harmonize with the existing building.

Modernization

For the existing building, any exterior modifications will fully match existing.

- We are using windows, metal siding and metal roofs provided by the original manufactures.
- Exterior plaster finishes will match; colors will match; roof pitches and eave details will match.
- The intent is to make the expansion invisible.

New Construction

The new building will include materials and details identical to the existing building.

Roofs will be a slightly lower pitch to reduce height and visual impact on the neighbors. Metal siding, metal roofs and exterior plaster will match existing.

Colors will vary slightly as this is a subordinate building to the main building.

Architectural Impacts

Additions to the existing building will have no substantial visual impacts. View perimeters will occur generally within the existing view perimeters. Massing will occur in foregrounds of existing massing and not extend above.

The new building is oriented such that the low eaves occur at the property lines and the higher ridges occur at the interior of the site.

Colors and finishes will harmonize with the existing building and will be an improvement over existing carport and containers where visible.

Landscaping and Fencing

The project will require no removal of existing landscaping with the exception of existing gravel (Zone 0) areas near the front of the existing building.

Small areas of landscaping are proposed in front of the new building and at portions of the rear of the new building outside the exercise area. Such landscaping will consist of decorative gravels in Zone 0 areas and native plants requiring no water (after startup) in planting areas.

Landscaping will be design-build, likely performed by volunteers. We are describing performance intention only in these plans.

Natural Features

No relevant natural features occur on site. This is a fully developed urban property and all areas of the site have been utilized in one way or another.

Grading

Grading is limited to the property to accommodate the new building. The building has been designed to respond to existing grades except for the new garage area.

Geotechnical study requires sub-excavation and engineered fill under the new building. Requirement is for 2 ft. of fill below footings which results in a minor sub-ex task.

All material will be stockpiled on site. Minimum export is required.

Import will generally consist of graded drain rock and Class II material to make up final grades.

The Tank

Project includes removal and salvage of an existing "draft tank." This is a training tank containing fresh water used for firefighter hose training. Water is pumped out and sprayed back into the tank (as a water conservation measure).

	The tank will be removed and stockpiled for use at a future training facility off-site. The hole will be filled with engineered fill to the same specs as those for the new building.
Site Grading	Site grading is limited to the area of the new building. Existing parking and drives will remain as presently configured. Very slight changes to the existing drive near the new garage will be elevated by up to 9 inches and this will be accomplished with AC paving overlays. Cut and fill will occur below the new building to accommodate the new garage and to address the sub-ex requirement.
Lighting	All exterior lighting, including street lighting is existing. No new lighting is proposed on the existing building or the site. New exterior lighting for paths of travel will occur on the new building. These will be dome lights that direct all light toward the ground and will not be visible across property lines. Refer to attached cut sheet for new exterior light.
Materials and Colors	Refer to material color sheet in plans for materials and building colors. Refer to notes above for our approach to material and color design.
Parking	All site parking and circulation is existing. All parking was constructed in 2004. The only changes proposed are slight grade adjustments around the new garages to accommodate required drive slopes. No increase in staff accommodation or visitation will be triggered by this project. This project only addresses specific deficiencies in amenities and is not aimed at growth in any way. Thus, we are proposing no new parking or visitors. No parking deficiencies have been identified, existing parking generally accommodates all present uses with the exception of occasional open house events that may occur less than three times a year.
Accessibility	All public portions of the property were in compliance with Access requirements when the building was built in 2004. Only minor changes are being made to these areas and such changes will be fully compliant with Title 24. Compliance includes: • Required van parking on public way • Path of Travel • Interior Path of Travel • Toilet Facilities Most areas of the site and buildings are used and fully maintained only by firefighters. No public access is allowed.
Construction Methods & Timing	Construction will occur in two phases so that the Station can remain operational for the duration. Refer to submitted construction management plans. Phase 1 will include the new building and the second-floor addition at the back of the main building. All staging and logistics will occur at the rear of the site. Offsite parking for station staff will be arranged on an adjacent property. Phase 2 will include modernization and additions to the existing main building. All staging and logistics will occur on site or on the adjacent vacant property. For Phase 2 we will be seeking closure of the public parking on Geyserville Avenue in front of the station and control of parking across Geyserville Avenue for contractor vehicle parking. Additional staging and parking overflow will be arranged at the North Coast Railroad site on Railroad Ave. in Geyserville as needed.

Submittal Outline

continued

Project **Geyserville Station Mod - 2525**
Address **20975 Geyserville Ave., Geyserville 95441**
Date **04/20/26**

All construction is standard wood frame, convention foundations and conventional finishes. All equipment and processes will be small scale and will not require cranes or heavy equipment.

Work hours will be standard Sonoma County work hours.

Environmental Review

We are assuming the original project was certified as CEQA compliant and that the general plan designations and zoning ordinance have been tested for CEQA compliance.

We are assuming the County will determine that the project is either:

Categorically Exempt:

Basis

Negative Declaration:

Basis

Because the impacts of the project are minimal or non-existent we assume there will be no mitigation associated with a Neg Dec.

Project Management

Code Outline

Project Station 1 Modernization

Date 4/12/2026

DTA Project # 2525

Project Data

Project	20975 Geyserville Avenue, Geyserville, CA 95425
Primary APN	140-100-070
Original APNs	140-000-061
Orig Permit	BLD03-7104

2003 Approval

Zoning	K PF	Recreation and Visitor-Serving Commercial District Public Facilities District (Base District)
Land Use	PQP, RVSC	
Lot Size (ft. ²)	44,500 sf 1.02 ac	
Existing Building Area		Occupancy Group B
Ground Floor	9,630	Ground Floor B
Second Floor	2,920	Second Floor B
Total	12,550	Construction Group V-N
		Automatic Sprinkler System Yes
		Height 2 Story

Proposed Project Data

	Allowable / Required	Existing	Added	Removed	Total Proposed
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Lot Coverage

Lot Area		44,500 sf	- sf	- sf	44,500 sf
Lot Area		1.02 ac	- ac	- ac	1.02 ac
Building Footprint	17,800 sf	9,886 sf	3,862 sf	- sf	13,748 sf
Lot Coverage Total	40.00% %	22.22% %	8.68% %	%	30.89% %

Building Area

Bldg M					
Ground Floor					
Building M: Ground		9,630 sf	718 sf	0 sf	10,348 sf
Tower		256 sf	0 sf	0 sf	256 sf
Subtotal Ground	0 sf	9,886 sf	718 sf	0	10,604 sf
Second Floor					
Building M: 2nd	sf	2,920 sf	1,650 sf	0 sf	4,570 sf
Tower	sf	256 sf	0 sf	0 sf	256 sf
Subtotal Second	0 sf	3,176 sf	1,650 sf	0 sf	4,826 sf
Total Bldg M	0 sf	13,062 sf	2,368 sf	0 sf	15,430
Bldg B					
Bldg B: Ground			3,144 sf		3,144 sf
Total Bldg B	0 sf	0 sf	3,144 sf	0 sf	3,144 sf
Total Building Area	0 sf	13,062 sf	5,512 sf	0 sf	18,574 sf

Height

Building M	35 ft	35 ft		35 ft
Building B	35 ft	- ft	23 ft	23 ft

Building Intensity

	Not required for PF Zone				
BH	35 ft	35 ft	-	-	35 ft
Lot Cov	17,800 sf	9,886 sf	3,862 sf	- sf	13,748 sf
	623,000 cf	346,010 cf			481,180 cf

Setbacks

	Zone K Allowable / Required	Zone PF Allowable / Required	Existing	Proposed	Notes
Bldg M					
Front Property Line	15 ft	20 ft	31 ft	31 ft	No change
Side Property Line: South	5 ft	5 ft	5 ft	5 ft	No change
Side Property Line: North	5 ft	5 ft	45 ft	42 ft	Bldg M addition
Rear Property Line	10 ft	20 ft	118 ft	118 ft	No change
Bldg B					
Front Property Line	15 ft	20 ft	- ft	224 ft	Bldg B addition
Side Property Line: South	5 ft	5 ft	- ft	40 ft	Bldg B addition
Side Property Line: North	5 ft	5 ft	- ft	10 ft	Bldg B addition
Rear Property Line	10 ft	20 ft	- ft	10 ft	Bldg B addition

Parking

	Allowable / Required	Existing	Added	Removed	Total Proposed
Standard (on street)	sp	5 sp	sp	sp	5 sp
Van (on street)	sp	1 sp	sp	sp	1 sp
Standard (off street)	sp	19 sp	sp	sp	19 sp
Total	0 sp	25 sp	0 sp	0 sp	25 sp



Santa Rosa - Main Office
3501 Industrial Drive, Suite A
Santa Rosa, CA 95403
707-544-1072

Napa Office
1041 Jefferson St, Suite 4
Napa, CA 94559
707-252-8105

Lakeport Office
149 N. Main Street, Suite 201
Lakeport, CA 95453
707-544-1072

February 23, 2026

Dreiling Terrones Architecture Inc.
Attn: Martin Dreiling
1103 Juanita Ave
Burlingame, CA
md@dtbarch.com

Geotechnical Engineering Report Update
Geyserville Fire Station Addition
20975 Geyserville Ave
Geyserville, California

Project Number: 8246.001.12.1

Introduction

We understand it is proposed to modernize the existing firehouse located at 20975 Geyserville Avenue in Geyserville, California. We performed a geotechnical study for the original construction of the fire station, which included demolishing an existing firehouse and garage, and presented the results in a report dated July 15, 2003 (attached). We provided geotechnical observation and testing services during construction and presented the results in a letter dated September 28, 2005. The planned modernization includes three additions to the existing fire station and a new structure plan reference west of the existing station. The addition portion of the project includes adding a second-floor storage area above the first-floor rooms on the west side of the apparatus bay, a day room north of the kitchen, and a new lobby off the eastern side of the building. The new structure will be one story and includes storage, offices, an exercise room with sauna and toilet, and a garage. The purpose of this letter is to update our original geotechnical report to current design standards and to be applicable to the modernization project.

Site Reconnaissance and Seismic Survey Line

On February 13, 2026, we performed a site reconnaissance and observed that, with the exception of the shipping containers, the site conditions have not changed significantly since construction was completed. During our site reconnaissance, we performed a geophysical site evaluation to determine the Site Class for calculating the seismic design criteria. RGH's geophysical site evaluation included subsurface seismic imaging and earthquake ground shaking potential evaluation using Terēan's VsSurf ReMi™ seismic data processing software. A seismic survey was performed to determine the seismic site class per American Society of Civil Engineers (ASCE) 7-22, titled "Minimum Design Loads and Associated Criteria for Buildings and Other Structures," using the weighted-average soil shear wave velocity for the upper 100 feet (Vs100). The survey was performed by recording active and/or ambient (passive) seismic sources. The seismic

recording array for this survey consisted of 12, 4.5 Hertz (Hz) geophones at 20-foot spacing, for a total survey length of 220 feet. Active noise was generated by 12-pound hammer hits during data acquisition while ambient noise was generated from traffic along the nearby roads. The seismic data were acquired using a ReMiNode™ 2-12 channel seismograph, while data was processed using Terēan's ReMi™ software (terean.com/products). Survey results indicate a weighted-average soil shear wave velocity for the upper 100 feet (V_{s100}) of 1,039 feet per second (ft/s). This results in a designation of a Seismic Site Class CD according to Table 20.2-1 of ASCE 7-22. A detailed description of the seismic survey can be found in the attached shear-wave velocity report.

Conclusions and Recommendations

Based on our review and site evaluation, it is our opinion that the recommendations in our report, with the updated criteria presented below, are valid for design and construction of the improvements.

Seismic Design

Seismic design parameters presented below are based on the 2025 California Building Code (CBC). Based on Table 20.2-1 of ASCE Standard 7-22 and the results of our seismic survey line (attached), we have determined a Site Class of CD should be used for the site. Using a site latitude and longitude of 38.7055°N and 122.9040°W, respectively, the United States Geological Survey web services, the procedures outlined in Section 20.3 of ASCE Standard 7-22, and the results of our seismic survey line, we calculated seismic motions using the measured shear wave velocity for Site Class CD. We recommend that the following seismic design criteria be used for applicable structures at the site designed using the 2025 CBC.

2025 CBC Seismic Criteria	
Spectral Response Parameter	Acceleration (g)
S_s (0.2 second period)	2.730
S_1 (1 second period)	0.940
S_{M5} (0.2 second period)	2.840
S_{M1} (1 second period)	1.820
S_{D5} (0.2 second period)	1.890
S_{D1} (1 second period)	1.220

Retaining Walls

Retaining walls should be designed to resist the following earth equivalent fluid pressures (triangular distribution):

EARTH EQUIVALENT FLUID PRESSURES		
Loading Condition	Pressure (pcf)	Additional Seismic Pressure (pcf)*
Active - Level Backfill	40	23
Active - Sloping Backfill 3:1 or Flatter	53	71
At Rest - Level Backfill	63	3**

* If required

**Per Lew, Sitar, Al Atik, Pourzanjani, and Hudson (2010)

These pressures do not consider additional loads resulting from adjacent foundations or other loads. If these additional surcharge loadings are anticipated, we can assist in evaluating their effects. Where retaining wall backfill is subject to vehicular traffic, the walls should be designed to resist an additional surcharge pressure equivalent to two feet of additional backfill. Recommendations for retaining wall backfill and drainage are presented in the attached report.

Limitations

This report has been prepared by RGH for the exclusive use of the Northern Sonoma County Fire Protection District and Dreiling Terrones Architecture, Inc., and their consultants as an aid in the design and construction of the proposed improvements described in this report.

The validity of the recommendations contained in this report depends upon an adequate testing and monitoring program during the construction phase. Unless the construction monitoring and testing program is provided by our firm, we will not be held responsible for compliance with design recommendations presented in this report and other addendum submitted as part of this report.

Our services consist of professional opinions and conclusions developed in accordance with generally accepted geotechnical engineering principles and practices. We provide no warranty, either expressed or implied. Our conclusions and recommendations are based on the information provided to us regarding the proposed construction, the results of our field exploration, laboratory testing program, and professional judgment. Verification of our conclusions and recommendations is subject to our review of the project plans and specifications, and our observation of construction.

The borings represent the subsurface conditions at the locations and on the date indicated. It is not warranted that they are representative of such conditions elsewhere or at other times. Site conditions and cultural features described in the text of this report are those existing at the time of our field exploration and may not necessarily be the same or comparable at other times.

The scope of our services did not include an environmental assessment or a study of the presence or absence of toxic mold and/or hazardous, toxic or corrosive materials in the soil, surface water, groundwater or air (on, below or around this site), nor did it include an evaluation or study for the presence or absence of wetlands. These studies should be conducted under separate cover, scope and fee and should be provided by a qualified expert in those fields.

We trust this provides the information you require at this time. If you have questions, please contact the project manager.

Very truly yours,
RGH Consultants



Eric G. Chase
Principal Geotechnical Engineer
Project Manager



EGC:JJP:nrm:msh:brw
Electronically submitted

[https://rghgeo.sharepoint.com/sites/shared/shared documents/project files/8000-8250/8246/8246.001.12.1 geyserville fire station addition/.01 - rup/8246.001.12.1 report update.docx](https://rghgeo.sharepoint.com/sites/shared/shared%20documents/project%20files/8000-8250/8246/8246.001.12.1%20geyserville%20fire%20station%20addition/.01%20rup/8246.001.12.1%20report%20update.docx)

Attachments: Shear-Wave Velocity Report

Geotechnical Study, Geyserville Fire Protection District Headquarters, 20975 Geyserville Avenue, Geyserville, California, RGH Project No. 2057.01.04.1, July 15, 2003.

References: Lew, M, Sitar, N, Al Atik, L, Pourzanjani, M, and Hudson, M.B., Seismic Earth Pressures on Deep Building Basements, SEAOC 2010 Convention Proceedings.

Array Location

Datum: WGS84
Site Name: **GeyservilleFireStation_NS_20ft**
Center: 38.70583333, -122.90388889
Geophone 1: 38.70611111, -122.90361111
Geophone12: 38.70555556, -122.90416667

Results

Vs100: **1,039 ft/s**
Site Class: CD
ASCE: ASCE 7-22
Depth: **110 ft**

Survey Parameters

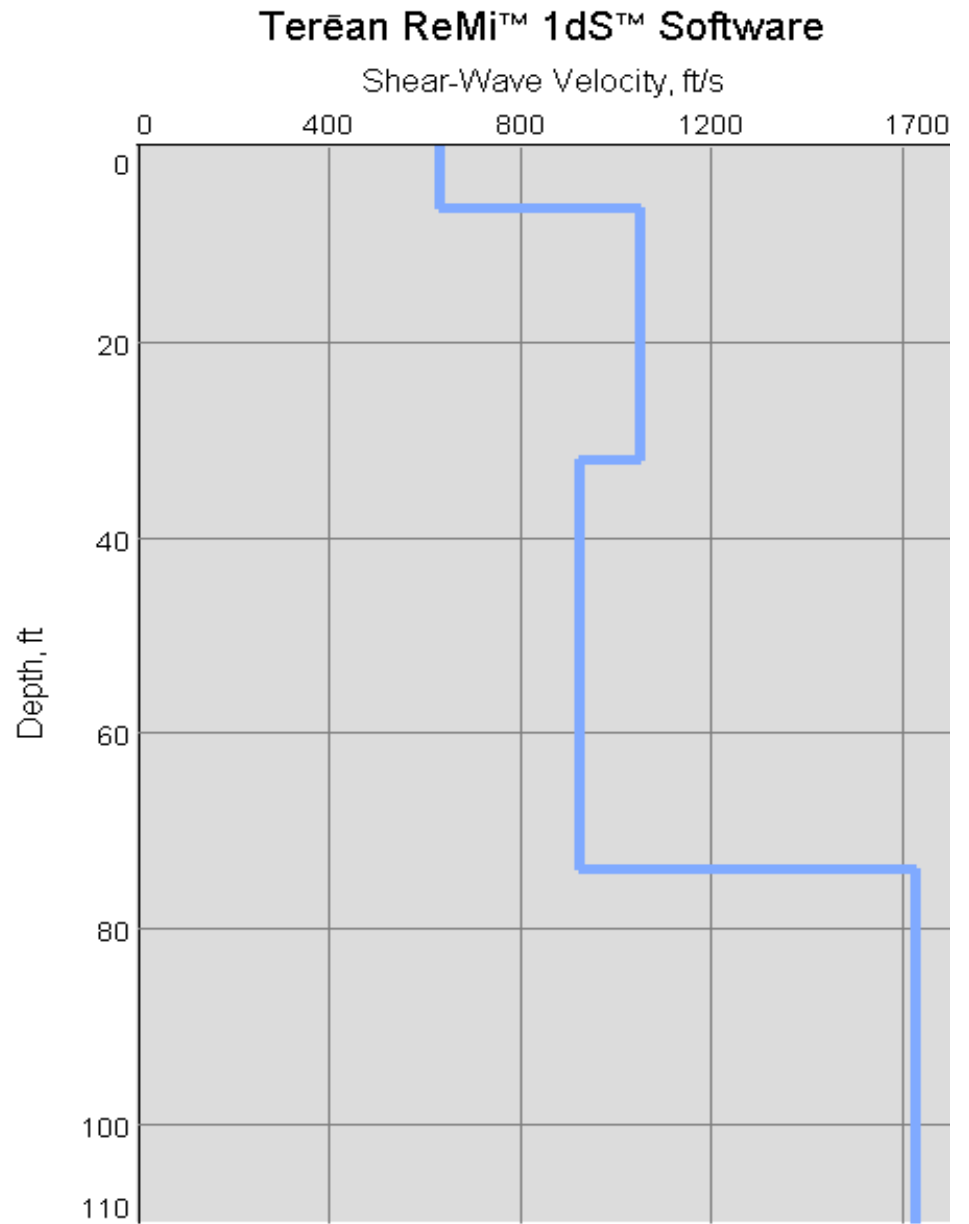
Geophone Count: 12
Geophone Spacing: 20 ft
Array Length: 220 ft
Survey Date: 02/13/2026
Performed By: RGH Consultants
Analysis By: NRM
Analysis Date: Fri Feb 13 13:42:44 PST 2026

Narrative

SHEAR WAVE VELOCITIES AND SEISMIC SITE CLASS

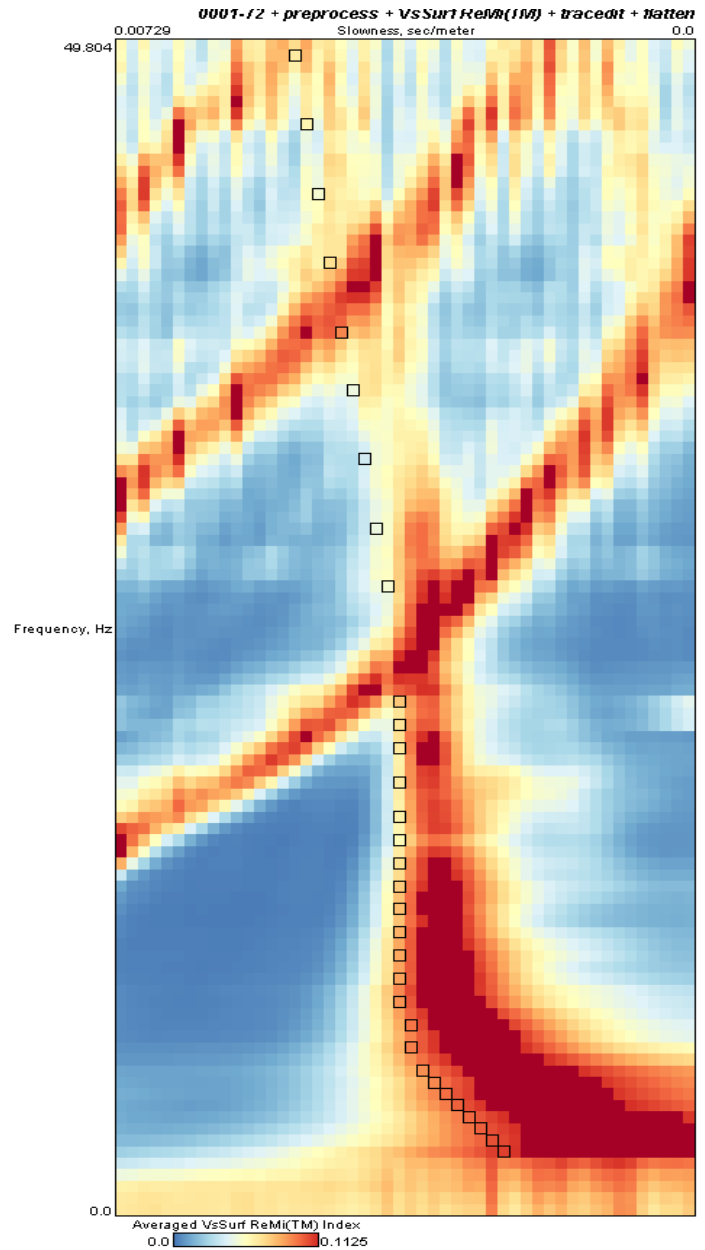
RGH Consultants's geophysical site evaluation included subsurface seismic imaging and earthquake ground shaking potential evaluation using Terēan's VsSurf ReMi™ seismic data processing software. Seismic surveys were performed to determine depth to bedrock and the seismic site class per ASCE 7-22 using the weighted-average soil shear wave velocity for the upper 100 feet (Vs100). The surveys were performed by recording active and/or ambient (passive) seismic sources. The seismic recording array for these surveys consisted of 12, 4.5 Hz geophones at 20 ft spacing, for a total survey length of 220 feet. Noise generated by 12lb hammer hits during data acquisition while ambient noise was generated from traffic along the nearby roads. The seismic data were acquired using a ReMiNode™ 2-12 channel seismograph, while data was processed using Terēan's ReMi™ software (terean.com/products). Survey results indicate a weighted-average soil shear wave velocity of the upper 100 feet (Vs100) of 1,039 ft/s. This results in a designation of a Seismic Site Class CD according to Table ASCE 7-22.

Depth (ft)	Vs (ft/s)
0.0	633
6.3	633
6.3	1,051
31.9	1,051
31.9	925
73.8	925
73.8	1,626
110.0	1,626



Frequency (Hz)	Slowness (s/m)
49.316	0.00510
46.386	0.00495
43.457	0.00481
40.527	0.00466
37.597	0.00452
35.156	0.00437
32.226	0.00422
29.296	0.00408
26.855	0.00393
21.972	0.00379
16.113	0.00379
20.996	0.00379
20.019	0.00379
18.554	0.00379
17.089	0.00379
14.160	0.00379
15.136	0.00379
13.183	0.00379
12.207	0.00379
11.230	0.00379
10.253	0.00379
9.277	0.00379
8.301	0.00364
7.324	0.00364
6.348	0.00349
5.859	0.00335
5.371	0.00320
4.883	0.00306
4.394	0.00291
3.906	0.00277
3.418	0.00262
2.930	0.00247

Seismic File: 0001.sgy
Pre-Processing: preprocess
Surveyed Geophones: No
Max Frequency, Hz: 50
Min Velocity, m/s: 137



R

G

H

REPORT

GEOTECHNICAL STUDY
GEYSERVILLE FIRE PROTECTION DISTRICT HEADQUARTERS
20975 GEYSERVILLE AVENUE
GEYSERVILLE, CALIFORNIA

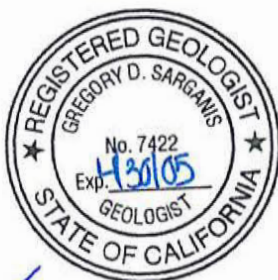
Project Number: 2057.01.04.1

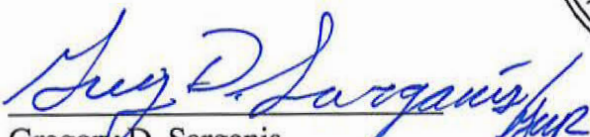
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July 15, 2003

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INTRODUCTION

This report presents the results of our geotechnical study for the Geyserville Fire Protection District Headquarters to be constructed at 20975 Geyserville Avenue in Geyserville, California. The approximately one acre property extends over gently to moderately sloping terrain west of Geyserville Avenue and contains the existing firehouse with an attached garage and a detached outbuilding in the northeastern ¼ of the project. The site location is shown on Plate 1, Appendix A.

We understand that the existing firehouse and garage will be demolished and a new approximately 12,500 square foot fire station will be constructed on the property. The fire station will consist of a structure that has a two-story, wood-framed portion with a high one-story CMU walled truck bay at the rear. Slab-on-grade floors are planned in both portions of the structure. A concrete driveway with ramps and an apron in front of the truck bay will provide vehicle access. Vehicle access will be along the northern edge of the property off Geyserville Avenue. Public parking will be provided along the frontage with Geyserville Avenue and employee parking behind the fire station. Retaining walls will be needed to provide the necessary level breaks across the site. It is also planned to relocate the outbuilding to the southwestern corner of the property.

Actual foundation loads are not known at this time. We anticipate the loads will be typical for the moderately heavy type of construction planned and that wall loads will range from about 1 to 2 kips per lineal foot.

Grading plans are not available, but we anticipate that the planned grading will be the minimum amount needed to construct a level building pad and provide the building site and paved and exterior slab areas with positive drainage. This grading could include cuts and fills on the order of 3 to 6 feet.

SCOPE

The purpose of our study, as outlined in our Professional Service Agreement dated June 11, 2003, was to generate geotechnical information for the design and construction of the project. Our scope of services included reviewing selected published geologic data pertinent to the site; evaluating subsurface conditions with test borings, test pits and laboratory tests; analyzing the field and laboratory data; and presenting this report with the following geotechnical information:

1. A brief description of soil and groundwater conditions observed during our study;
2. A discussion of seismic hazards that may affect the proposed development;
3. Conclusions and recommendations regarding:
 - a. Primary geotechnical engineering concerns and mitigating measures, as applicable;
 - b. Site preparation and grading including remedial grading of weak, porous, compressible surface soils;
 - c. Foundation types, design criteria, and estimated settlement behavior;
 - d. Lateral loads for retaining wall design;
 - e. Support of concrete slabs-on-grade, including modulus of subgrade reaction;
 - f. Utility trench backfill;

- g. Geotechnical engineering drainage improvements; and
- h. Supplemental geotechnical engineering services.

STUDY

Site Exploration

We reviewed our previous geotechnical studies in the vicinity and selected geologic references pertinent to the site. The geologic literature reviewed is listed in Appendix B.

On June 19, 2003, we performed a geotechnical reconnaissance of the site and explored the subsurface conditions by excavating three test pits to depths ranging from about 7 to 9 feet. The test pits were excavated with a rubber-tired backhoe at the approximate locations shown on the Exploration Plan, Plate 2. We returned to the site on June 26, 2003 and further explored the subsurface conditions by drilling four test borings to depths ranging from about 13 to 26½ feet. The borings were drilled with a truck-mounted drill rig equipped with 6-inch diameter, solid stem and 8-inch diameter hollow stem augers, at the approximate locations shown on the Exploration Plan, Plate 2. The test boring and pit locations were determined approximately by pacing their distance from features shown on the Exploration Plan and should be considered accurate only to the degree implied by the method used. Our Registered Geologist located and logged the borings and pits, and obtained samples of the materials encountered for visual examination, classification and laboratory testing. Due to the presence of underground utilities and the existing structures, we did not explore in the asphalt area in front of the existing firehouse or inside the existing firehouse. A majority of the structural improvements occur outside this area.

Relatively undisturbed samples were obtained from the borings at selected intervals by driving a 2.43-inch inside diameter, split spoon sampler, containing 6-inch long brass

liners, using a 140-pound hammer dropping approximately 30 inches. The sampler was driven 12 to 18 inches. The blows required to drive each 6-inch increment were recorded and the blows required to drive the last 12 inches, or portion thereof, were converted to equivalent Standard Penetration Test (SPT) blow counts for correlation with empirical data.

The logs of the test borings and pits showing the materials encountered, groundwater conditions, converted blow counts and sample depths are presented on Plates 3 through 9. The soils are described in accordance with the Unified Soil Classification System, outlined on Plate 10.

The test boring and pit logs show our interpretation of subsurface soil and groundwater conditions on the dates and at the locations indicated. Subsurface conditions may vary at other locations and times. Our interpretation is based on visual inspection of soil samples, laboratory test results, and interpretation of drilling, excavation and sampling resistance. The location of the soil boundaries should be considered approximate. The transition between soil types may be gradual.

Laboratory Testing

The samples obtained from the test borings and pits were transported to our office and re-examined by the project engineer to verify soil classifications, evaluate characteristics, and assign tests pertinent to our analysis. Selected samples were laboratory tested to determine their water content, dry density, classification (Atterberg Limits, percent of silt and clay), unconfined compressive strength, expansion potential (UBC Expansion Index - EI) and R-value. The test results are presented on the test boring and pit logs, with the results of the strength data presented in accordance with the Key to Test Data on Plate 10. Results of the classification, unconfined compression, and R-value tests are presented on Plates 11, 12 and 13, respectively.

In addition to the above geotechnical testing, two samples were sent to ETS for corrosion testing. Results of the corrosion tests are presented in Appendix C.

SITE CONDITIONS

General

Sonoma County is located within the California Coast Range geomorphic province. This province is a geologically complex and seismically active region characterized by sub-parallel northwest-trending faults, mountain ranges and valleys. The oldest bedrock units are the Jurassic-Cretaceous Franciscan Complex and Great Valley sequence sediments originally deposited in a marine environment. Subsequently, younger rocks such as the Tertiary-age Sonoma Volcanics group, the Plio-Pleistocene-age Clear Lake Volcanics and sedimentary rocks such as the Guinda, Domengine, Petaluma, Wilson Grove, Cache, Huichica and Glen Ellen formations were deposited throughout the province. Extensive folding and thrust faulting during late Cretaceous through early Tertiary geologic time created complex geologic conditions that underlie the highly varied topography of today. In valleys, the bedrock is covered by thick alluvial soils. The site is located on the western side of the northern end of the Alexander Valley.

Geology and Soils

The United States Geological Survey (USGS) geologic maps reviewed, Blake et al. 1971), indicate the property is underlain by alluvium (Qal). The alluvium is said to comprise sand, gravel, silt and clay.

Mapping by the U.S. Soil Conservation Service (Miller, 1990) has classified soil over the portion of this property proposed for development as belonging to the Arbuckle series. The Arbuckle series is said to comprise well-drained gravelly sandy loams that have gravelly clay loam subsoil. The Arbuckle series (0 to 5 percent slopes) comprises two soil horizons. The topsoil is shown to be a gravelly sandy loam and loam (ML or SC) that

exhibits low plasticity ($LL = 10-25$; $PI = 0-10$) and low shrink-swell potential, and extends to a depth of 27 inches. The subsoil is shown to be a gravelly clay loam (CL) that exhibits low to moderate plasticity ($LL = 20-40$; $PI = 10-30$) and low to moderate shrink-swell potential, and extends from a depth of 27 to 72 inches. Runoff over these soils is slow. The hazard of erosion is slight depending on slope. The risk of corrosion is given as low to moderate for uncoated steel. Corrosivity tests were performed on two samples from the borings and the results of those tests and a brief commentary are presented in Appendix C.

Landslides

The California Geological Survey's (CGS), formerly known as the California Division of Mines and Geology (CDMG), reconnaissance photo interpretation maps of landslides (Huffman, 1980) reviewed do not indicate slope instability at the project site, and we did not observe landslides at the site during our study.

Surface

The site ascends gently to moderately towards the west from Geyserville Avenue. An existing firehouse with an attached garage is located on the northeastern corner of the property (approximately $\frac{1}{4}$ -acre). The firehouse is accessed by an asphalt paved driveway. A fuel pad is located south of the firehouse. A wood-frame outbuilding is located near the southwest corner of the firehouse. The remaining approximately $\frac{3}{4}$ -acre portion of the site is currently undeveloped but was previously planted with grapevines. The vines are no longer present and the site appears to have been recently tilled or disked. A pile of burned debris was observed in the center of the property. The site is bordered by Geyserville Avenue on the east, remaining vineyard on the south and west and residences on the north.

In general, the ground surface is soft and spongy. This is a condition generally associated with weak, porous and disced surface soils. Natural drainage consists of sheet flow over the ground surface that concentrates in drainage ditches and topographic low areas.

Subsurface

Our test borings, pits and laboratory tests indicate that the portion of the site we studied is blanketed by 1½ to 2 feet of weak, porous, compressible, clayey soils (disced or tilled topsoil). These soils exhibit low plasticity (LL = 30; PI = 11) and low expansion potential (EI = 33). These topsoil materials are underlain by interbedded layers of medium dense to very dense clayey sands and gravels and stiff to very stiff sandy clays to the maximum depth explored (26.5 feet). These soils also exhibit low plasticity (LL = 27; PI = 8) and low expansion potential (EI = 31).

A detailed description of subsurface conditions found in our test pits and borings are given in Plates 3 through 9, Appendix A.

Groundwater

Groundwater seeped into the borings between 10 and 23 feet below the ground surface at the time of drilling and was closer to the surface in the uphill borings than in the downhill borings. On sloping terrain, rainwater typically percolates through the porous surface materials and migrates downslope in the form of seepage at the interfaces of the surface materials and less permeable materials or bedrock. Fluctuations in the seepage rates typically occur due to variations in rainfall intensity, duration and other factors such as periodic irrigation.

Flooding

Our review of the Federal Emergency Management Agency (FEMA) Flood Zone Map for Sonoma County, California, Unincorporated Areas (NO. 060375 0335 B) dated April 2, 1991, indicates that the proposed building site is located within Zone "X," an area outside the 500-year flood plain. Evaluation of flooding potential is typically the responsibility of the project civil engineer.

DISCUSSION AND CONCLUSIONS

Seismic Hazards

General

We did not observe subsurface conditions within the portion of the property we studied that would suggest the presence of materials that may be susceptible to seismically induced densification, liquefaction, or lurching. Therefore, we judge the potential for occurrence of these phenomena at the headquarters site to be low.

Seismicity

Data presented by the Working Group on California Earthquake Probabilities (2002) estimates the chance of one or more large earthquakes (Magnitude 6.7 or greater) in the San Francisco Bay region within the next 30 years to be approximately 62 percent. Therefore, future seismic shaking should be anticipated at the site. It will be necessary to design and construct the proposed headquarters in strict adherence with current standards for earthquake-resistant construction.

Faulting

Maps prepared by CGS (Huffman, 1980) indicate that a trace of the Healdsburg fault is located about 1¼ miles west of the headquarters site. Prior to 1983, CGS Alquist-Priolo Special Studies Zones maps included the Healdsburg fault in their special studies zone for active faults. Subsequently, this fault was removed from zoning. However, studies performed along traces of the Healdsburg fault by private consultants indicate recent activity. As such, we did not observe landforms within the area that would indicate the presence of active faults and the site is not within a current Alquist-Priolo Earthquake Fault Zone. Therefore, the risk of fault rupture at the site is considered to be low. However, the site is within an area affected by strong seismic activity. Several northwest-trending Earthquake Fault Zones exist in close proximity to, and within several miles of the site (Brown, 1970; Helley and Herd, 1977; Bortugno, 1982). The shortest distances from the site to the mapped surface expression of these faults are presented below in Table 1.

TABLE 1
ACTIVE FAULT PROXIMITY

Fault	Direction	Distance-Miles
San Andreas	SW	22
Healdsburg-Rodgers Creek	SE	8
Maacama	NE	3

Liquefaction

Liquefaction is a rapid loss of shear strength experienced in saturated, predominantly granular soils below the groundwater level during strong earthquake ground shaking due to an increase in pore water pressure. The occurrence of this phenomenon is dependent on many complex factors including the intensity and duration of ground shaking, particle size

distribution and density of the soil. As discussed in the "Subsurface" section of the report, granular soils (sand and gravel) are present at the site. An analysis of the blowcounts from the borings compared to the those required to have the potential for liquefaction using the methods of Seed and Idriss (1982), Seed and others (1985) and Youd and Idriss (1997) yield localized pockets of soil with a low susceptibility of liquefaction (Normalized blowcounts greater than 20 blows per foot). In general, these localized soils were not saturated or located below the groundwater surface, which is required for liquefaction to occur. Additionally, the normalized blowcounts were such that the likely result of liquefaction, if it occurred, would be differential settlement of less than 1 inch.

Densification

Densification is the settlement of loose, granular soils above the groundwater level due to earthquake shaking. This typically occurs in soils that, if saturated, would be susceptible to liquefaction. As discussed in the previous section, soils with a low susceptibility to liquefaction are present at the site. Some of these soils may be removed or reworked during site grading and also have a relatively high fines (clay and silt) content. Therefore, we judge that the potential for settlement due to densification is low. If densification were to occur, differential settlement would typically be less than ½-inch.

Geotechnical Issues

General

Based on our study, we judge the proposed Geyserville Fire Protection District Headquarters can be built as planned, provided the recommendations presented in this report are incorporated into its design and construction. The primary geotechnical concerns during design and construction of the project are:

1. The presence of 1 to 2 feet of weak, porous, compressible, surface clayey soils.
2. The detrimental effects of uncontrolled surface runoff and groundwater seepage on the long-term satisfactory performance of firehouses given the porous nature of the surface soils.
3. The strong ground shaking predicted to impact the site during the life of the project.

Weak, Porous Surface Soils

Weak, porous surface soils, such as those found at the headquarters' site, appear hard and strong when dry but will lose strength rapidly and settle under the load of fills, foundations, slabs, and pavements as their moisture content increases and approaches saturation. The moisture content of these soils can increase as the result of rainfall, periodic irrigation or when the natural upward migration of water vapor through the soils is impeded by, and condenses under fills, foundations, slabs, and pavements. The detrimental effects of such movements can be remediated by strengthening the soils during grading. This can be achieved by excavating the weak soils and replacing them as properly compacted (engineered) fill. Satisfactory foundation support can also be obtained below the weak surface soils if so desired.

Foundation, Slab and Pavement Support - After remedial grading, satisfactory foundation support for the headquarters can be obtained from spread footings bottomed on the engineered fill. Slab-on-grade floors and pavements can also be satisfactorily supported on the engineered fill.

We understand that drilled pier foundations may be required for some of the proposed structures. This would consist of a system of grade beams supported on drilled piers that gain support below the weak surface materials. With this alternative, it will not be

necessary to remove and recompact the weak surface materials within living areas provided that:

- (1) Wood floors supported on joist above grade are used in living areas; and
- (2) The weak soils are removed and recompactd in their entirety in concrete slab-on-grade and paved areas.

On-Site Soil Quality

All fill materials used in the upper 30 inches of the building area and the upper 12 inches of exterior slab and pavement subgrade must be select, as subsequently described in "Recommendations." We anticipate that, with the exception of organic matter and of rocks or lumps larger than 6 inches in diameter, the excavated material will be suitable for re-use as general and select fill.

Select Fill

The select fill can consist of approved on-site soils or import materials with a low expansion potential. The geotechnical engineer must approve the use of on-site and/or import soils as select fill during grading.

Settlement

Provided remedial grading is performed as recommended herein, we estimate that total settlements of heavily loaded column footings will be about 1-inch and settlement of the strip footings will be ½-inch. We estimate that post-construction differential settlements between columns and lightly loaded perimeter footings will be about ½-inch.

Surface Drainage

Because of topography, the site will be impacted by surface runoff from the upgradient slopes. Surface runoff typically sheet flows over the ground surface but can be concentrated by the planned site grading, landscaping, and drainage. The surface runoff can pond against structures and seep into the slab rock. Therefore, strict control of surface runoff is necessary to provide long-term satisfactory performance of projects constructed on or near hillsides. It will be necessary to divert surface runoff around improvements and provide positive drainage away from structures. This can be achieved by constructing the building pad several inches above the surrounding area and conveying the runoff into man made drainage ditches or natural swales that lead downgradient of the site.

Groundwater

We anticipate that rainwater will percolate through the porous topsoil and migrate downslope at the interface of the topsoil and the less permeable subsoil. Therefore, it will be necessary to intercept, collect and divert groundwater outside of the proposed improvements. This can be accomplished by installing retaining wall backdrains and slab underdrains as recommended herein.

RECOMMENDATIONS

Seismic Design

The site is within 1997 Uniform Building Code (UBC) (ICBO, 1997) seismic zone 4; therefore, a Seismic Zone Factor "Z" of 0.4 (Table 16-I), modified as necessary to conform to ordinance(s) adopted by the County of Sonoma, should be used. The soil profile

at the site approximates type S_D (Table 16-J). The 1997 UBC has identified the locations of known active fault near-source zones in California and along the California/Nevada border. The purpose of these zones is to determine a near-source factor to be used in design for every site located within Seismic Zone 4. The near-source zones have been mapped considering the surface projection of the source (as opposed to the mapped surface expression of the fault) using the dip angle of the fault. For non-vertical faults, the dip of the fault is an important parameter because it determines the location of the fault at depth.

Active faults have been classified as A, B, or C in accordance with the criteria specified in the 1997 UBC (Table 16-U). Only faults classified as A or B are shown on the 1997 UBC maps (ICBO, 1998) because the UBC assumes faults classified as C do not increase the near-source factor. The distance from the site to the A and B faults that have the potential to control the near-source factors are listed in Table 2 below. The distances shown in Table 2 are not converted from Table 1, presented earlier. Each table measures different distances, as previously discussed. It should be noted that the Healdsburg fault does not have a UBC near-source zone because it is not considered active by CGS. However, as discussed previously, there is evidence of recent activity. The Healdsburg fault, if considered active, would most likely be a Type B fault and within $2\frac{1}{2}$ km of the headquarters property.

TABLE 2
1997 UBC NEAR-SOURCE ZONE DISTANCES

Fault	Direction	Fault Type	Distance-km
Rodgers Creek	SE	B	>15
Maacama (south)	NE	B	4.2

Using Tables 16-S and 16-T of the 1997 UBC and Table 2, the near-source factors, N_a and N_v , for the site are 1.1 and 1.3, respectively. As discussed above, the site is likely

within 2½ km of the Healdsburg fault (Type B). Therefore, at the owner's discretion, it may be prudent to increase the near-source factors, N_a and N_v , to 1.3 and 1.6, respectively. The project structural engineer should determine the appropriate seismic response coefficients (C_a and C_v) needed to determine total design lateral force in accordance with the UBC.

Grading

Site Preparation

Areas to be developed should be cleared of vegetation and debris including that left by the removal of obsolete structures. Trees and shrubs that will not be part of the proposed development should be removed and their primary root systems grubbed. Cleared and grubbed material should be removed from the site and disposed of in accordance with County Health Department guidelines. We did not observe septic tanks, leach lines or underground fuel tanks during our study. Any such appurtenances found during grading should be capped and sealed and/or excavated and removed from the site, respectively, in accordance with established guidelines and requirements of the County Health Department. Voids created during clearing should be backfilled with engineered fill as recommended herein.

Stripping

Areas to be graded should be stripped of the upper few inches of soil containing organic matter. Soil containing more than two percent by weight of organic matter should be considered organic. Actual stripping depth should be determined by a representative of the geotechnical engineer in the field at the time of stripping. The strippings should be removed from the site, or if suitable, stockpiled for re-use as topsoil in landscaping.

Excavations

Following initial site preparation, excavation should be performed as planned or recommended herein. Excavations extending below the proposed finished grade should be backfilled with suitable materials compacted to the requirements given below.

Within fill, building, exterior slab, and pavement areas, the weak, porous, compressible surface soils should be excavated to within 6 inches of their entire depth (about 1½ to 2 feet in our test borings and pits). The excavation of weak, porous, compressible, surface materials should extend at least 5 feet beyond the outside edge of the exterior footings of the proposed buildings and 3 feet beyond the edge of exterior slabs and/or pavements. The excavated materials should be stockpiled for later use as compacted fill, or removed from the site, as applicable. The grading contractor should review this report, become familiar with site conditions as they pertain to his operation and draw his own conclusions regarding excavation characteristics and suitable grading equipment.

At all times, temporary construction excavations should conform to the regulations of the State of California, Department of Industrial Relations, Division of Industrial Safety or other stricter governing regulations. The stability of temporary cut slopes, such as those constructed during the installation of underground utilities, should be the responsibility of the contractor. Depending on the time of year when grading is performed, and the surface conditions exposed, temporary cut slopes may need to be excavated to 1¼:1, or flatter. The tops of the temporary cut slopes should be rounded back to 2:1 in weak soil zones.

Fill Quality

All fill materials should be free of perishable matter and rocks or lumps over 6 inches in diameter and must be approved by the geotechnical engineer prior to use. The upper 30 inches of fill beneath and within 5 feet of the building area (building footprint) and the upper 12 inches of fill beneath and within 3 feet of exterior slabs and/or pavement edges

should be select fill. We judge the on-site soils are generally suitable for use as general and select fill.

Select Fill

Select fill should be free of organic matter, have a low expansion potential, and conform in general to the following requirements:

SIEVE SIZE	PERCENT PASSING (By Dry Weight)
6 inch	100
4 inch	90 - 100
No. 200	10 - 60
Liquid Limit - 40 Percent Maximum	
Plasticity Index - 15 Percent Maximum	
R-value - 20 Minimum	

In general, imported fill, if needed, should be select. Material not conforming to these requirements may be suitable for use as import fill; however, it shall be the contractor's responsibility to demonstrate that the proposed material will perform in an equivalent manner. The geotechnical engineer should approve imported materials prior to use as compacted fill.

Fill Placement

The surface exposed by stripping and removal of weak, compressible surface soils should be scarified to a depth of at least 6 inches, uniformly moisture-conditioned to near optimum and compacted to at least 90 percent of the maximum dry density of the materials as determined by ASTM Test Method D-1557. Approved fill material should then be spread in thin lifts, uniformly moisture-conditioned to near optimum and properly

compacted. All structural fills, including those placed to establish site surface drainage, should be compacted to at least 90 percent relative compaction. Only approved select materials should be used for fill within the upper 30 inches of interior slab subgrades and within the upper 12 inches of exterior slabs and/or pavement subgrades.

Permanent Cut and Fill Slopes

In general, cut and fill slopes should be no steeper than 2:1. Where steeper slopes are required, retaining walls should be used.

Wet Weather Grading

Generally, grading is performed more economically during the summer months when on-site soils are usually dry of optimum moisture content. Delays should be anticipated in site grading performed during the rainy season or early spring due to excessive moisture in on-site soils. Special and relatively expensive construction procedures, including dewatering of excavations and importing granular soils, should be anticipated if grading must be completed during the winter and early spring or if localized areas of soft saturated soils are found during grading in the summer and fall.

Open excavations also tend to be more unstable during wet weather as groundwater seeps towards the exposed cut slope. Severe sloughing and occasional slope failures should be anticipated. The occurrence of these events will require extensive clean up and the installation of slope protection measures and thus delay projects. The general contractor is responsible for the performance, maintenance and repair of temporary cut slopes.

Foundation Support

Provided the weak surface soils are removed by or strengthened by remedial grading as recommended herein, the proposed and relocated structure can be supported on continuous and isolated spread footings that bottom on firm, natural soils or select engineered fill.

Spread Footings

Spread footings should be at least 12 inches wide and should bottom on firm, natural soils or select engineered fill, as applicable, at least 12 inches below lowest adjacent grade. Because of the potential for uneven soil support, continuous footings (except for retaining walls) should have sufficient reinforcement to span, as a simple beam, an unsupported distance of approximately 10 feet.

The bottoms of all footing excavations should be thoroughly cleaned out or wetted and compacted using hand-operated tamping equipment prior to placing steel and concrete. This will remove the soils disturbed during footing excavations, or restore their adequate bearing capacity, and reduce post-construction settlements.

Bearing Pressures - Footings installed in accordance with these recommendations may be designed using allowable bearing pressures of 2000, 2500 and 3000 pounds per square foot (psf), for dead loads, dead plus code live loads, and total loads (including wind and seismic), respectively, for both engineered fill and firm natural soils.

Lateral Pressures - The portion of spread footing foundations extending into firm natural soil or select engineered fill may impose a passive equivalent fluid pressure and a friction factor of 350 pcf and 0.35, respectively, to resist sliding. Because full mobilization of the passive resistance requires some horizontal movement (diminished frictional resistance), the frictional component should be reduced by 50 percent if both passive

pressure and friction are used simultaneously. Passive pressure should be neglected within the upper 6 inches, unless the soils are confined by concrete slabs or pavements.

Drilled Piers

As requested by the project structural engineer, we are providing recommendations for drilled, cast-in-place, reinforced concrete piers. Drilled piers should be at least 12 inches in diameter and extend 8 feet into competent soil.

Skin Friction - The portion of the piers extending below the weak surface soils may be designed using an allowable skin friction of 500 psf for dead load plus long term live loads. This value can be increased by $\frac{1}{3}$ for total loads, including wind or seismic forces. End bearing should be neglected because of the difficulty of cleaning out small diameter pier holes, and the uncertainty of mobilizing end bearing and skin friction simultaneously.

Lateral Forces - Lateral loads on piers will be resisted by passive pressure on the soil. An allowable equivalent fluid pressure of 300 pcf acting on two pier diameters should be used for design using the California Building Code pole equations. Confinement for passive pressure may be assumed from 2 feet below the lowest adjacent finished ground surface, as applicable.

The piers should be interconnected with grade beams to support building loads and to redistribute stresses imposed by wind or earthquakes. The grade beams should be designed to span between the piers in accordance with structural requirements. The steel from the piers should extend sufficient distance into the grade beams to develop its full bond strength.

Pier Drilling - We encountered groundwater and/or caving-prone soils within the planned pier depth during our study. If groundwater is encountered during drilling, it may be necessary to de-water the holes and/or place the concrete by the tremmie method. If

caving soils are encountered, it may be necessary to case the holes. The drilling subcontractor should review this report, become familiar with site conditions as they pertain to his operation and draw his own conclusions regarding drilling characteristics, suitable drill rigs and the need for casing and dewatering prior to bidding.

Concrete - Concrete mix design and placement should be done in accordance with the current ADSC and/or ACI specifications. Concrete should not be allowed to mushroom at the top of the piers or below the bottom of grade beams.

Retaining Walls

Retaining walls constructed at the site must be designed to resist lateral earth pressures plus additional lateral pressures that may be caused by surcharge loads applied at the ground surface behind the walls.

Retaining walls free to rotate (yielding greater than 0.1 percent of the wall height at the top of the backfill) should be designed for active lateral earth pressures. If walls are restrained by rigid elements to prevent rotation, they should be designed for "at rest" lateral earth pressures.

Retaining walls should be designed to resist the following earth equivalent fluid pressures (triangular distribution):

Active Pressure (level backfill)	40 pcf
Active Pressure (3:1 or steeper backfill)	60 pcf
At Rest Pressure.....	70 pcf

These pressures do not consider additional loads resulting from adjacent foundations or other loads. If these additional surcharge loadings are anticipated, we can assist in evaluating their effects.

Where retaining wall backfill is subject to vehicular traffic, the walls should be designed to resist an additional surcharge pressure equivalent to two feet of additional backfill. Retaining walls will yield slightly during backfilling. Therefore, walls should be backfilled prior to building on or adjacent to the walls.

Backfill against retaining walls should be compacted to at least 90 and not more than 95 percent relative compaction. Over-compaction or the use of large compaction equipment should be avoided because increased compactive effort can result in lateral pressures higher than those recommended above.

Foundation Support

Retaining walls should be supported on spread footings or drilled piers, as applicable, designed in accordance with the recommendations presented in this report. Retaining wall foundations should be designed by the project civil or structural engineer to resist the lateral forces set forth in this section.

Wall Drainage and Backfill

Retaining walls should be backdrained as shown on Plate 14, Appendix A. The backdrains should consist of 4-inch diameter, rigid perforated pipe embedded in Class 2 permeable material. The pipe should be PVC Schedule 40 or ABS with SDR 35 or better, and the pipe should be sloped to drain to outlets by gravity. The top of the pipe should be at least 8 inches below lowest adjacent grade. The Class 2 permeable material should extend to within 1½ feet of the surface. The upper 1½ feet should be backfilled with compacted soil to exclude surface water. Expansive soils should not be used for wall backfill. The ground surface behind retaining walls should be sloped to drain. Where migration of moisture through retaining walls would be detrimental, retaining walls should be waterproofed.

Slab-On-Grade

Provided grading is performed in accordance with the recommendations presented herein, slabs should be underlain by firm, natural soil and/or select engineered fill. Slab-on-grade subgrade should be rolled to produce a dense, uniform surface. The slabs should be underlain with a capillary moisture break consisting of at least 4 inches of clean, free-draining crushed rock or gravel (excluding pea gravel) at least 1/4-inch and no larger than 3/4-inch in size. Interior slabs subject to vehicular traffic should be underlain by crushed rock. Class 2 aggregate base can be used for slab rock under exterior slabs. Where migration of moisture vapor through slabs would be detrimental, an impermeable membrane moisture vapor barrier should be provided between the drain rock and the slabs. Outlets should be provided for the interior slab rock to drain through foundation walls into a perimeter subdrain. As an alternative, interior slabs should be provided with an underdrain system. The installation of these alternative subdrain systems is discussed in "Geotechnical Drainage Improvements."

Slabs should be at least 4 inches thick, and should be designed by the project civil or structural engineer to support the anticipated loads and reduce cracking. Slabs subjected to heavy concentrated wheel loads, such as forklift or trailer-trucks, should be designed to carry the anticipated wheel loads. A modulus of subgrade reaction of 80 pound per cubic inch should be used for design. Slabs should be grooved at regular intervals to induce and control cracking.

Utility Trenches

The shoring and safety of trench excavations is solely the responsibility of the contractor. Attention is drawn to the State of California Safety Orders dealing with "Excavations and Trenches."

Unless otherwise specified by the County of Sonoma, on-site, inorganic soil may be used as general utility trench backfill. Where utility trenches support pavements, slabs and foundations, trench backfill should consist of aggregate baserock. The baserock should comply with the minimum requirements in Caltrans Standard Specifications, Section 26 for Class 2 Aggregate Base. Trench backfill should be moisture-conditioned as necessary, and placed in horizontal layers not exceeding 8 inches in thickness, before compaction. Each layer should be compacted to at least 90 percent relative compaction as determined by ASTM Test Method D-1557. The top 6 inches of trench backfill below vehicle pavement subgrades should be moisture-conditioned as necessary and compacted to at least 95 percent relative compaction. Jetting or ponding of trench backfill to aid in achieving the recommended degree of compaction should not be attempted.

Pavements

An R-value of 17 was measured on a bulk sample of near-surface soil obtained in the central portion of the site (between Borings 1 and 3). Because of potential variation in the on-site soils, we selected an R-value of 15 for use in pavement design calculations. Based on the selected R-value, we have computed pavement sections for Traffic Indices (TI) ranging from 4.5 to 7.0 presented below. The project engineer, in consultation with County officials, should choose the pertinent TI for this project.

TABLE 3
PAVEMENT SECTIONS

TI	THICKNESS (feet)		
	ASPHALT CONCRETE	CLASS 2 AGGREGATE BASE	AGGREGATE SUBBASE
7.0	0.30	1.15	0.0
6.5	0.30	1.00	0.0
6.0	0.25	0.95	0.0
5.5	0.25	0.8	0.0
5.0	0.20	0.8	0.0
4.5	0.20	0.65	0.0

Pavement thicknesses were computed using Method 301 F of the Caltrans Highway Design Manual and are based on a pavement life of 20 years.

These recommendations are intended to provide support for the auto and light truck traffic represented by the indicated Traffic Indexes. They are not intended to provide pavement sections for heavy concentrated construction storage or wheel loads such as forklifts, parked truck-trailers and concrete trucks or for post-construction concentrated wheel loads such as self-loading dumpster trucks or fire trucks. We understand that the fire trucks will use the concrete slab-on-grade driveway and apron.

In areas where heavy construction storage and wheel loads are anticipated, the pavements should be designed to support these loads. Support could be provided by increasing pavement sections or by providing reinforced concrete slabs. Alternatively, paving can be deferred until heavy construction storage and wheel loads are no longer present. Loading areas for self-loading dumpster trucks should be provided with reinforced concrete slabs at least 6 inches thick, and reinforced with No. 4 bars at 12-inch centers each way. Alternatively, the asphalt concrete section should be increased to at least 8 inches in these areas.

Prior to placement of aggregate base, the upper 6 inches of the pavement subgrade soils should be scarified, uniformly moisture-conditioned to near optimum, and compacted to at least 95 percent relative compaction to form a firm, non-yielding surface. Aggregate base materials should be spread in thin layers, uniformly moisture-conditioned, and compacted to at least 95 percent relative compaction to form a firm, non-yielding surface. The materials and methods used should conform to the requirements of the County of Sonoma and the current edition of the Caltrans Standard Specifications, except that compaction requirements should be based on ASTM Test Method D-1557. Aggregate used for the base course should comply with the minimum requirements specified in Caltrans Standard Specifications, Section 26 for Class 2 Aggregate Base.

Wet Weather Paving

In general, the pavements should be constructed during the dry season to avoid the saturation of the subgrade and base materials, which often occurs during the wet winter months. If pavements are constructed during the winter, a cost increase relative to drier weather construction should be anticipated. Unstable areas may have to be overexcavated to remove soft soils. The excavations will probably require backfilling with imported crushed (ballast) rock. The geotechnical engineer should be consulted for recommendations at the time of construction.

Geotechnical Drainage

Surface water should be diverted away from slopes, foundations and edges of pavements. Surface drainage gradients within 5 feet of building foundations should be constructed with a minimum slope of 2 percent for paved areas and 4 percent for unpaved areas. Where a flatter gradient is required to satisfy design constraints, area drains should be installed within the rear and side yard swales with a spacing no greater than about 20

feet. Roofs should be provided with gutters and the downspouts should empty onto splash blocks that discharge directly onto paved areas or be connected to closed (glued Schedule 40 PVC or better) conduits discharging well away from foundations, preferably onto paved areas or erosion resistant natural drainages, or into the site's surface drainage system.

Where interior slab subgrades are less than 6 inches above adjacent exterior grade and where mitigation of moisture through the slab would be detrimental, slab underdrains should be installed to prevent surface runoff from entering the slab rock. Slab underdrains should consist of trenches that extend at least 6 inches below the bottom of the slab rock and slope to drain by gravity. The slab underdrain trenches should be at least 6 inches wide and spaced no further than 15 feet, both ways. Additional drain trenches should be installed, as necessary, to drain all isolated under slab areas. Three-inch diameter perforated pipe sloped to drain to outlets by gravity should be placed in the bottom of the trenches. Slab underdrain trenches should be backfilled to subgrade level with clean, free draining slab rock. An illustration of this system is shown on Plate 15. Perimeter subdrains may be deleted where slab underdrains are used. Conversely, slab underdrains may be deleted where the slab rock is drained through sleeves that are cast through the foundations and outletted into a perimeter subdrain. If slab underdrains are not used, it should be anticipated that water will enter the slab rock, permeate through the concrete slab and ruin floor coverings.

Roof downspouts and surface drains must be maintained entirely separate from foundation drains, slab underdrains and retaining wall backdrains.

Water seepage or the spread of extensive root systems into the soil subgrade of footings, slabs or pavements could cause differential movements and consequent distress in these structural elements. Landscaping should be planned with consideration for these potential problems.

Maintenance

Periodic land maintenance will be required. Surface and subsurface drainage facilities should be checked frequently, and cleaned and maintained as necessary. A dense growth of deep-rooted ground cover must be maintained on all slopes to reduce sloughing and erosion. Sloughing and erosion that occurs must be repaired promptly before it can enlarge.

Supplemental Services

RGH Geotechnical and Environmental Consultants (RGH) recommends that we be retained to review the project plans and specifications to determine if they are consistent with our recommendations. In addition, we should be retained to observe construction, particularly site excavations, compaction of fills and backfills, foundation and subdrain installations, and perform field and laboratory testing.

If, during construction, we observe subsurface conditions different from those encountered during the explorations, we should be allowed to amend our recommendations accordingly. If different conditions are observed by others, or appear to be present beneath excavations, RGH should be advised at once so that these conditions may be evaluated and our recommendations reviewed and updated, if warranted. The validity of recommendations made in this report is contingent upon our being notified and retained to review the changed conditions.

If more than 18 months have elapsed between the submission of this report and the start of work at the site, or if conditions have changed because of natural causes or construction operations at, or adjacent to, the site, the recommendations made in this report may no longer be valid or appropriate. In such case, we recommend that we be retained to review this report and verify the applicability of the conclusions and recommendations or

modify the same considering the time lapsed or changed conditions. The validity of recommendations made in this report is contingent upon such review.

These supplemental services are performed on an as-requested basis and are in addition to this geotechnical study. We cannot accept responsibility for items that we are not notified to observe or for changed conditions we are not allowed to review.

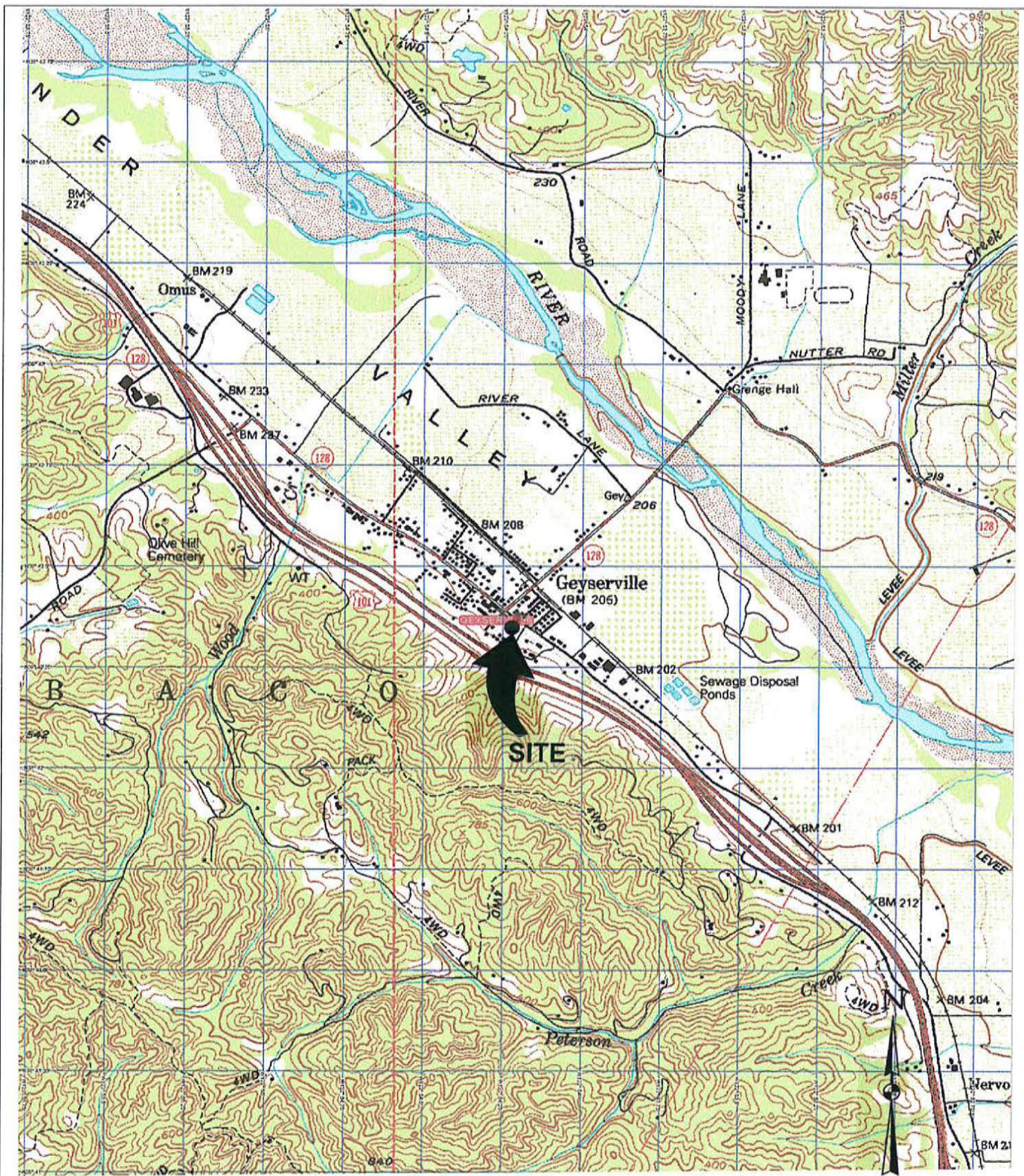
LIMITATIONS

This report has been prepared by RGH for the exclusive use of the Geyserville Fire Protection district and their consultants as an aid in the design and construction of the proposed headquarters described in this report.

The validity of the recommendations contained in this report depends upon an adequate testing and monitoring program during the construction phase. Unless the construction monitoring and testing program is provided by our firm, we will not be held responsible for compliance with design recommendations presented in this report and other addendum submitted as part of this report.

Our services consist of professional opinions and conclusions developed in accordance with generally accepted geotechnical engineering principles and practices. We provide no other warranty, either expressed or implied. Our conclusions and recommendations are based on the information provided to us regarding the proposed construction, the results of our field exploration, laboratory testing program, and professional judgment. Verification of our conclusions and recommendations is subject to our review of the project plans and specifications, and our observation of construction.

The test borings and pits represent subsurface conditions at the locations and on the date indicated. It is not warranted that they are representative of such conditions elsewhere or at other times. Site conditions and cultural features described in the text of this report are



Reference: U.S.G.S. 3-D TopoQuad 7½-Minute Geyserville, California Quadrangle

Scale: 1" = 2,000'

R G H

Geotechnical and
Environmental
Consultants

Job No: 2057.01.04.1

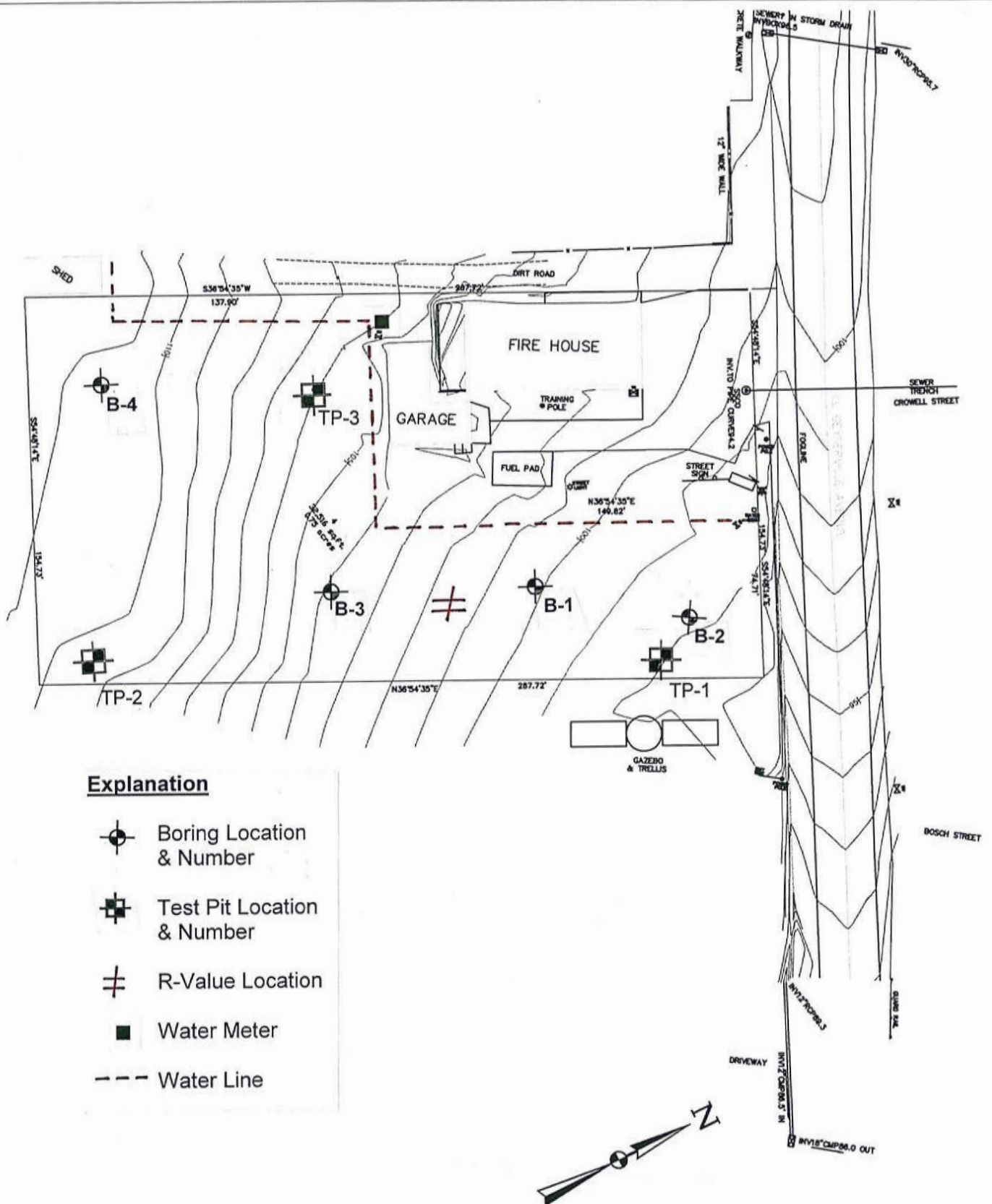
Appr: *esc*

Drwn: kabs

Date: July 2003

SITE LOCATION MAP
GEYSERVILLE FIRE PROTECTION DISTRICT
HEADQUARTERS - 20975 GEYSERVILLE AVE.
Geyserville, California

PLATE
1



Reference: Topographic Map by Crabtree Land Surveying, dated 08/31/02

Not to Scale

R G H

Geotechnical and
Environmental
Consultants

Job No: 2057.01.04.1

Appr: *EJC*

Drwn: kabs

Date: July 2003

EXPLORATION PLAN
GEYSERVILLE FIRE PROTECTION DISTRICT
HEADQUARTERS - 20975 GEYSERVILLE AVE.
Geyserville, California

PLATE
2

OTHER LABORATORY TESTS	DRY DENSITY (pcf)	MOISTURE (%)	BLOWS/ FOOT*	DEPTH (ft)	SAMPLES	SYMBOLS	MATERIALS DESCRIPTION
							BROWN SANDY CLAY (CL) medium stiff, dry; very porous, w/ gravel, disced to 1½ feet (Topsoil)
							MOTTLED BROWN CLAYEY SAND (SC) medium dense, moist; very porous, w/ some gravel that includes friable siltstone fragments and volcanics, and hard graywacke fragments
				5			MOTTLED BROWN CLAYEY GRAVEL (GC) medium dense, moist; w/ trace of free water
							NO FREE GROUNDWATER OBSERVED
				10			**ELEVATION - Existing Ground Surface

PROJECT	Geyserville Fire Protection	EXCAVATION COMPANY	Pearson Exploration	PLATE
LOCATION	District Headquarters, Geyserville	DATE EXCAVATED	06/19/03	3
JOB NUMBER	2057.01.04.1	SURFACE ELEVATION**	98 Feet	
LOGGED BY	GS	BOTTOM OF PIT	8.5 Feet	
DRILL RIG	Rubber-Tired Backhoe: 24" Bucket	APPROVED BY	<i>EJC</i>	

OTHER LABORATORY TESTS	DRY DENSITY (pcf)	MOISTURE (%)	BLOWS/ FOOT*	DEPTH (ft)	SAMPLES	SYMBOLS	MATERIALS DESCRIPTION
							MOTTLED BROWN & GRAY SANDY GRAVEL (GP) medium dense, dry in upper 30 inches; porous, w/ small, well-rounded cobbles and gravels <i>Caving pit sidewalls from 2-4½ feet</i>
				5			MOTTLED YELLOW & REDDISH-BROWN GRAVELLY CLAY & CLAYEY GRAVEL (CL-GC) very stiff and dense, moist; contains friable siltstone and sandstone fragments
				10			NO FREE GROUNDWATER OBSERVED **ELEVATION - Existing Ground Surface

PROJECT <u>Geyserville Fire Protection</u>	EXCAVATION COMPANY <u>Pearson Exploration</u>	PLATE 4
LOCATION <u>District Headquarters, Geyserville</u>	DATE EXCAVATED <u>06/19/03</u>	
JOB NUMBER <u>2057.01.04.1</u>	SURFACE ELEVATION** <u>109.5 Feet</u>	
LOGGED BY <u>GS</u>	BOTTOM OF PIT <u>9 Feet</u>	
DRILL RIG <u>Rubber-Tired Backhoe: 24" Bucket</u>	APPROVED BY <u>EJC</u>	

OTHER LABORATORY TESTS	DRY DENSITY (pcf)	MOISTURE (%)	BLOWS/ FOOT*	DEPTH (ft)	SAMPLES	SYMBOLS	MATERIALS DESCRIPTION
				5			MOTTLED BROWN CLAYEY SAND (SC) medium dense, dry in upper 1 foot, moist below 1 foot; porous, w/ some gravel, roots in upper 8"
				10			NO FREE GROUNDWATER OBSERVED
							**ELEVATION - Existing Ground Surface

PROJECT	Geyersville Fire Protection	EXCAVATION COMPANY	Pearson Exploration	PLATE 5
LOCATION	District Headquarters, Geyersville	DATE EXCAVATED	06/19/03	
JOB NUMBER	2057.01.04.1	SURFACE ELEVATION**	106 Feet	
LOGGED BY	GS	BOTTOM OF PIT	7 Feet	
DRILL RIG	Rubber-Tired Backhoe: 24" Bucket	APPROVED BY	<i>EJE</i>	

OTHER LABORATORY TESTS	DRY DENSITY (pcf)	MOISTURE (%)	BLOWS/ FOOT*	DEPTH (ft)	SAMPLES	SYMBOLS	MATERIALS DESCRIPTION
Strength Test (See Plate 12)	111	12.5	19		■	▨	BROWN SANDY CLAY (CL) very stiff, dry; porous, w/ gravel, disced to 1½ feet (Topsoil)
Strength Test (See Plate 12)	114	15.3	15		■	▨	MOTTLED BROWN SANDY CLAY (CL) stiff to very stiff, moist; w/ small siltstone fragments and gravel
	119	11.2	27	5	■	•	MOTTLED BROWN CLAYEY SAND (SC) medium dense, moist; w/ gravels that include hard sandstone and friable volcanics rocks and siltstone fragments
	116	14.9	24	10	■	•	
				15	■	▨	MOTTLED BROWN SANDY CLAY (CH) very stiff, moist;
			29		■	▨	
				20			▽ Water Encountered @ 20 Feet @ 1030 Hours
			15 31		■	▨	Spongy, fine sandy clay @ 21 feet
				25	■	•	MOTTLED BROWN GRAVELLY SAND (SP) dense to very dense, wet;
			50+		■	•	
				30			

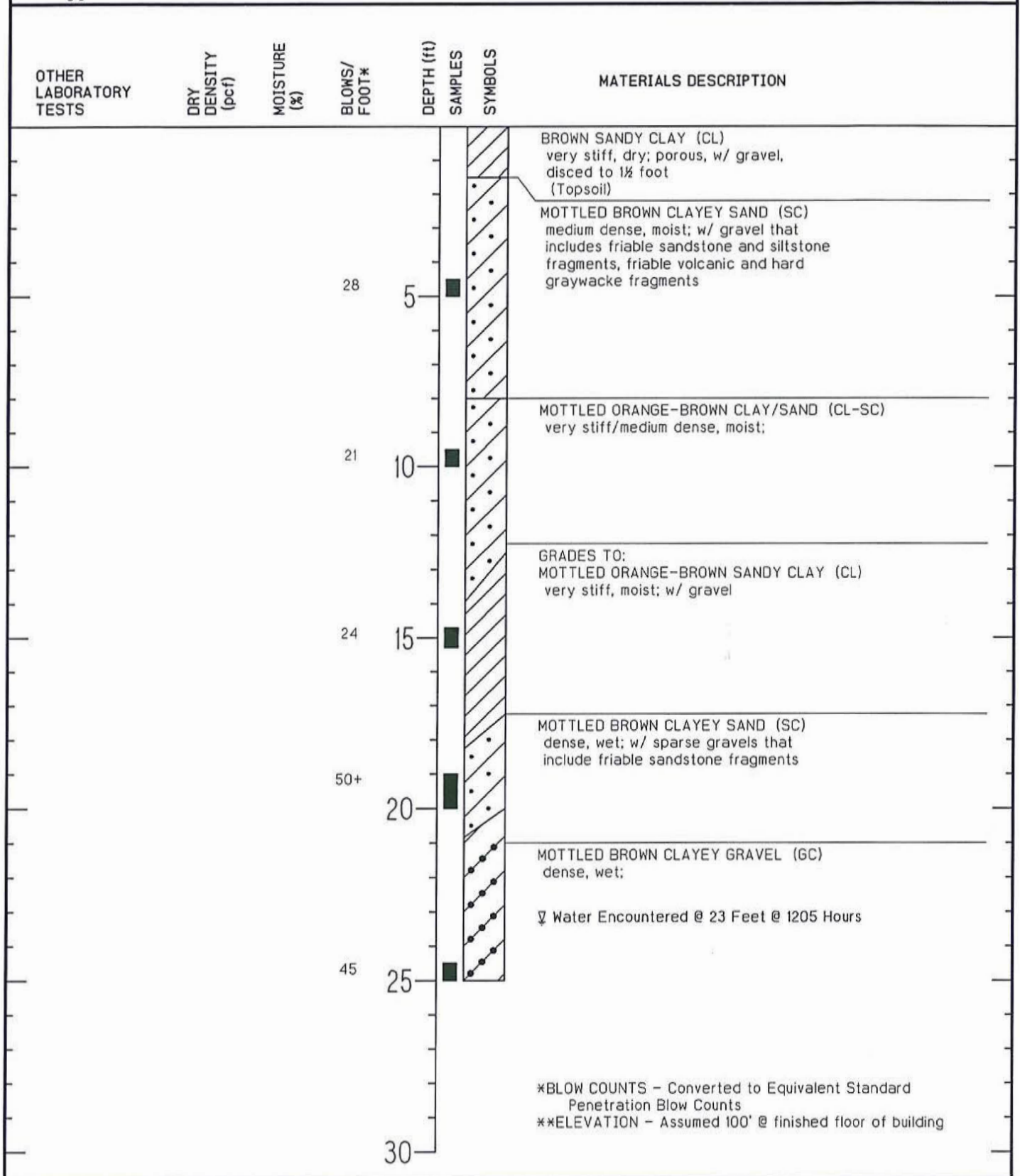
*BLOW COUNTS - Converted to Equivalent Standard
Penetration Blow Counts

**ELEVATION - Assumed 100' @ finished floor of building

PROJECT Geyserville Fire Protection
LOCATION District Headquarters, Geyserville
JOB NUMBER 2057.01.04.1
LOGGED BY GS
DRILL RIG Truck-Mounted: 6" Solid Stem Auger

DRILLING COMPANY Pearson Drilling
DATE DRILLED 06/26/03
SURFACE ELEVATION** 100 Feet
TOTAL DEPTH OF HOLE 26.5 Feet
APPROVED BY EJC

PLATE
6



PROJECT <u>Geyersville Fire Protection</u>	DRILLING COMPANY <u>Pearson Drilling</u>	PLATE 7
LOCATION <u>District Headquarters, Geyersville</u>	DATE DRILLED <u>06/26/03</u>	
JOB NUMBER <u>2057.01.04.1</u>	SURFACE ELEVATION** <u>98.5 Feet</u>	
LOGGED BY <u>GS</u>	TOTAL DEPTH OF HOLE <u>25 Feet</u>	
DRILL RIG <u>Truck-Mounted: 6" Solid Stem Auger</u>	APPROVED BY <u>[Signature]</u>	

OTHER LABORATORY TESTS	DRY DENSITY (pcf)	MOISTURE (%)	BLOWS/ FOOT*	DEPTH (ft)	SAMPLES	SYMBOLS	MATERIALS DESCRIPTION
			15				MOTTLED LIGHT BROWN SAND (SC) loose, dry; porous, w/ gravel, disced to 1½ feet (Topsoil)
Passing -200=36%			9				MOTTLED BROWN CLAYEY SAND (SC) medium dense, moist to dry; w/ gravel
			5				
Passing -200=33%			19				Becoming dense, increased gravels below 8½ feet
			10				MOTTLED BROWN CLAYEY GRAVEL (GC) very dense, moist;
			50+				NO FREE GROUNDWATER OBSERVED
			15				
			20				
			25				
			30				

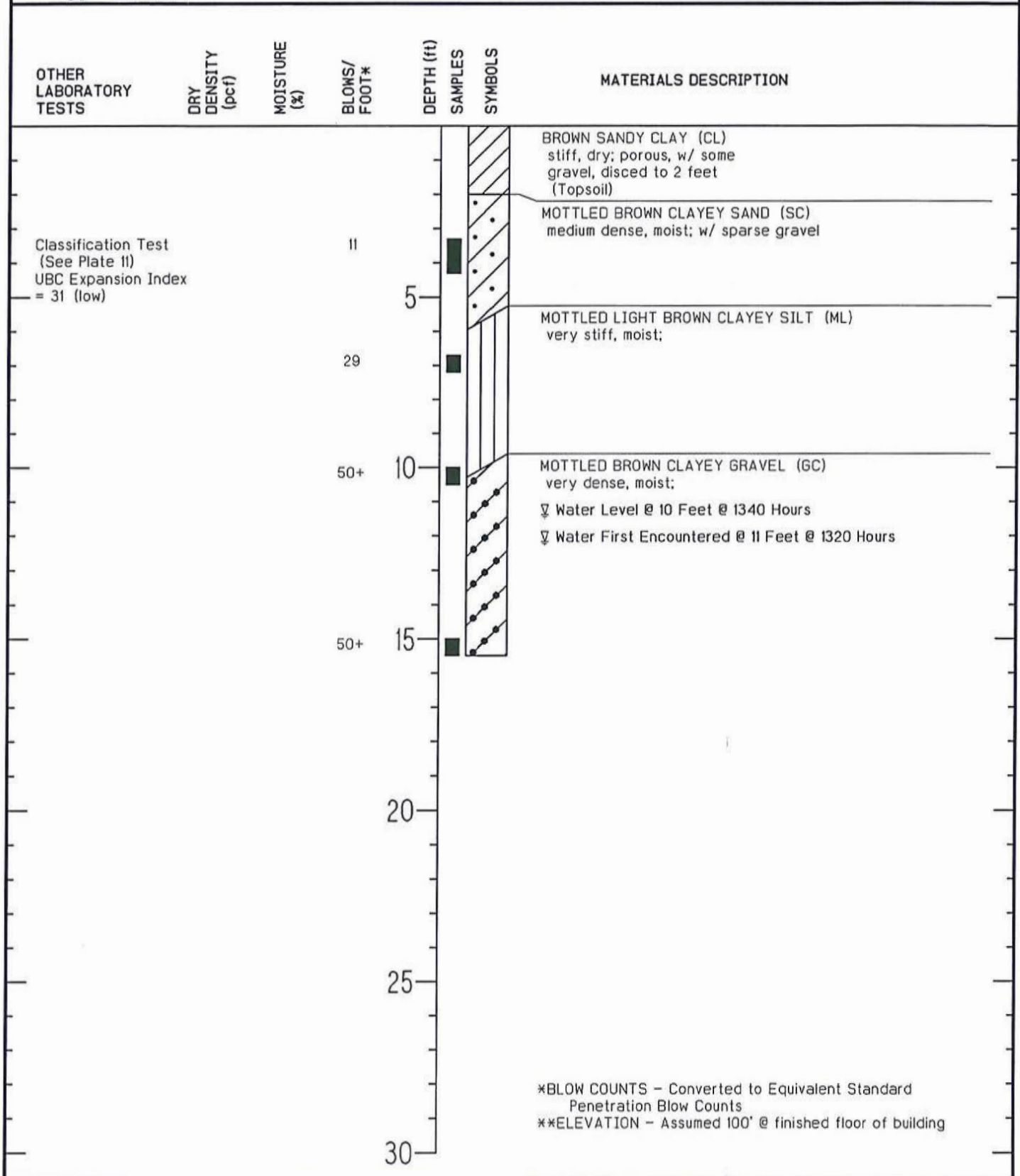
*BLOW COUNTS - Converted to Equivalent Standard
Penetration Blow Counts

**ELEVATION - Assumed 100' @ finished floor of building

PROJECT Geyserville Fire Protection
LOCATION District Headquarters, Geyserville
JOB NUMBER 2057.01.04.1
LOGGED BY GS
DRILL RIG Truck-Mounted: 6" Solid Stem Auger

DRILLING COMPANY Pearson Drilling
DATE DRILLED 06/26/03
SURFACE ELEVATION** 104 Feet
TOTAL DEPTH OF HOLE 12.8 Feet
APPROVED BY EJC

PLATE
8



PROJECT <u>Geyserville Fire Protection</u>	DRILLING COMPANY <u>Pearson Drilling</u>	PLATE 9
LOCATION <u>District Headquarters, Geyserville</u>	DATE DRILLED <u>06/26/03</u>	
JOB NUMBER <u>2057.01.04.1</u>	SURFACE ELEVATION** <u>110.5 Feet</u>	
LOGGED BY <u>GS</u>	TOTAL DEPTH OF HOLE <u>15.5 Feet</u>	
DRILL RIG <u>Truck-Mounted: 6" Solid Stem Auger</u>	APPROVED BY <u>[Signature]</u>	

MAJOR DIVISIONS				TYPICAL NAMES
COARSE GRAINED SOILS More Than Half Is Larger Than #200 Sieve	GRAVELS More Than Half Coarse Fraction Is larger Than No. 4 Sieve Size	Clean Gravels With Little or No Fines	GW	Well Graded Gravels, Gravel - Sand Mixtures
			GP	Poorly Graded Gravels, Gravel - Sand Mixtures
		Gravels With Over 12% Fines	GM	Silty Gravels, Poorly Graded Gravel - Silt - Silt Mixtures
			GC	Clayey Gravels, Poorly Graded Gravel - Sand - Clay Mixtures
	SANDS More Than Half Coarse Fraction Is Smaller Than No. 4 Sieve Size	Clean Sands With Little or No Fines	SW	Well Graded Sands, Gravelly Sands
			SP	Poorly Graded Sands, Gravelly Sands
		Sands With Over 12% Fines	SM	Silty Sands, Poorly Graded Sand - Silt Mixtures
			SC	Clayey Sands, Poorly Graded Sand - Clay Mixtures
FINE GRAINED SOILS More Than Half Is Smaller Than #200 Sieve	SILTS AND CLAYS Liquid Limit Less Than 50		ML	Inorganic Silts and Very Fine Sands, Rock Flour, Silty or Clayey Fine Sands, or Clayey Silts with Slight Plasticity
			CL	Inorganic Clays of Low to Medium Plasticity, Gravelly Clays, Sandy Clays, Silty Clays, Lean Clays
			OL	Organic Clays and Organic Silty Clays of Low Plasticity
	SILTS AND CLAYS Liquid Limit Greater Than 50		MH	Organic Silts, Micaceous or Diatomaceous Fine Sandy or Silty Soils, Elastic Silts
			CH	Inorganic Clays of High Plasticity, Fat Clays
			OH	Organic Clays of Medium to High Plasticity, Organic Silts
	HIGHLY ORGANIC SOILS		Pt	Peat and Other Highly Organic Soils

UNIFIED SOIL CLASSIFICATION SYSTEM

		Shear Strength, psf		Confining Pressure, psf	
Consol	- Consolidation	Tx	320 (2600)	Unconsolidated Undrained Triaxial	
LL	- Liquid Limit (In %)	Tx CU	320 (2600)	Consolidated Undrained Triaxial	
PL	- Plastic Limit (In %)	DS	2750 (2000)	Consolidated Drained Direct Shear	
PI	- Plastic Index	FVS	470	Field Vane Shear	
G _s	- Specific Gravity	UC	2000	Unconfined Compression	
SA	- Sieve Analysis	LVS	700	Laboratory Vane Shear	
☒	"Undisturbed" Sample	SS	- Shrink Swell		
☒	Bulk or Disturbed Sample	EXP	- Expansion		
☒	Standard Penetration Test	P	- Permeability		
☐	Sample Attempt With No Recovery				

Note: All strength tests on 2.8" or 2.4" diameter sample unless otherwise indicated.

KEY TO TEST DATA

R G H

Geotechnical and
Environmental
Consultants

Job No: 2057.01.04.1

Appr: *EJC*

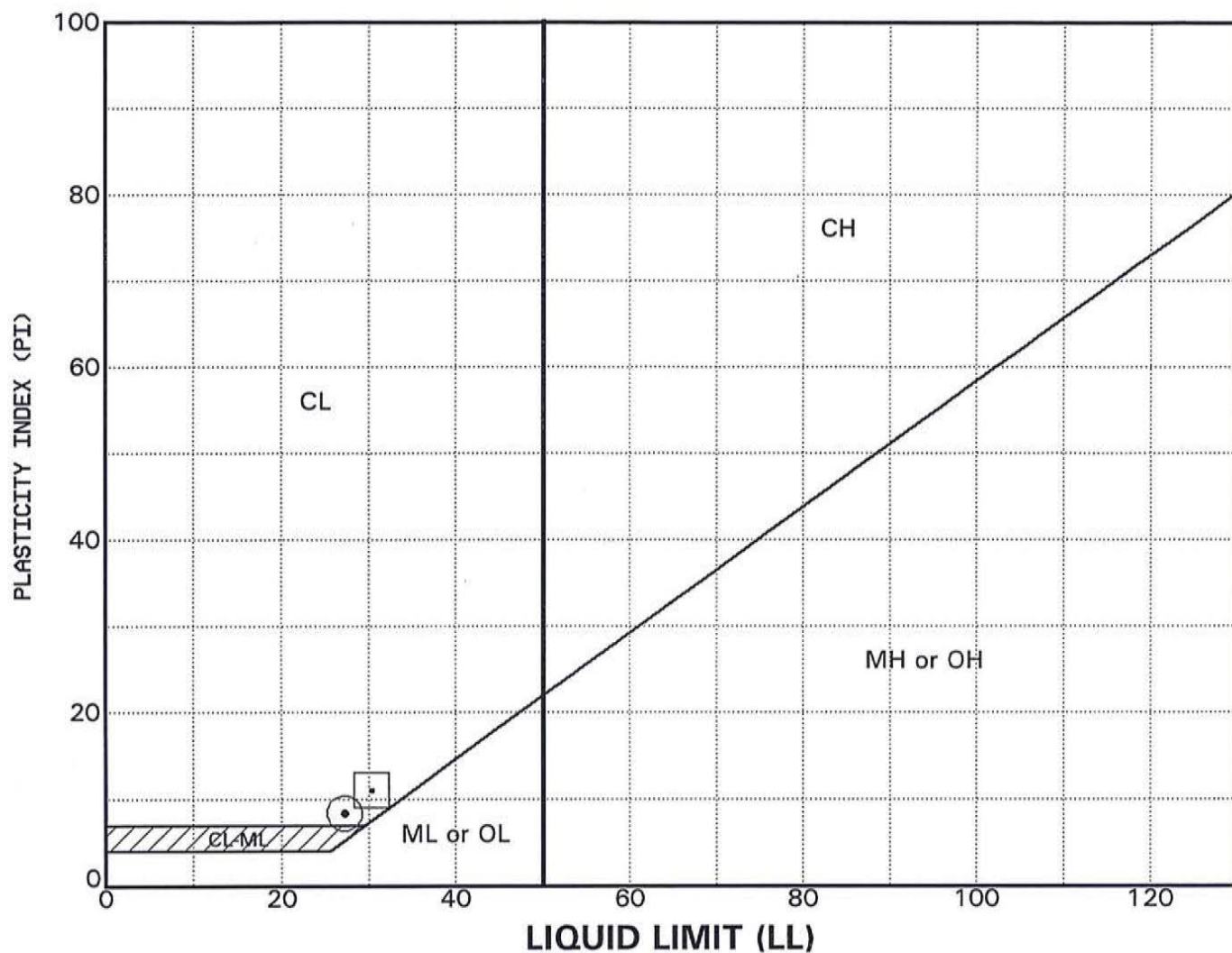
Drwn: kabs

Date: July 2003

SOIL CLASSIFICATION CHART AND KEY TO TEST DATA

GEYSERVILLE FIRE PROTECTION DISTRICT
HEADQUARTERS - 20975 GEYSERVILLE AVE.
Geyserville, California

PLATE
10



SAMPLE SOURCE	CLASSIFICATION	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	% PASSING #200 SIEVE	UBC EXPANSION
⊙ B-4 @ 3.3'	Brown Clayey Sand (SC)	27	8	42	
□ S-1 @ 2.5'	Brown Clayey Sand W/Gravel (SC)	30	11	35	33

R G H

Geotechnical and
Environmental
Consultants

Job No: 2057.01.04.1

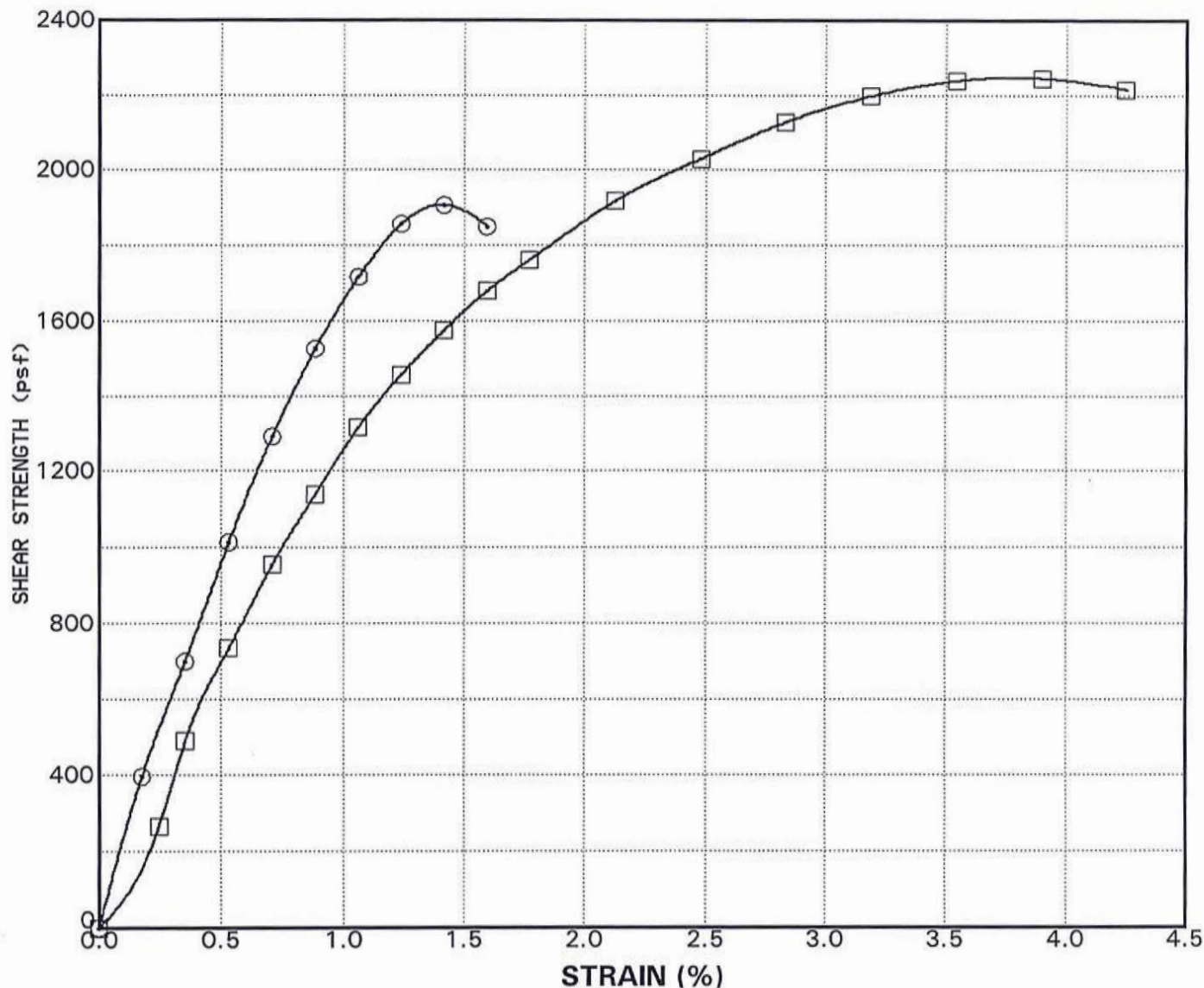
Appr: *[Signature]*

Drwn: kabs

Date: July 2003

CLASSIFICATION TEST DATA
GEYSERVILLE FIRE PROTECTION DISTRICT
HEADQUARTERS - 20975 GEYSERVILLE AVE.
 Geyserville, California

PLATE
11



Sample Source	Classification	Type of Test	Confinement Pressure (psf)	Ultimate Strength (psf)	Strain (%)	Dry Density (pcf)	Moisture Content (%)
⊙ B-1 @ 1.5'	Brown Sandy Lean Clay (CL)	TX/UU		1905	1	111	12.5
□ B-1 @ 3.5'	Brown Sandy Lean Clay (CL)	TX/UU		2243	4	114	15.3

UC = Unconfined Compression

TX/UU = Unconsolidated Undrained Triaxial

R G H

Geotechnical and
Environmental
Consultants

Job No: 2057.01.04.1

Appr: *[Signature]*

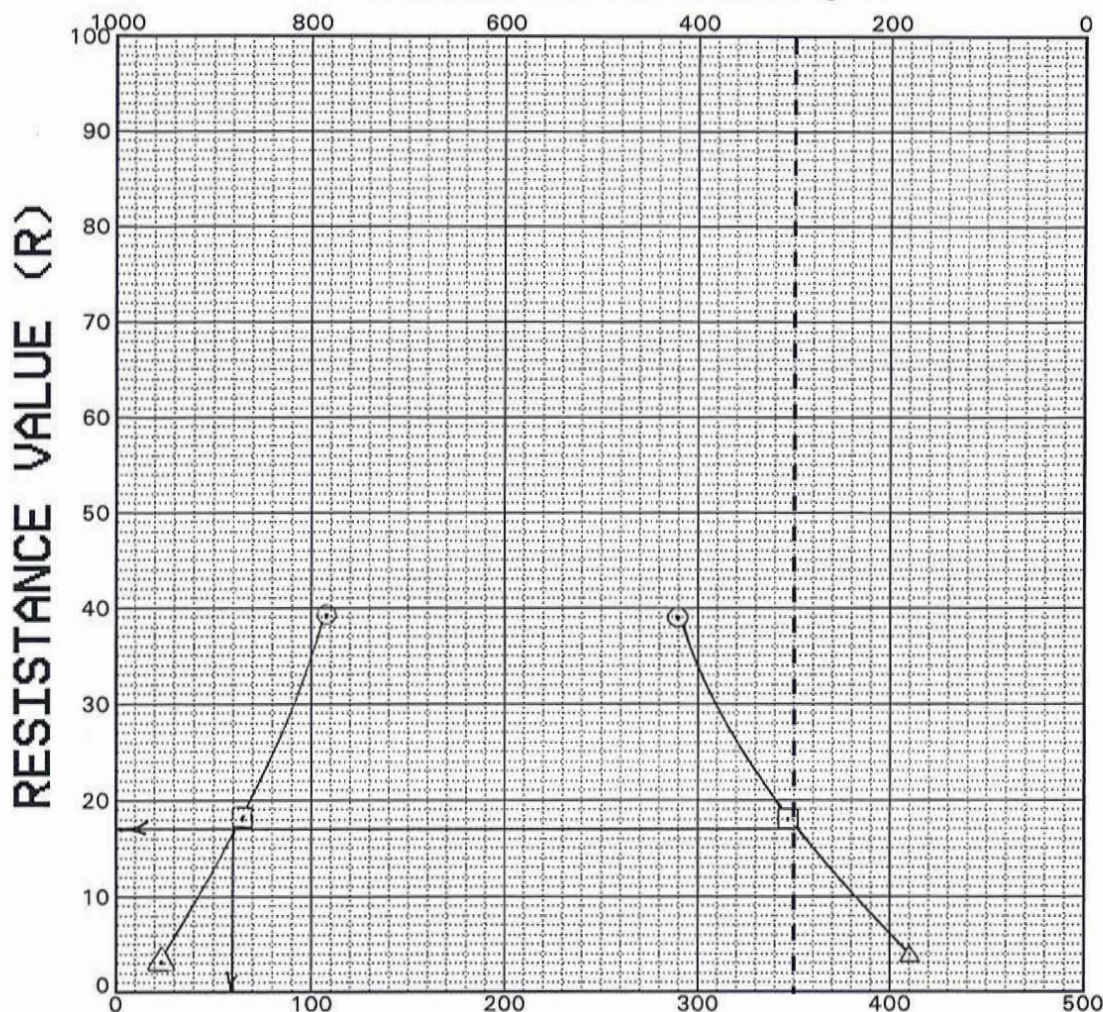
Drwn: kabs

Date: July 2003

STRENGTH TEST DATA
GEYSERVILLE FIRE PROTECTION DISTRICT
HEADQUARTERS - 20975 GEYSERVILLE AVE.
 Geyserville, California

PLATE
12

EXUDATION PRESSURE (psi)



EXPANSION PRESSURE (psf)

SPECIMEN NO.	⊙	□	△
MOISTURE CONTENT (%)	12.5	13.1	14.2
DRY DENSITY (PCF)	124	122	120
EXUDATION PRESSURE (PSI)	421	306	180
EXPANSION PRESSURE (PSF)	108	65	22
RESISTANCE VALUE (R)	39	18	4

SAMPLE SOURCE	CLASSIFICATION	SAND EQUIVALENT	EXPANSION PRESSURE	R-VALUE
S-1 @ 2.50	Brown Clayey Sand W/Gravel (SC)		60	17

ASTM D 2844, Cal Test 301

R G H
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Job No: 2057.01.04.1

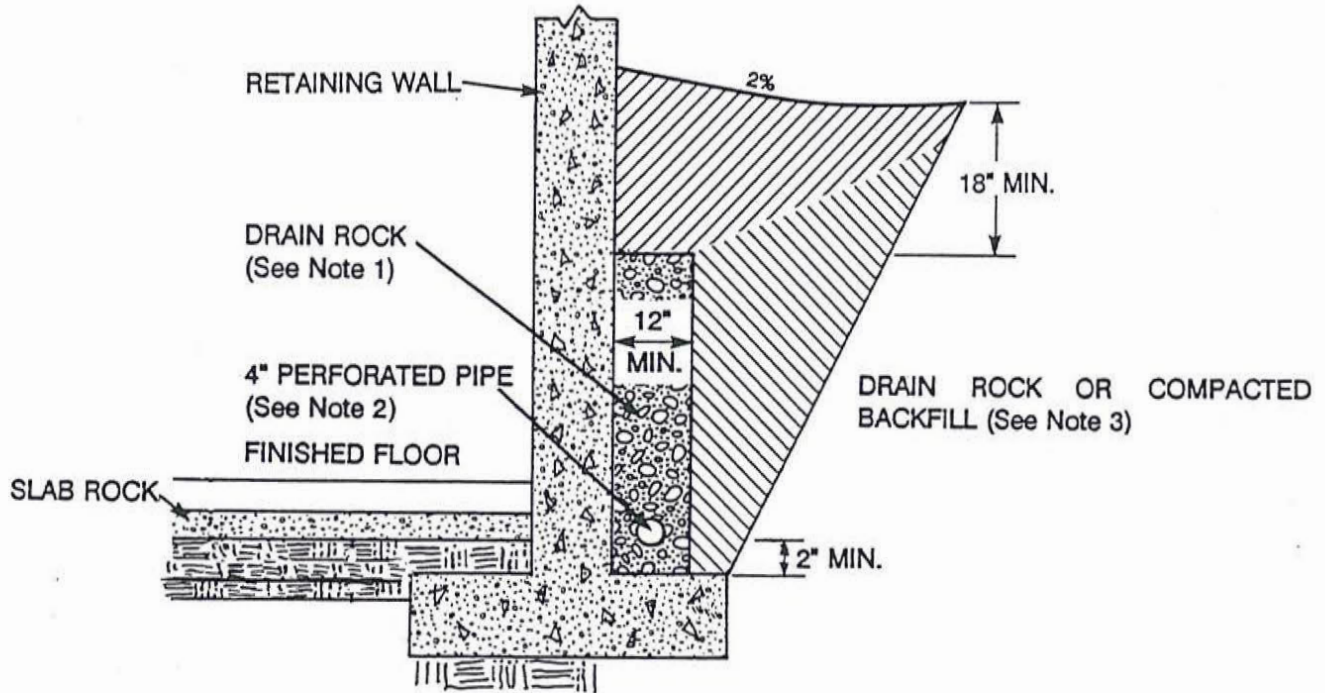
Appr: *EJC*

Drwn: kabs

Date: July 2003

RESISTANCE (R) VALUE DATA
GEYSERVILLE FIRE PROTECTION DISTRICT
HEADQUARTERS - 20975 GEYSERVILLE AVE.
Geyserville, California

PLATE
13



Notes:

1. Drain rock should meet the requirements for Class 2, Permeable Material, Section 68, State of California "Caltrans" Standard Specification, latest edition. Drain rock should be placed to approximately three-quarters the height of the retaining wall.
2. Pipe should conform to the requirements of Section 68 of State of California "Caltrans" Standards, perforations placed down, sloped at 1% for gravity flow to outlet or sump with automatic pump. The pipe invert should be located at least 8 inches below the finished grade.
3. During compaction the contractor should use appropriate methods such as temporary bracing and/or light compaction equipment to avoid overstressing the walls.

Not to Scale

R G H

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Environmental
Consultants

Job No: 2057.01.04.1

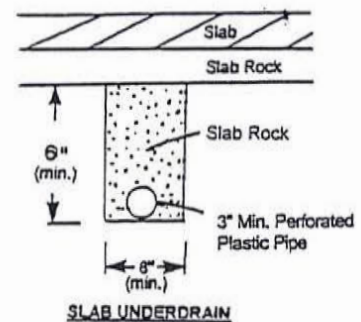
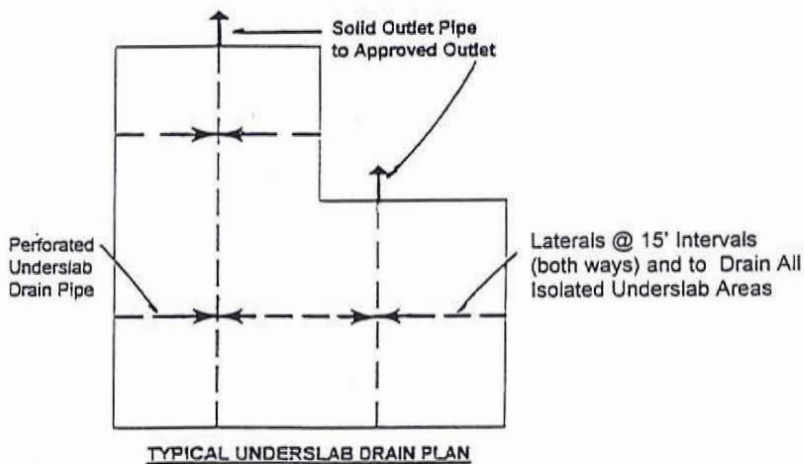
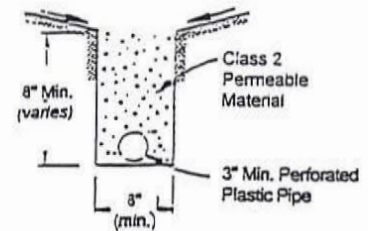
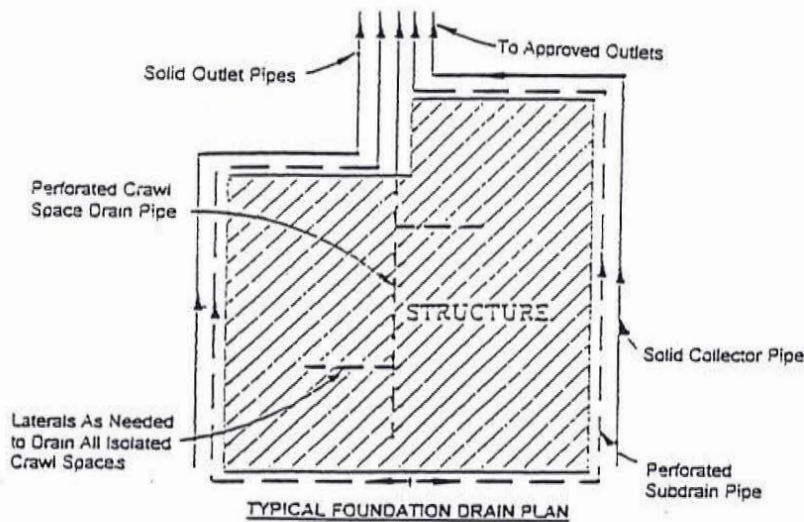
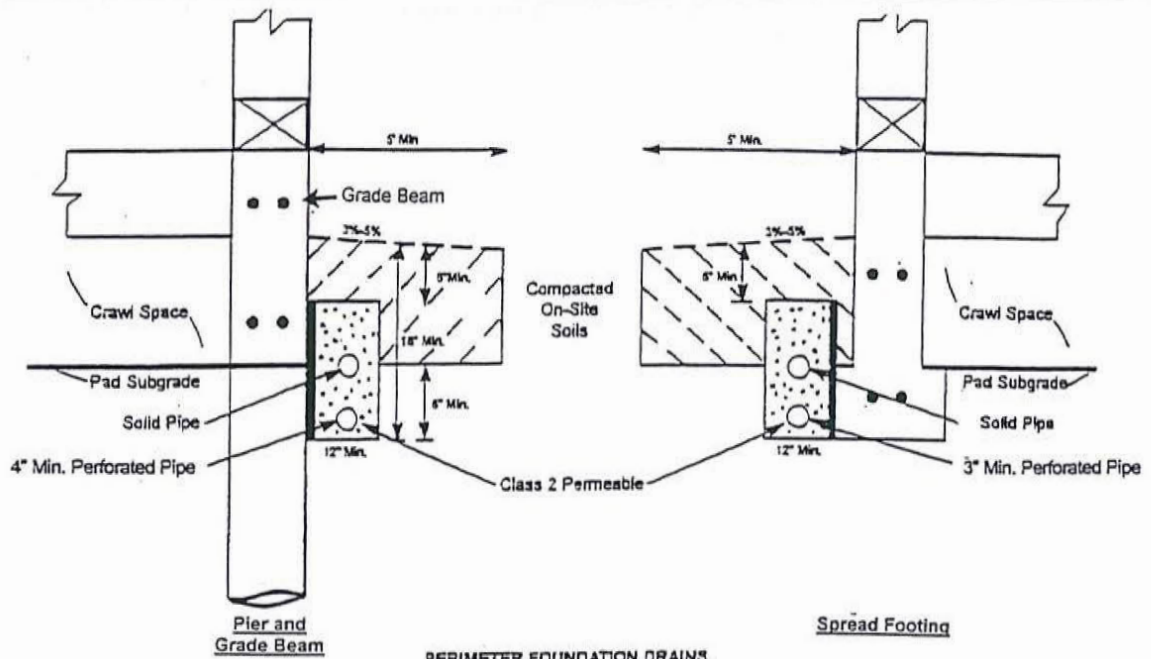
Appr: *[Signature]*

Drwn: kabs

Date: July 2003

**RETAINING WALL BACKDRAIN ILLUSTRATION
GEYSERVILLE FIRE PROTECTION DISTRICT
HEADQUARTERS - 20975 GEYSERVILLE AVE.
Geyserville, California**

PLATE
14



Not to Scale

R G H

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Environmental
Consultants

Job No: 2057.01.04.1

Appr: *EJC*

Drwn: kabs

Date: July 2003

TYPICAL SUBDRAIN DETAILS ILLUSTRATION
GEYSERVILLE FIRE PROTECTION DISTRICT
HEADQUARTERS - 20975 GEYSERVILLE AVE.
Geyserville, California

PLATE
15

APPENDIX B - REFERENCES

- Blake, M. C., Jr., et al., 1971, Preliminary Geologic Map of Western Sonoma County and Northwestern Marin County, California: U.S. Geological Survey Open File Map, Basic Data Contribution 12, Scale 1:62,500.
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July 15, 2003
Geyserville Fire Protection District Headquarters
Project Number: 2057.01.04.1

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Youd, T.L., and Idriss, I.M., eds., 1997, Proceedings of National Center for Earthquake Engineering Research (NCEER) Workshop on Liquefaction Resistance of Soils, Salt Lake City, Utah, January 4-5: NCEER, Buffalo, New York, Technical Report NCEER-97-0022, 276 p.

July 15, 2003
Geyserville Fire Protection District Headquarters
Project Number: 2057.01.04.1

APPENDIX C - CORROSION TEST RESULTS

E T S

1343 Redwood Way
Petaluma, CA 94954

(707) 795-9605/FAX 795-9384

*Environmental
Technical
Services*

Soil, Water, Air, Plant
Tissue and Other
Testing & Monitoring
Analytical Labs
Technical Support

Serving people and the environment so that both benefit.

COMPANY: RGH Geotech, 1305 N. Dutton Avenue, Santa Rosa, CA 95401	DATE RECEIVED 7/2/03	DATE of COMPLETION 7/14/03	ANALYST(S) M. Banwait G. Hundt	SUPERVISOR D. Jacobson
ATTN: Eric Chase				LAB DIRECTOR G.S. Conrad PhD
JOB SITE: Geyserville Fire Dept. Geyserville, California.				
JOB #: unk				

LAB SAMPLE NUMBER	SAMPLE ID	DESCRIPTION of SOIL and/or SEDIMENT	SOIL pH -log[H+]	SINGLE POINT RESISTIVITY ohm-cm	ELECTRICAL CONDUCTIVITY μmhos/cm	SULFATE SO4 ppm	CHLORIDE Cl ppm
00044-1	GFS1	B-2 @ 4.5'	5.61	4650	[215]	42	30
00044-2	GFS2	B-3 @ 1.5'	5.18	6900	[145]	78	28

[illegible]

COMMENTS

Resistivities are over 4,000 & 6000 ohm/cm which is very good, but soil reactions (i.e., pHs) are moderately acidic which does not help; sulfates and chlorides are low. The CalTrans times to perforation for these soils are as follows: for GFS1 and 18 ga steel the time is a mediocre 16.5 yrs., and for 12 ga it is 36.4 yrs; and for BW2 the respective times are 17.3 yrs & 38.2 yrs. Sulfates are not high enough that they should have any adverse impact on grouts mortars, cements or concrete; and chlorides are safely below the limit where they can adversely impact contained steel reinforcement. These soils are good candidates for lime treating as concerns exposed steel. Lime treatment of these soils is calculated to improve the 18 ga times to perforation for GFS1 to the 45-50 year range, and to the 55-60 year range for GFS2 which is much better improvement than upgrading to 12 ga steel alone. Obviously, some combination of upgrading exposed steel and lime treating would result in even greater improvements. Last, as concerns these corrosivity results the standard concrete mix should be sufficient in these soils (but certainly lime treatment wouldn't hurt).

\\\\\\NOTES: Methods are from following sources: extractions by Cal Trans protocols as per Cal Test 417 (SO4), 422 (Cl), and 532/643 (pH & resistivity); &/or by ASTM Vol. 4.08 & ASTM Vol. 11.01 (=EPA Meth Chem Anal, or Standard Methods); pH - ASTM G 51; Spec. Cond. - ASTM D 1125; resistivity - ASTM G 57; redox - Pt probe/ISE; sulfate - extraction Title 22, detection ASTM D 516 (=EPA 375.4); chloride - extraction Title 22, detection ASTM D 512 (=EPA 325.3); sulfides - extrac. Title 22, detection EPA 376.2 (=SMEWW 4500-S D); cyanides - extraction Title 22, detection ASTM D 4374 (=EPA 335.2).

APPENDIX D - DISTRIBUTION

Geyserville Fire Protection District (2,0)
% Robert Stewart
4020 Pine Flat Road
Healdsburg, California 95448

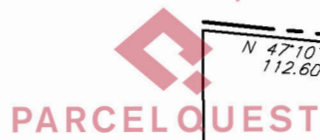
RRM Design Group (6,0)
Attn: Seth Hanley
1207 Vine Street, Suite G
Healdsburg, California 95448

Biggs Cardosa Associates (1,0)
Attn: Anthony Richardson
444 Spear Street, Suite 207
San Francisco, California 95403

Adobe Associates (1,0)
Attn: Tim Schram
415 Russell Avenue
Santa Rosa, California 95403

EGC:GWR:GDS:ec:jj(2057141.rpt)

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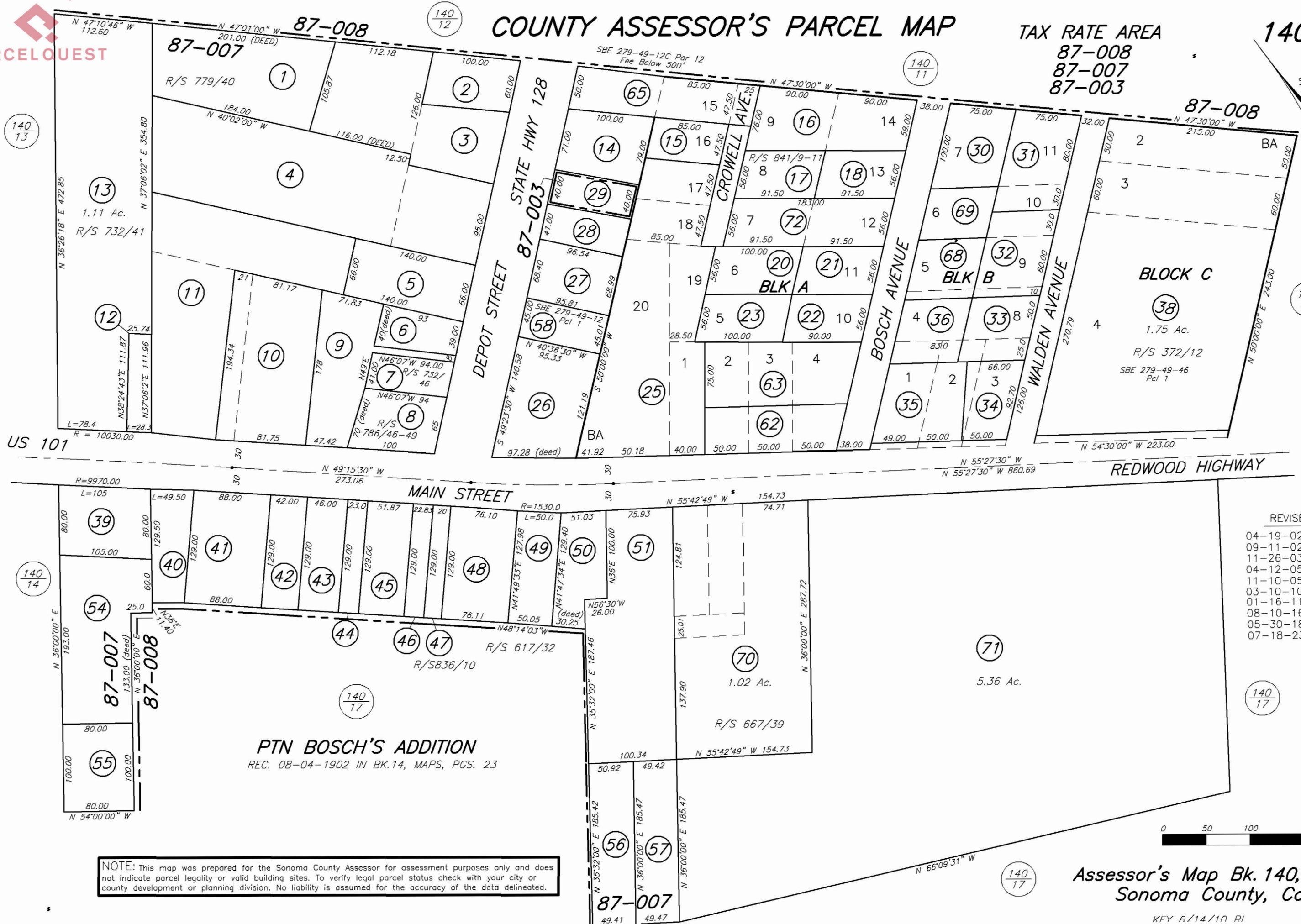
COUNTY ASSESSOR'S PARCEL MAP

TAX RATE AREA

87-008
87-007
87-003

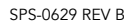
140-10

SCALE: 1"=100'

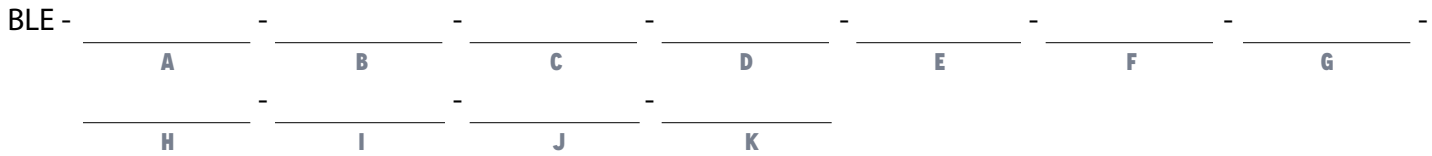


- REVISED
- 04-19-02=R/S-KB
 - 09-11-02=69-RM
 - 11-26-03=71-RM
 - 04-12-05=R/S-RM
 - 11-10-05=BLA-LW
 - 03-10-10=R/S-RL
 - 01-16-11=72-BJ
 - 08-10-16=R/S-KB
 - 05-30-18=R/S-DL
 - 07-18-23=R/S-DL

NOTE: This map was prepared for the Sonoma County Assessor for assessment purposes only and does not indicate parcel legality or valid building sites. To verify legal parcel status check with your city or county development or planning division. No liability is assumed for the accuracy of the data delineated.



JOB NAME: _____ FIXTURE TYPE: _____



A - MOUNTING STYLE

D – MOUNTING

NA Not Applicable

CSA LISTED CORDS:

SBK	Standard Black
SWH	Standard White
CSB	Black Cloth
YLW	Buttery Yellow Cloth
NAVY	Navy Cloth
RBLU	Royal Blue Cloth
SLVR	Silver Cloth
CRM	Creme Cloth
ORG	Orange Cloth
RBS	Barn Red & Black Swirl Cloth
NGRN	Neon Green Cloth
SBLU	Sky Blue Cloth
SFM	Sea Foam Cloth
RST	Rust Cloth
BPNK	Blush Pink Cloth
EGRN	Emerald Green Cloth
FGRN	Fern Green Cloth
CNF	Confetti Cloth

CHAIN MOUNT OPTIONS:

CN36 3' of Chain
CN72 6' of Chain

GOOSENECK OPTIONS:

G1²	G14²
G2	G15
G6	G22
G7	G24
G8	G36
G9	G64
G11	G65

STEM MOUNT OPTIONS:

ST506	.5" Stem Mount, 6"
ST512	.5" Stem Mount, 12"
ST518	.5" Stem Mount, 18"
ST524	.5" Stem Mount, 24"
ST536	.5" Stem Mount, 36"
ST548	.5" Stem Mount, 48"
ST706	.75" Stem Mount, 6"
ST712	.75" Stem Mount, 12"
ST718	.75" Stem Mount, 18"
ST724	.75" Stem Mount, 24"
ST736	.75" Stem Mount, 36"
ST748	.75" Stem Mount, 48"

E - CORD LENGTH³

NA	Not Applicable
7FT	7' of Cord
12FT	12' of Cord
20FT	20' of Cord

F – MOUNTING FINISH

Please Note: See **Section C** for Finish Options. 980-Brushed Aluminum is also available for Gooseneck (except G36, G64 & G65) and Stem mounting styles.

(I) If Cord Hung [C], selection identifies canopy finish

(II) If Chain Hung [CN] Mounting Style, selection identifies chain and canopy finish.

(III) If Stem Mount [S] Mounting Style, selection identifies stem and canopy finish

(IV) If Flush Mount [F] Mounting Style, selection identifies Hex Coupler and Canopy Finish

980 Brushed Aluminum

G – CORDS⁴

Please Note: See **Section D** for all applicable CSA Listed Cord Options for Chain Hung Mounting Style

NA Not Applicable

H - MOUNTING ACCESSORY

Please Note: Mounting Accessories below are only applicable with select Mounting Styles. Please refer to product listings on our website for further detail.

NA	None/Not Applicable
HSC	Hang Straight Canopy ⁵
LDBPC	LED Decorative Backing Plate Cover ^{6,7}
LDCHX	LED Decorative Backing Plate Cover & Hex Cover ^{6,7}

I - LIGHT SOURCE

LED11	850 Lumen
LED16	1250 Lumen
LED27	2000 Lumen
LED38	3000 Lumen
LED43	4000 Lumen

J - COLOR TEMPERATURE

2700K	2700K, Warm White
3000K	3000K, Neutral White
3500K	3500K, Bright White
4000K	4000K, Cool White

K - LED LENS

DL	Domed Lens
FL	Flat Lens

B - SHADE SIZE

THE ORIGINAL™:
WHS24 24" Shade
WHS28 28" Shade

C - SHADE FINISH

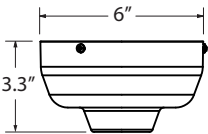
POWDER COAT FINISHES¹:

100	Black
105	Textured Black
106	Matte Black
200	White
206	Matte White
300	Dark Green
307	Emerald Green
311	Seafoam
320	Fern Green
370	Mint
380	Chartreuse
390	Teal
400	Barn Red
420	Orange
430	Soft Clay
470	Watermelon
480	Blush Pink
490	Magenta
495	Sherbet Orange
500	Buttery Yellow
570	Sunflower
600	Bronze
601	Chocolate
605	Textured Rust
607	Taupe
615	Textured Bronze
630	Beige
700	Royal Blue
705	Navy
715	Sky Blue
790	Lavender
800	Industrial Grey
805	Charcoal Granite
815	Light Grey
975	Galvanized

IMPORTANT: (1) All Powder Coat finished shades, Galvanized excluded, feature a white interior (2) Not available with 28" size shade (3) Only applicable if Cord Hung Mounting Style is selected in Section A, select NA if another Mounting Style is selected in Section A (4) Only applicable if Chain Hung Mounting Style selected in Section A, select NA if another Mounting Style is selected (5) Only applicable if Stem Mounting Style is selected in Section A (6) Not available with G11 and G15 Gooseneck option (7) Only applicable if Gooseneck Mounting Style is selected in Section A

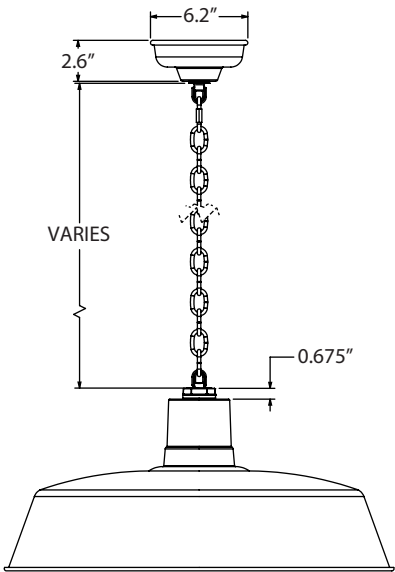
MOUNTING STYLE

HIGH LUMEN CANOPY FOR PENDANT MOUNT



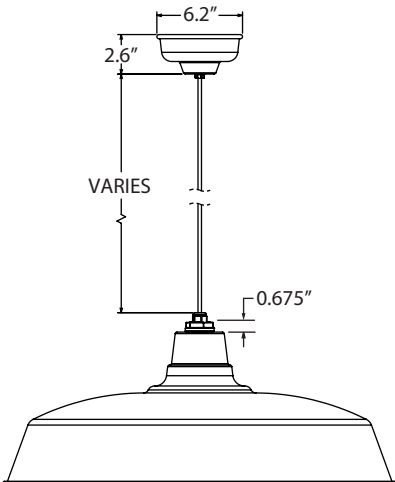
Required if LED 27, 38, 43 Light Source selected in Section J

CHAIN HUNG PENDANT (CN)



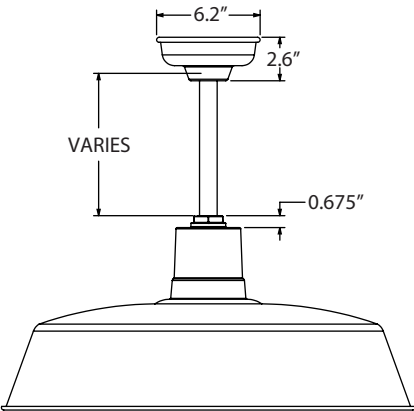
Canopy shown required if LED 11 & 16 Light Source selected in Section J

CORD HUNG PENDANT (C)



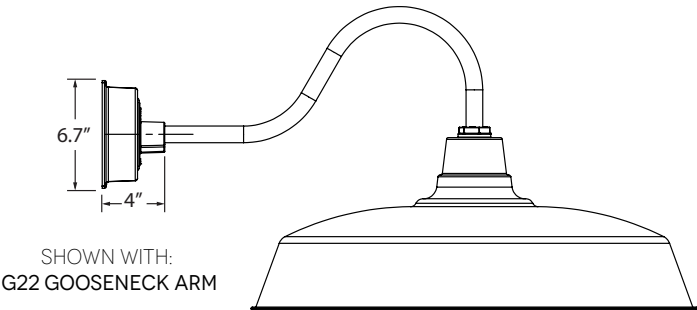
Canopy shown required if LED 11 & 16 Light Source selected in Section J

STEM MOUNT PENDANT (S)



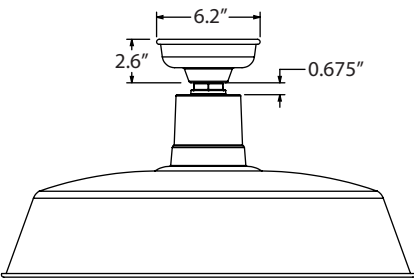
Canopy shown required if LED 11 & 16 Light Source selected in Section J

GOOSENECK (G)



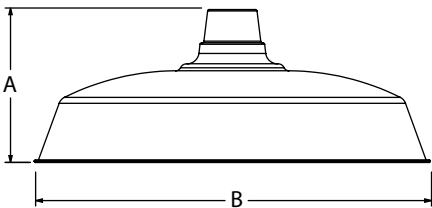
SHOWN WITH:
G22 GOOSENECK ARM

FLUSH (F)



Canopy shown required if LED 11 & 16 Light Source selected in Section J

LUMINAIRE DIMENSIONS



SHADE CODE	HEIGHT (A)	DIAMETER (B)
WHS24	10.75"	24"
WHS28	11"	28"

SPECIFICATIONS**CERTIFICATIONS, LISTINGS & WARRANTY**

MADE IN THE USA
 Manufactured and Hand-Crafted in Our 60,000 Square Foot Facility
 Located in Titusville, FL

CSA LISTED FOR WET LOCATIONS
 Includes All Gooseneck, Flush and Stem Mounting Styles

CSA LISTED FOR DAMP LOCATIONS
 Includes All Chain and Cord Hung Mounting Styles

LIMITED WARRANTY
 For Additional Information on Our Limited Warranty, Please See
 Our Terms & Conditions

CONSTRUCTION & FINISH

POWDER COAT SHADE
 Hand-Spun from High Purity 3003-O Temper Aluminum

POWDER COAT FINISHES
 Polyester Powder Coat Finishes Are Electro-Statically Applied and
 Thermocured

STEM
 1/2" Nominal (0.84" Actual) or 3/4" Nominal (1.05" Actual) Sch 40,
 6063 Aluminum Mounting Stem. Custom Lengths Available upon
 Request.

GOOSENECK
 1/2" Nominal (0.84" Actual) or 3/4" Nominal (1.05" Actual) Sch 40,
 6063 Aluminum Gooseneck

CORD
 Cord Hung Pendants Include 7', 12' or 20' of Cord (Overall Cord
 Length is Measured from Base of Socket and Varies Based on
 Fixture)

CHAIN
 4-Gauge Chain Complete with Quick Link for On-Site Adjustments
 to Chain's Length

LED LIGHT SOURCE

AVAILABLE CCT: 2700K, 3000K, 3500K, 4000K
 90+ CRI

ELECTRICAL/LED DRIVER

POWER FACTOR > 0.9

EFFICIENCY: UP TO 87% TYPICAL

INPUT VOLTAGE: 120-277 VAC (SEE DIMMING SECTION)

CLASS 2 POWER SUPPLY

LED DRIVER LIFETIME: > 100,000 HOURS (@ 25 DEG C AMBIENT)

IP64 MINIMUM RATING CASE WITH SILICONE-BASED POTTING

PROTECTIONS

Output Open Load, Over-Current and Short-Circuit (hiccup), and
 Over-Temperature with Auto Recovery

CONDUCTED AND RADIATED EMI

Compliant with FCC CFR Title 47 Part 15 Class B (120 Vac), Class A
 (277 Vac) and EN55015 (CISPR 15) at 220, 230 and 240 Vac

COMPLIANT WITH

Complies with Energy Star® DLR (DesignLight Consortium®) and CA
 Title 24 Technical Requirements

DIMMING**TRI-MODE DIMMING™**

Compatible with TRIAC (forward-phase or leading-edge), ELV
 (reverse-phase or trailing-edge) and 0-10 V Dimmers

DIMMING RANGE 1-100%

The Dimming Range Is Dependent on Each Specific Dimmer. May
 Not Be Able to Achieve 1% Dimming with Some Dimmers.

TRIAC AND ELV DIMMING ONLY AT 120 VAC

ELV DIMMING AT 230 VAC
 Available by Request

COMPATIBLE DIMMERS

Consult Factory for Additional Information on Dimming

Abbreviations

Scope

Sheet Index

Site Context

TopoLN1.1

Topographic Boundary Survey
Landowner Notification Map

Architectural

A0.0

Cover Sheet

A1.1

Site Plan

A1.2a

Site Development Plan

A1.2b

Site Development Plan

A1.3a

Construction Management

A1.3b

Construction Management

A1.4

Landscape Plan

A1.5

Site Section

A2.1

Floor Plan: First Floor Building M

A2.2

Floor Plan: Second Floor Building M

A2.3

Floor Plan: Building B

A2.4

Clerestory Plan: Building B

A3.1

High Roof Plan: Building M

A3.2

Flat Roof Plan: Building M

A3.3

Roof Plan: Building B

A4.1

Elevations: Building M

A4.2

Elevations: Building M

A4.3

Elevations: Building B

A4.4

Color & Material Board

Civil

C1

Title Sheet

C2

Site Plan

C3

Grading Plans

C4

utility Plans

C5

Driveway Profile and Sections

C6

Utility Details

C7

Grading Details

C8

Construction Notes

Project Data

Project Data

Project

20975 Geyserville Avenue, Geyserville, CA 95425

Primary APN

145-100-070

Original APNs

140-000-061

Orig Permit

BLD03-7104

2003 Approval

Zoning

K

Recreation and Visitor-Serving Commercial District

PF

PQP, RVSC

Public Facilities District (Base District)

Land Use

44,500 sf

Lot Size (ft.²)

1.02 ac

Existing Building Area

Ground Floor

9,630

Second Floor

2,920

Total

12,550

Occupancy Group

B

Construction Group

V-N

Automatic Sprinkler System

Yes

Height

2 Story

Proposed Project Data

Allowable / Required	Existing	Added	Removed	Total Proposed
Lot Coverage				
Lot Area	44,500 sf	- sf	- sf	44,500 sf
Lot Area	1.02 ac	- ac	- ac	1.02 ac
Building Footprint	17,800 sf	9,886 sf	3,862 sf	13,748 sf
Lot Coverage Total	40.00% %	22.22% %	8.68% %	30.89% %
Building Area				
Bldg M				
Ground Floor				
Building M: Ground	9,630 sf	718 sf	0 sf	10,348 sf
Tower	256 sf	0 sf	0 sf	256 sf
Subtotal Ground	0 sf	9,886 sf	718 sf	0
Second Floor				
Building M: 2nd	sf	2,920 sf	1,650 sf	0 sf
Tower	sf	256 sf	0 sf	0 sf
Subtotal Second	0 sf	3,176 sf	1,650 sf	0 sf
Total Bldg M	0 sf	13,062 sf	2,368 sf	0 sf
Bldg B				
Bldg B: Ground				
Total Bldg B	0 sf	0 sf	3,144 sf	0 sf
Total Building Area	0 sf	13,062 sf	5,512 sf	0 sf
Height				
Building M	35 ft	35 ft		35 ft
Building B	35 ft	- ft	23 ft	23 ft
Building Intensity				
BH	35 ft			35 ft
Lot Cov	17,800 sf	9,886 sf	3,862 sf	13,748 sf
	623,000 cf	346,010 cf		481,180 cf
Setbacks				
Zone K		Zone PF		
Allowable / Required	Allowable / Required	Existing	Proposed	Notes
Bldg M				
Front Property Line	15 ft	20 ft	31 ft	No change
Side Property Line: South	5 ft	5 ft	5 ft	No change
Side Property Line: North	5 ft	5 ft	45 ft	Bldg M addition
Rear Property Line	10 ft	20 ft	118 ft	No change
Bldg B				
Front Property Line	15 ft	20 ft	- ft	224 ft Bldg B addition
Side Property Line: South	5 ft	5 ft	- ft	40 ft Bldg B addition
Side Property Line: North	5 ft	5 ft	10 ft	Bldg B addition
Rear Property Line	10 ft	20 ft	- ft	10 ft Bldg B addition
Parking				
Allowable / Required	Existing	Added	Removed	Total Proposed
Standard (on street)	5 sp	1 sp	sp	5 sp
Van (on street)	1 sp	sp	sp	1 sp
Standard (off street)	19 sp	sp	sp	19 sp
Total	0 sp	25 sp	0 sp	25 sp

Legend

#

Grid Number

#

Door/Window Number

#

Plant Type

#

Keynote

#

Detail Sheet Number

#

Section Sheet Number

Property Line

Grid Line

Overhead Line

Center line

Vicinity Map

General Notes

Project Directory

Owner

Northern Sonoma County Fire Department

20975 Geyserville Ave, Geyserville, CA 95441

Voice: (707) 837-4373

Architect

Dreiling Terrones Architecture, Inc.

1103 Juanita Avenue

Burlingame, CA 94010

Voice: (650) 696-1200

Fax: (650) 343-9685

Structural

ZFA

250 D Street, Suite 200

Santa Rosa, CA 95404

Voice: (707) 526-0992

Mechanical

H&M Mechanical Group, Inc.

8105 Edgewater Drive, Suite 114

Oakland, CA 94621

Voice: (510) 569-2000

Plumbing

H&M Mechanical Group, Inc.

8105 Edgewater Drive, Suite 114

Oakland, CA 94621

Voice: (510) 569-2000

Electrical

The Engineering Enterprise

1305 Marina Village Parkway

Alameda, CA 95603

Voice: (510) 769-7600

Fax: (510) 769-1261

Civil

Atterbury & Associates, Inc.

16109 Healdsburg Ave

Healdsburg, CA 94621

Voice: (707) 433-0134

Fax: (707) 433-0135

Martin Dreiling

Principal Architect

Ben Klinger

Architectural Staff

Steven Granados

Architectural Staff

Luke Wilson

Principal

Kyle Bettencourt

Engineer

Garry Hennings

Principal

Stephen Wafer

Plumbing Designer

Scott Wheeler

Principal

Payton Henry

Engineer

Tom Atterbury

Principal

Northern Sonoma County Fire Protection District

Station 1 Modernization

20975 Geyserville Ave, Geyserville, CA 95441

Set Purpose

Planning Submittal

Minor Use Permit Mod

Design Review

Edition History

Schematic Design Set

1/9/2026

50% Progress Set

3/19/2026

Planning Submittal

4/20/2026

Sheet Subject

Program / Owner Proj #

DTA Project #

2525

Cover Sheet

A0.0

DTA

Dreiling Terrones Architecture Inc.

Architecture

Infrastructure

Environments

1103 Juanita Avenue

Burlingame, California

94010

650 696 1200

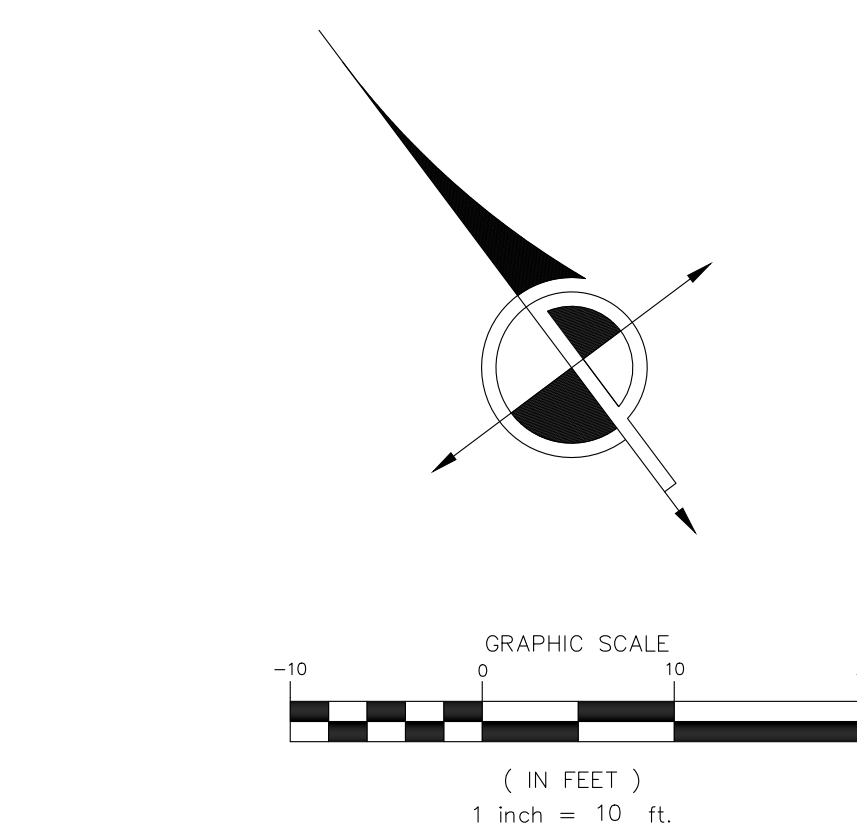
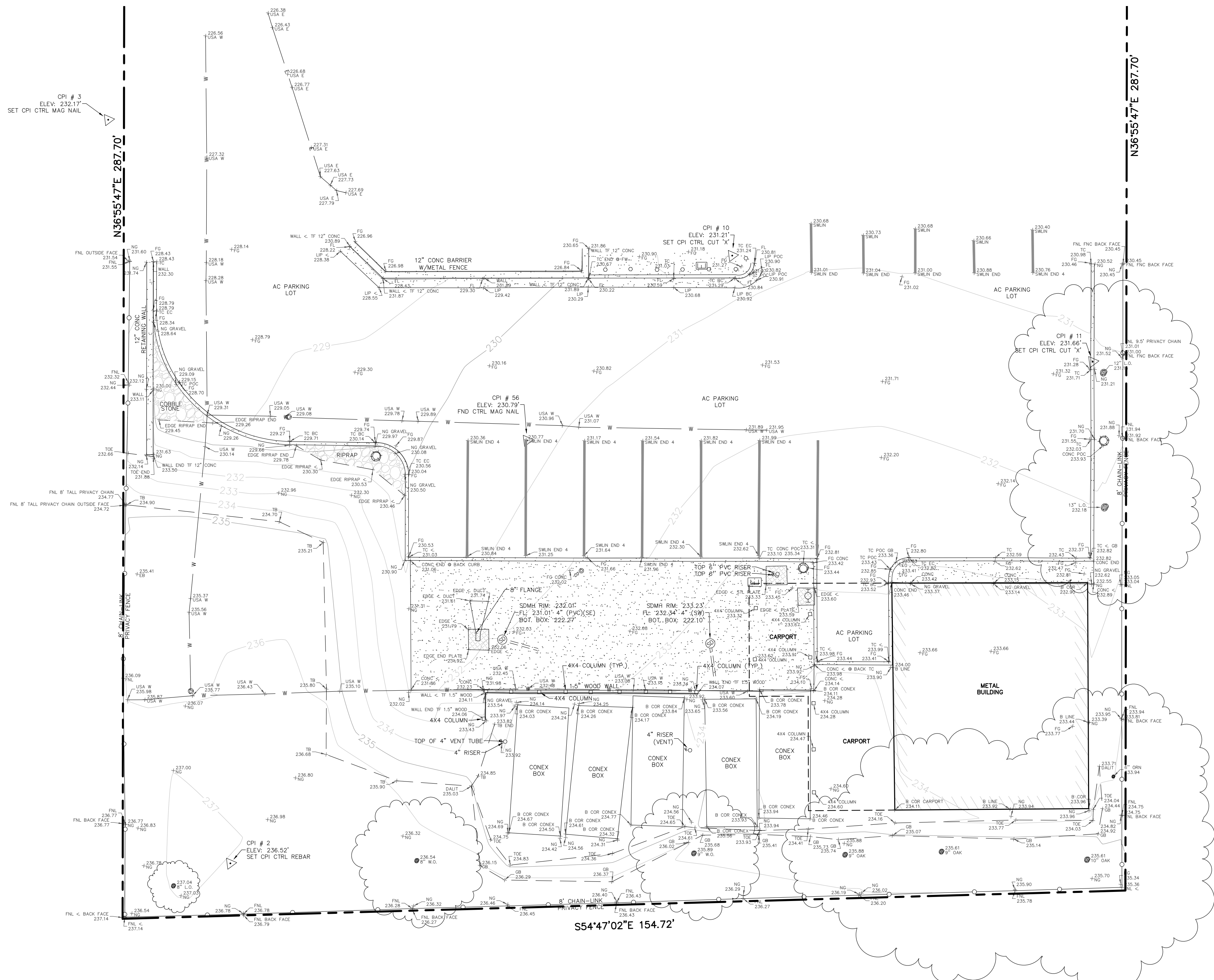
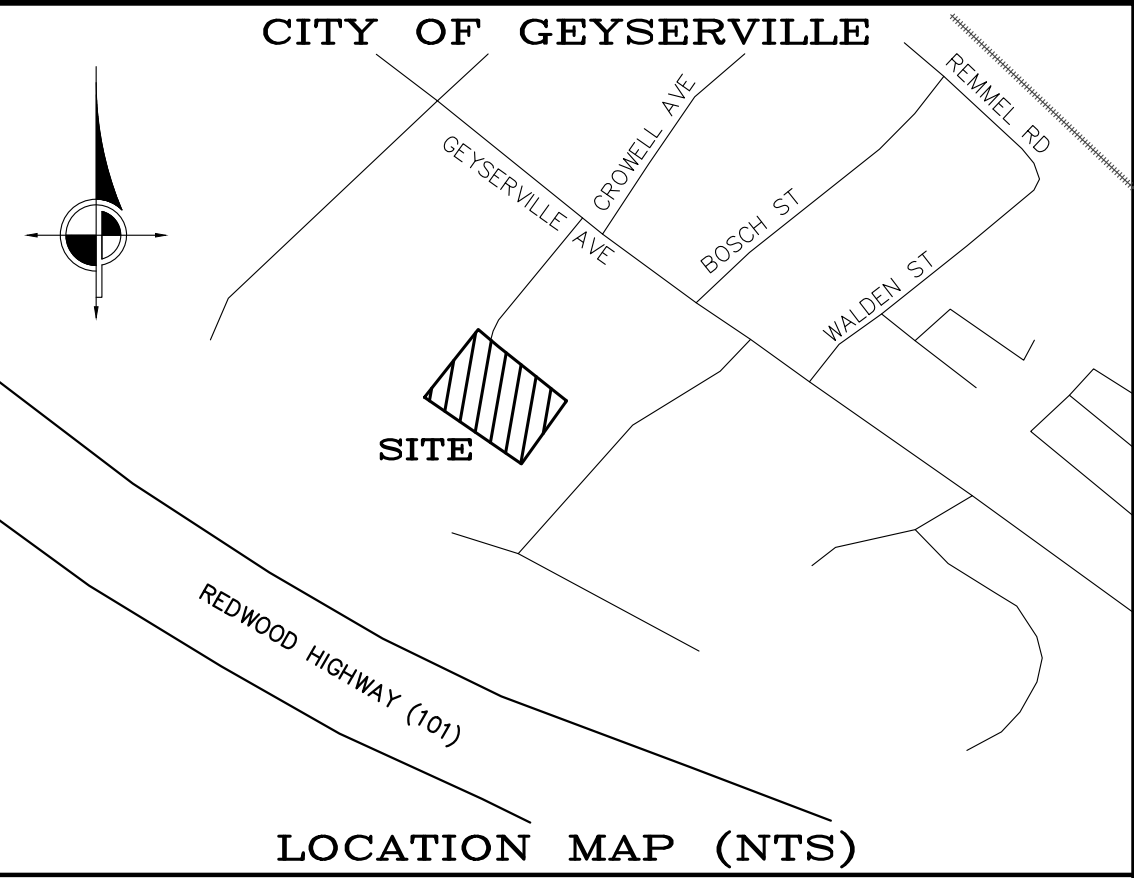
314 Center Street #220

Healdsburg, California

95448

707 431 1305

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HORIZONTAL DATUM

HORIZONTAL POSITIONS SHOWN HEREON ARE BASED ON THE CALIFORNIA COORDINATE SYSTEM OF 1983 (CCS83), ZONE 2, EPOCH 2025.00, U.S. SURVEY FOOT, AS DERIVED VIA STATIC GNSS TIES TO CALIFORNIA SPATIAL REFERENCE NETWORK STATIONS P188, P195, P203, AND P206, HOLDING THE PUBLISHED COORDINATES FROM THE DATA SET FOR SAID EPOCH.

VERTICAL DATUM

ELEVATIONS SHOWN HEREON ARE TIED TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88), AS DERIVED VIA STATIC GNSS TIES TO THE ABOVE-REFERENCED STATIONS, HOLDING THE PUBLISHED ELLIPSOID HEIGHTS FROM THE DATA SET FOR EPOCH 2025.00.

BENCHMARK

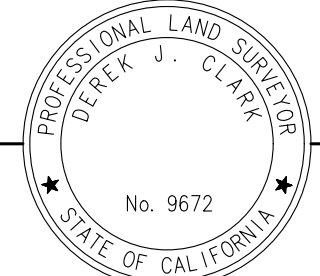
CONTROL POINT #10, A CUT 'X' IN CONCRETE, AS SHOWN HEREON.

EL=231.21' NAVD88

SURVEYOR'S STATEMENT

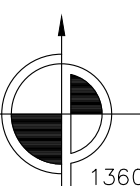
THIS MAP REPRESENTS A FIELD SURVEY MADE UNDER MY DIRECTION ON FEBRUARY 25, 2026, AND REPRESENTS THE VISUAL SURFACE CONDITIONS AS OF AFORESAID DATE.


DEREK J. CLARK, P.L.S. 9672



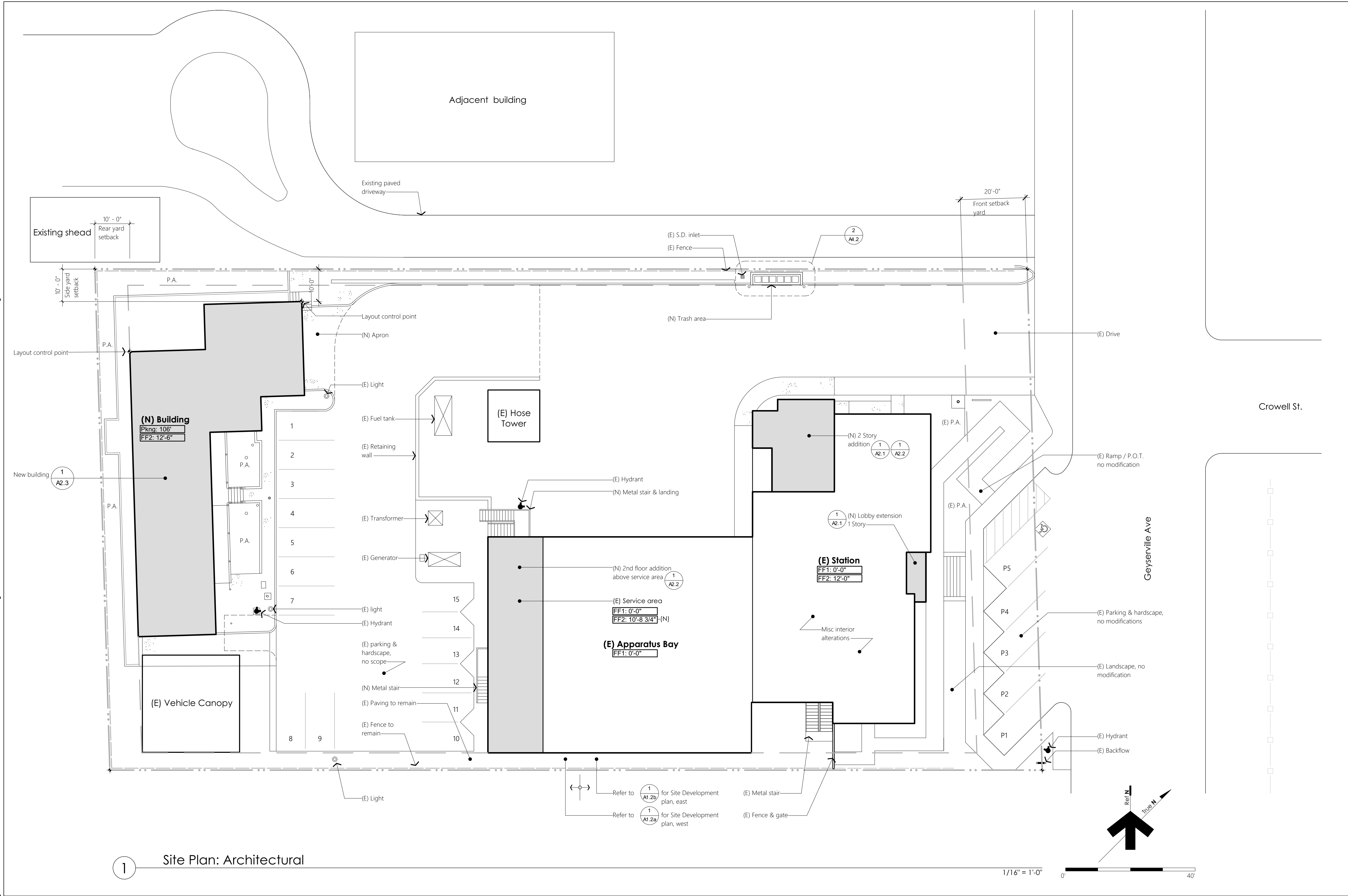
3-4-2026
DATE

LEGEND (ALL SYMBOLS MAY NOT APPLY)									
--- SUBJECT PROPERTY BOUNDARY	☐ CABLE TV BOXES	● RAINWATER LEADER	☐ PARKING METER	— OHE — OHE-HV	█ BUILDING PERIMETER	FG	FINISH GRADE	ETW	EDGE OF TRAVELED WAY
● FOUND IRON PIPE, AS NOTED.	☐ TELEPHONE BOXES	○ PG&E MANHOLE	● JUNCTION POLE	— OHU — OHU-HV	█ WOOD DECK/STAIRS	BSW	BACK OF SIDEWALK	DYBUT	DOUBLE YELLOW BUTTON
● FOUND MAG NAIL, AS NOTED.	⊕ TELEPHONE MANHOLE	☐ PG&E BOX	● JUNCTION POLE W/LIGHT	— OHT — OHT	█ DECO. ROCK/RIPRAP	EP	EDGE OF PAVEMENT	SYBUT	SINGLE YELLOW BUTTON
✓ FOUND PIN, AS NOTED.	☐ ELECTRIC BOXES/OUTLET	● TRAFFIC SIGNAL POLE	● POWER POLE	— OHG — OHG	█ CONCRETE	ER	EDGE NON-PAVED RD	DYLN	DOUBLE YELLOW LINE
● FOUND REBAR, AS NOTED.	⊕ EV CHARGING STATION	● TRAFFIC SIGNAL POLE W/LIGHT	● POWER POLE W/LIGHT	— E — E	█ TRUNCATED DOMES	TC	TOP FACE OF CURB	SWBUT	SINGLE WHITE BUTTON
▲ FOUND RAILROAD SPIKE, AS NOTED.	⊕ NATURAL GAS VALVE	☐ TRAFFIC SIGNAL BOX	● SERVICE POLE	— G — G	█ DIAMOND PLATE	BRC	BACK ROLLED CURB	SWLN	SINGLE WHITE LINE
⊕ FOUND SLASH, AS NOTED.	⊕ NATURAL GAS METER	☐ TRAFFIC DETECTOR POT	● TELEPHONE POLE	— SD — SD	█ WOOD WALL/LOG	LIP	LIP OF GUTTER	OMP	CORRUGATED METAL PIPE
✱ FOUND TAG, AS NOTED.	☐ SANITARY SEWER CLEANOUT	☐ TRAFFIC DETECTOR LOOP	● GUY POLE	— SS — SS-M	█ BRICK	BCOR	BUILDING CORNER	RCR	REINFORCED CONC. PIPE
● FOUND 'T' BAR, AS NOTED.	☐ STREET LIGHT/LANDSCAPE LIGHT	☐ STREET LIGHT BOX	● GUY ANCHOR	— W — W	█ ROADWAY STRIPING	BLNE	BUILDING LINE	HOPE	HIGH DENSITY POLYETHYLENE
● FOUND MONUMENT IN WELL, AS NOTED.	☐ STORM DRAIN CATCH BASIN	☐ UNKNOWN UTILITY BOX	— CHAINLINK FENCE	— FW — FW	█ NATURAL GROUND	RWT/RWB	RETAINING WALL TOP/BOTTOM	PL	PROPERTY LINE
▲ SURVEY CONTROL POINT	☐ STORM DRAIN MANHOLE	☐ UTILITY RISER	— WOOD FENCE	— UK — UK	█ FLOW LINE	UKN	UNKNOWN	DN	DOCUMENT NUMBER
± 100.00 SPOT ELEVATION	☐ JUNCTION BOX	☐ IRRIGATION VALVES	— WIRE FENCE	— UGTS — UGTS	█ GRADE BREAK	FNL	FENCE LINE	O.R.	OFFICIAL RECORD
			— IRON FENCE	— FO — FO	█ TOE/TOP OF BANK	FNC/FNI	FENCE CORNER/INTERSECTION	APN	ASSESSOR'S PARCEL NUMBER


CINQUINI & PASSARINO, INC.
PERFECTING GEOSPATIAL
1360 N. Dutton Ave. #150 Phone: (707) 542-6268
Santa Rosa, Ca. 95401 Fax: (707) 542-2106
WWW.CINQUINIPASSARINO.COM

● BOUNDARY
▲ TOPOGRAPHIC
▲ CONSTRUCTION
▲ SUBDIVISIONS





Site Notes

Topographic information used for design and represented in these Documents has been interpreted from survey information provided by:
Cinquini & Passano, Inc. 03/05/2026

The Architect bears no responsibility for accuracy of this information.

Design contours and drainage shown are schematic only and shall not be taken to represent final grading and drainage plans.

Refer Civil Drawings for grading, drainage, utilities, paving, street improvements.

Refer to Floor Plans and Sections for all vertical and horizontal dimensional information not provided.

Refer Foundation Plans for foundation dimensions and vertical controls.

Topsoil
Existing topsoil shall be removed from work area and stockpiled to be used for finish grading upon completion. Topsoil in excess of that required for finish grading shall be stored on site or disposed as directed by the Architect.

Waste excavation from foundation trenches that is not suitable for landscape use and is more than that required for foundation backfill shall be removed by Contractor.

Existing landscaping shall be protected as required to prevent any damage to plants and trees unless specified for removal in plans or by Architect.

Work Zone
Work Zone limits are shown on the Construction Management plans. The Contractor shall confine all operations, including delivery and stockpile of materials, to the Work Zone unless explicit agreement has been made for temporary placement with the Architect.

Roadways shall be maintained clear of construction equipment and materials at all times.

Contractor shall insure that emergency vehicles can exit the property in an emergency fully unimpeded by any aspect of construction operations.

The Contractor shall notify the Architect at least five days in advance when deliveries or other operations may impact areas outside the Work Zone.

The contractor shall maintain a clean work site. Construction debris and materials to be recycled shall be removed in a timely manner. Construction equipment and materials awaiting installation shall be stored in an orderly manner that reduces both damage to materials and reduces the visual impacts of the work site to the surrounding neighborhood.

Dust control measures shall be implemented as necessary. Measures may include watering, application of ground treatments, placement of rock and any other measure required to reduce or eliminate excessive dust.

Geotechnical Notes
The Contractor shall include and implement all pertinent requirements of this project as set forth in any Geotechnical Report prepared for the project. The Geotechnical Report shall become a part of the Contract Documents.

The Contractor shall review the Geotechnical Report and coordinate all required inspections, site observations and testing required by the report.

Where required, compaction tests shall be performed by Geotechnical Engineer for engineered sub-grades (including roads, pads below slabs and other areas where compacted fill is required.) The Contractor shall schedule work to allow for lead times required to contact and schedule activities of the Geotechnical Engineer.

Stormwater Management Notes
Storm Water Management: Refer to Civil Plans for all storm water management requirements during construction period. Requirements will affect project scope as well as scope affecting construction means and methods.

Keynotes

Mark	Description
1	Align as indicated
2.0	Existing Paving, Flatwork to Remain
2.01	Existing Grades, Landscaping to Remain
2.02	Existing Utilities to remain
2.1	New concrete flatwork, curbs
2.11	Concrete stair
2.12	Pervious paving units
2.2	Drainage, stormwater features
2.21	Planting area
2.22	Drainage inlet
2.3	New AC paving

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Station 1 Modernization
20975 Geyserville Ave, Geyserville, CA 95441

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Architecture
Infrastructure
Environments

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650.696.1200

314 Center Street #220
Healdsburg, California
95448
707.431.1305

Set Purpose

Planning Submittal
Minor Use Permit Mod
Design Review

Edition History

Schematic Design Set	1/9/2026
50% Progress Set	3/19/2026
Planning Submittal	4/20/2026

Sheet Subject

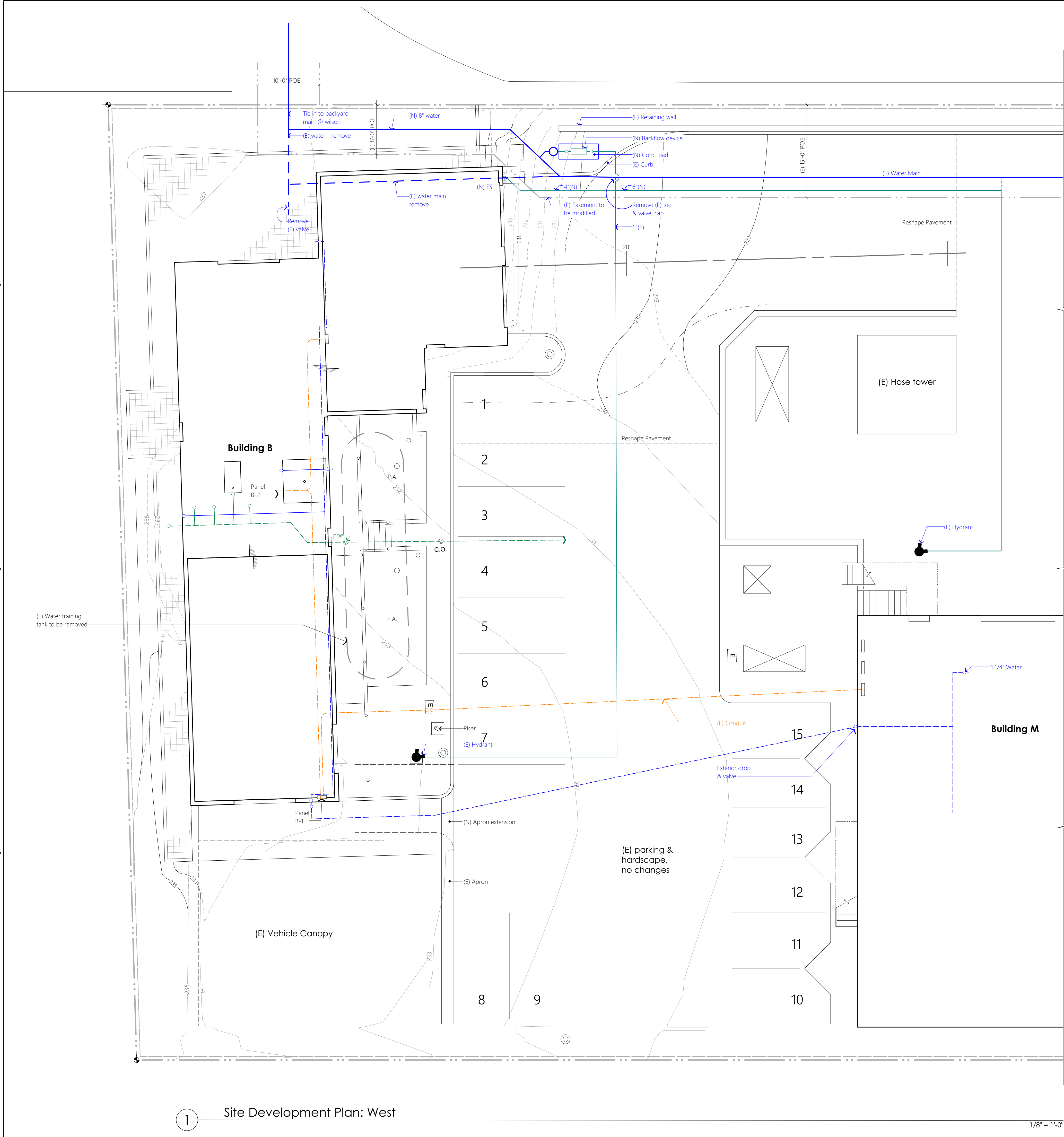
Site Plan

Program / Owner Proj #

DTA Project #

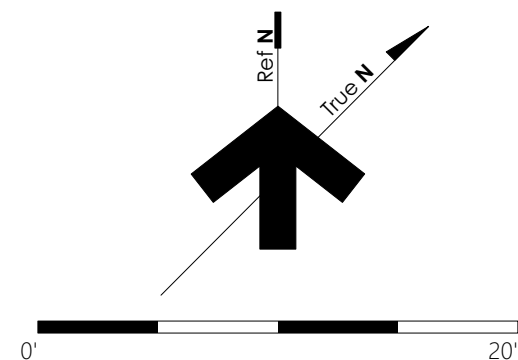
2525

A1.1



1 Site Development Plan: West

1/8" = 1'-0"



Site Notes

Refer A1.1 for site notes

Keynotes

Mark	Description
1	Align as indicated
2.0	Existing Paving, Flatwork to Remain
2.01	Existing Grades, Landscaping to Remain
2.02	Existing Utilities to remain
2.1	New concrete flatwork, curbs
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Design Review

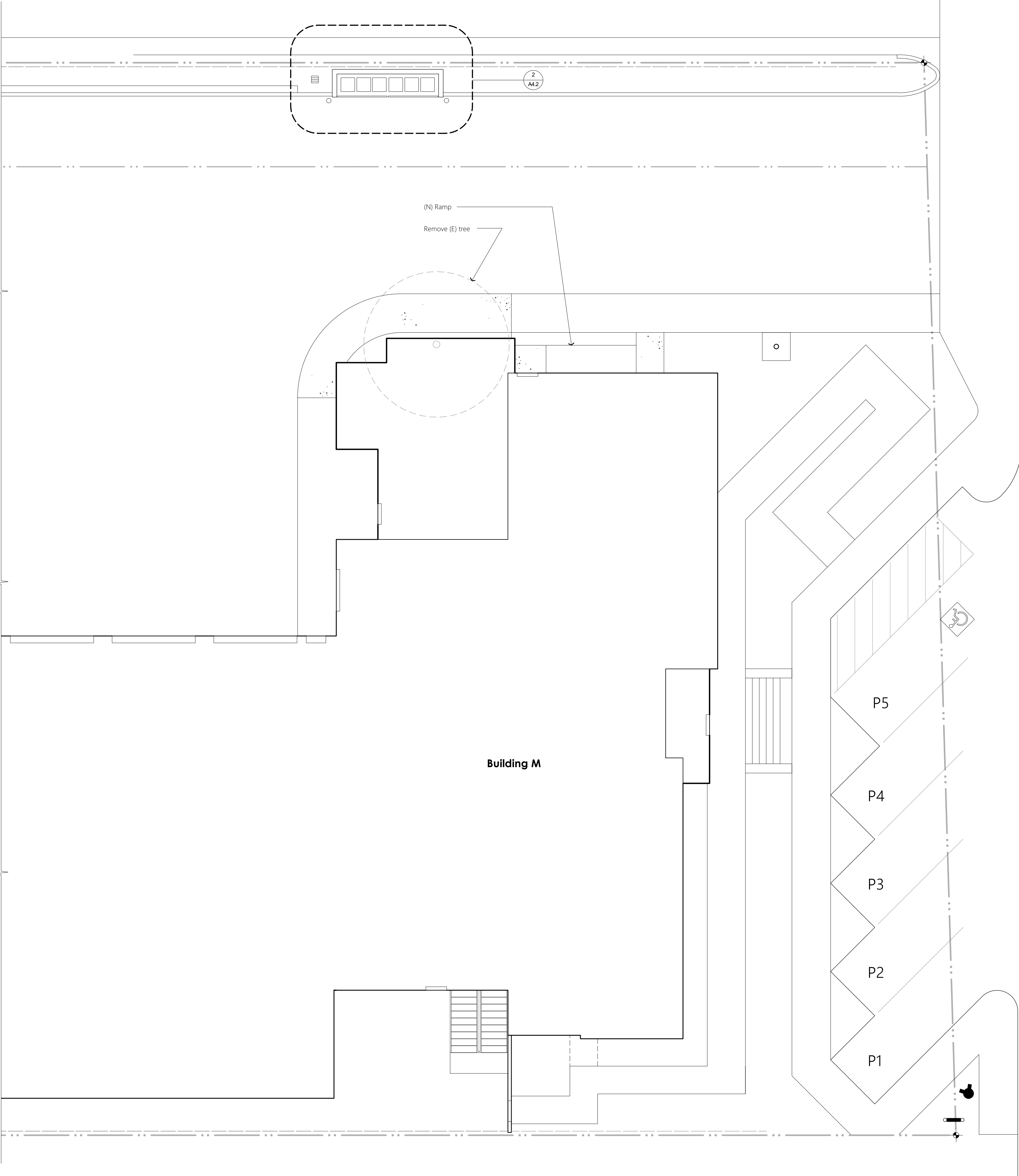
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50% Progress Set	3/19/2026
Planning Submittal	4/20/2026

Sheet Subject

West
Site Development Plan
Program / Owner Proj #

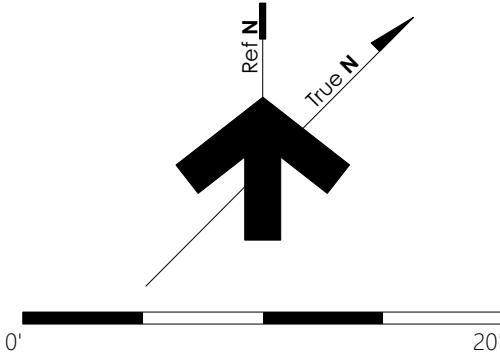
DTA Project #
2525

A1.2a



1 Site Development Plan: East

1/8" = 1'-0"



Site Notes

Refer A1.1 for site notes

Keynotes

Mark	Description
1	Align as indicated
2.0	Existing Paving, Flatwork to Remain
2.01	Existing Grades, Landscaping to Remain
2.02	Existing Utilities to remain
2.1	New concrete flatwork, curbs
2.11	Concrete stair
2.12	Pervious paving units
2.2	Drainage, stormwater features
2.21	Planting area
2.22	Drainage inlet
2.3	New AC paving

Northern Sonoma County Fire Protection District
Station 1 Modernization
20975 Geyserville Ave, Geyserville, CA 95441

Set Purpose
Planning Submittal
Minor Use Permit Mod
Design Review

Edition History	
Schematic Design Set	1/9/2026
50% Progress Set	3/19/2026
Planning Submittal	4/20/2026

Sheet Subject

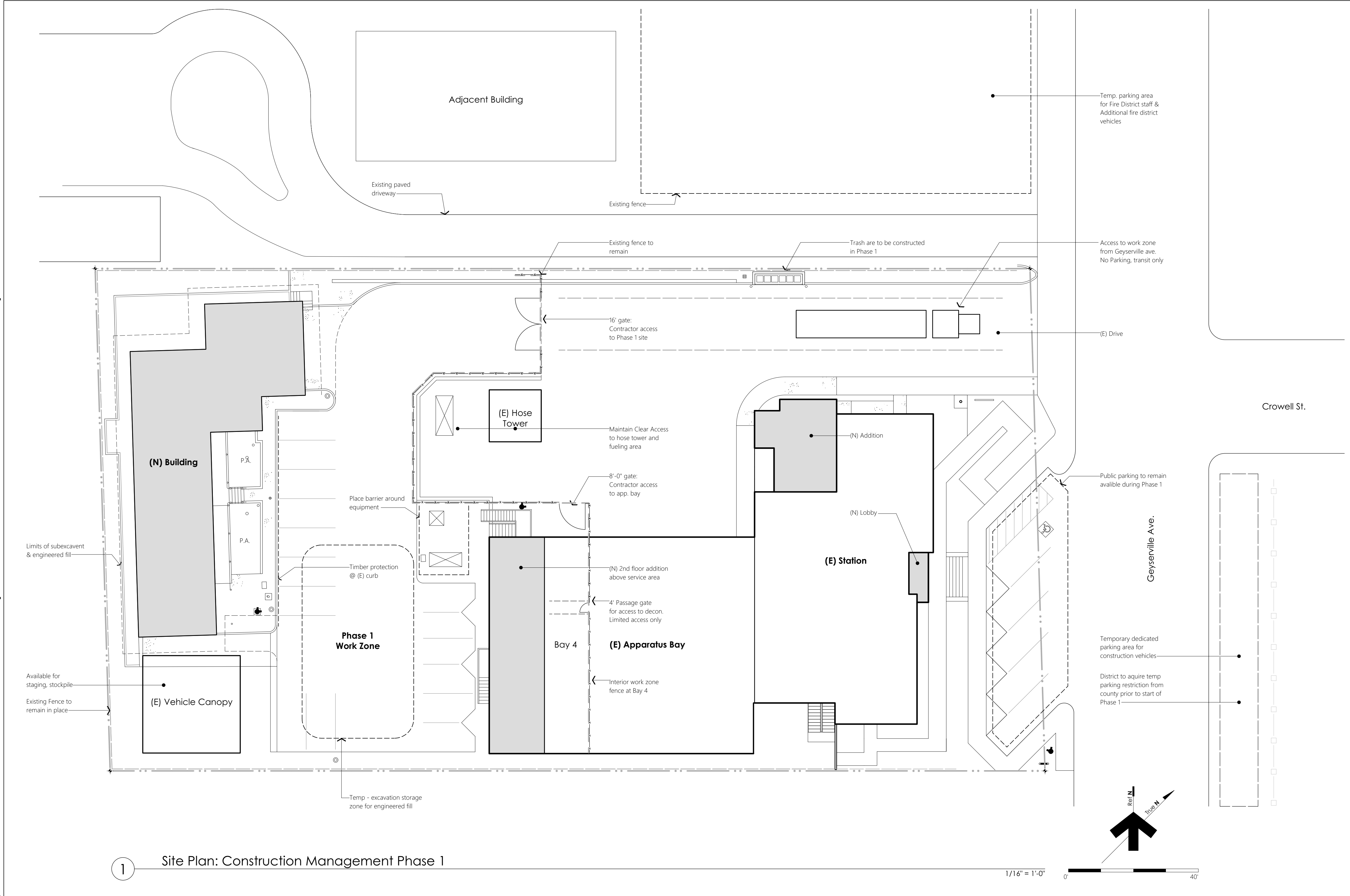
East
Site Development Plan
Program / Owner Proj #

DTA Project #
2525

A1.2b

DTA
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Architecture
Infrastructure
Environments

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707.431.1305



1 Site Plan: Construction Management Phase 1

Construction Mangement Notes

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Station 1 Modernization
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Set Purpose

Planning Submittal
**Minor Use Permit Mod
Design Review**

Edition History

Schematic Design Set	1/9/2026
50% Progress Set	3/19/2026
Planning Submittal	4/20/2026

Sheet Subject

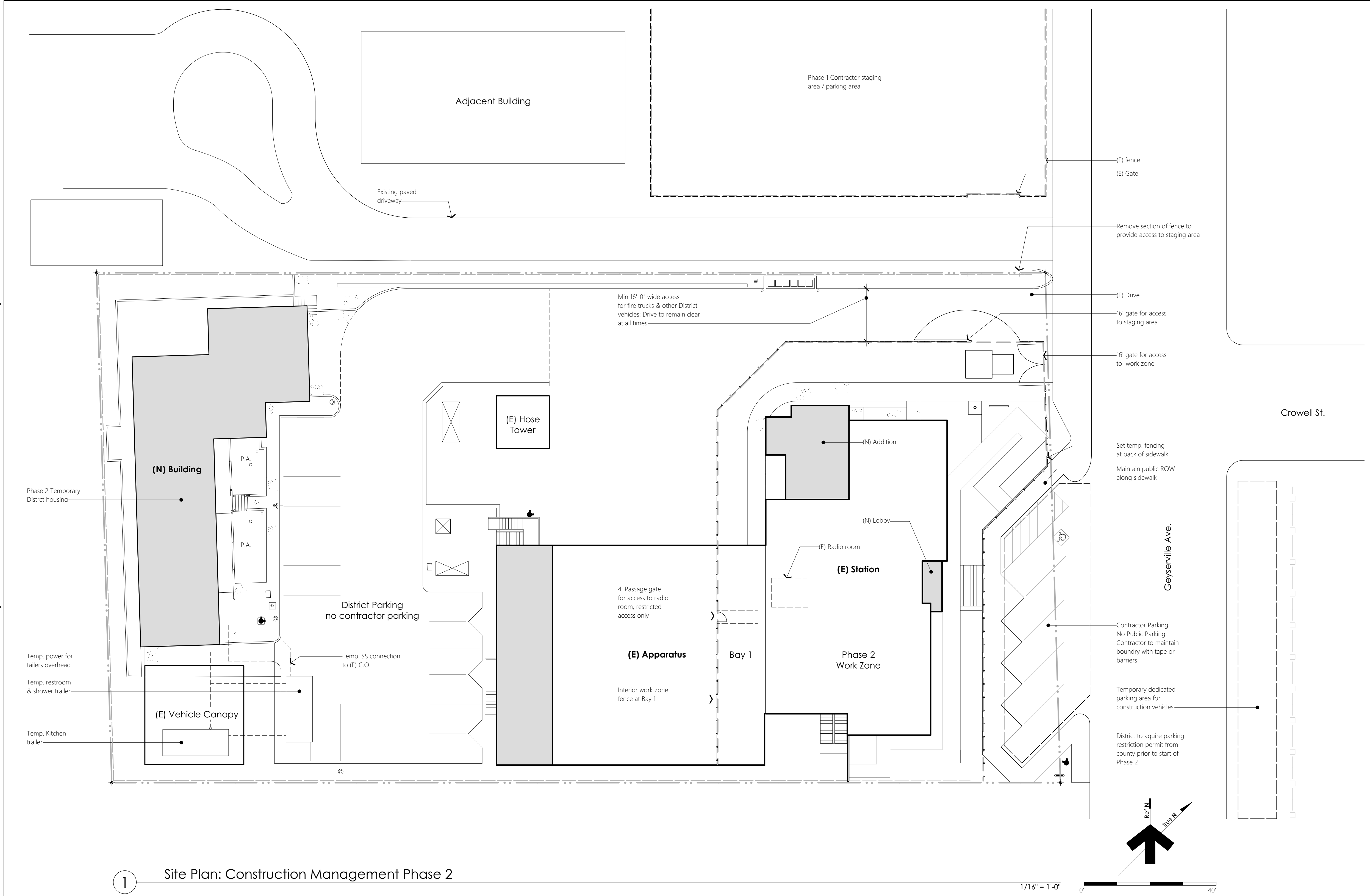
Phase 1
Construction Management
Site Plan

Program / Owner Proj #

DTA Project #

2525

A1.3a



1

Site Plan: Construction Management Phase 2

Construction Mangement Notes

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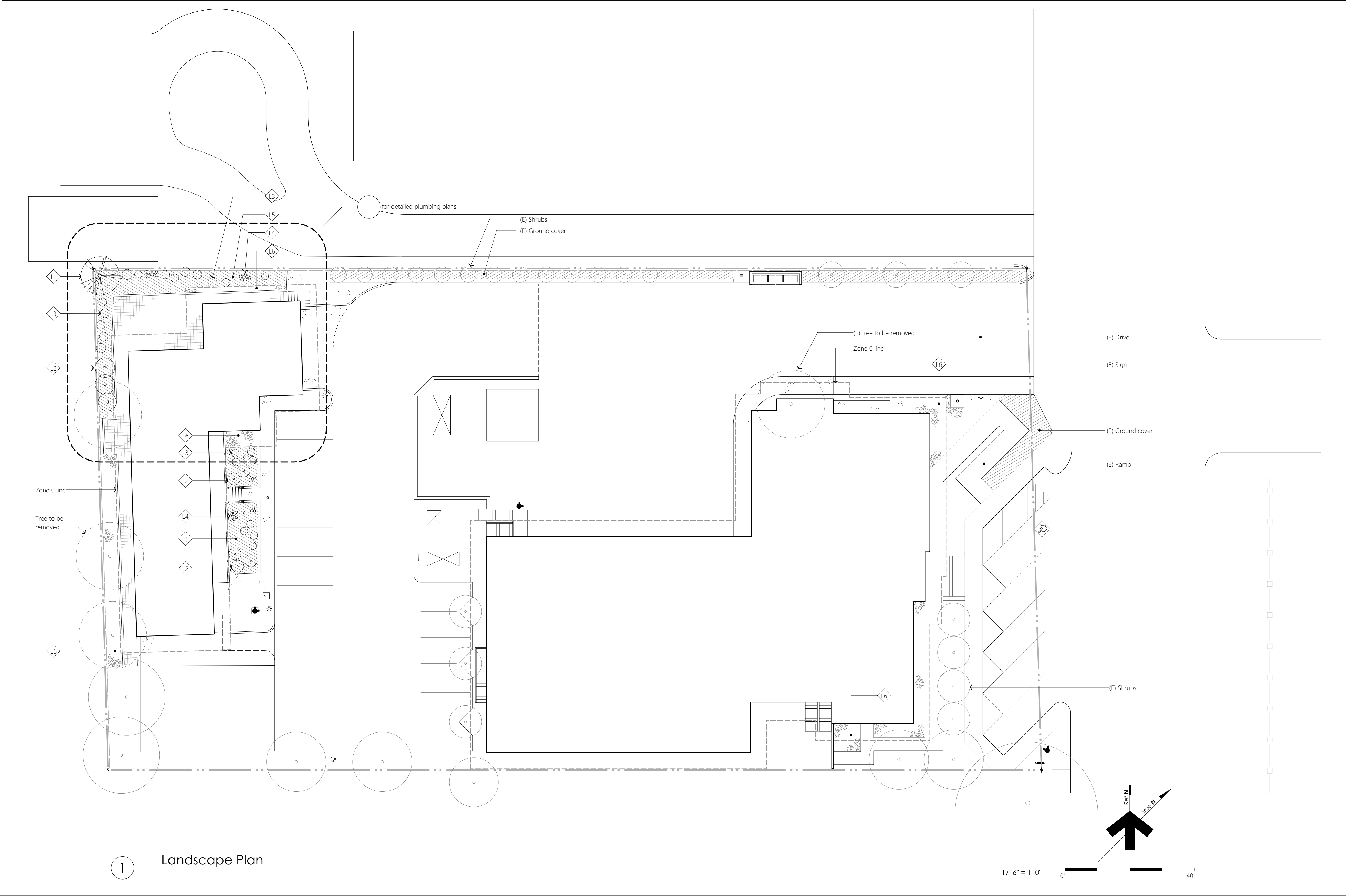
Set Purpose
Planning Submittal
Minor Use Permit Mod
Design Review

Edition History	
Schematic Design Set	1/9/2026
50% Progress Set	3/19/2026
Planning Submittal	4/20/2026

Sheet Subject
Phase 2
Construction Management
Site Plan

Program / Owner Proj #
DTA Project #
2525

A1.3b



Landscape Notes

This drawing represent a Schematic Landscape Planting Design only. They are intended for use by the Owner in securing the services of a qualified Landscape Contractor or for use by the owner as an instrument for Owner-Builder construction and are not intended to serve as complete construction documents.

The specific scope of the Design Build portion of the work includes:

- Finish grading after completion of hardscape
- Design and installation of a controlled irrigation system for use in establishing all planting
- Soil preparation
- Final plant selection in consultation with Architect
- Plant placement in consultation with Architect
- Plant installation and initial maintenance.

These drawings are intended for use in a negotiated contract and, therefore, may not detail or specify all materials, manufacturers or assemblies.

Schematic Design and Design Build Notes

The information contained in these documents is schematic in nature. This information is intended to be used by qualified persons to generate a completed design for Irrigation and Planting components of the project.

All hardscape layouts are controlled on the Architectural plans and are not part of the Design Build Landscape portion of the work.

Contractor is required to provide irrigation water to each planting area and to provide underground conduits for zone control.

Design-Build contractor shall provide control devices, control wiring, control connection and commissioning.

Site Preparation

Final grading and drainage are the responsibility of the Contractor and is controlled on Architectural and Civil plans. The contractor shall deliver final grading and hardscape per plans and the site shall ready for access by the Design Build contractor.

Topsoil in excess of that required for finish grading shall be stored on site or distributed as directed by Owner. Waste excavation from hardscape construction that is not suitable for landscape use and is in excess of that required for construction backfill shall be removed by Contractor.

Existing landscaping shall be protected as required to prevent any damage to plants and trees unless specified for removal by Owner.

Roadways shall be maintained clear of construction equipment or materials at all times.

Dust control measures shall be implemented as necessary.

Plant Selection and preparation

The Landscape Contractor is responsible for soil analysis and preparation. Soil amendments shall be provided as required for the plant materials specified. Include sand, organic materials and chemical fertilizers where appropriate.

Plant materials shall be purchased from a qualified wholesale nursery. Plants shall be selected individually and shall be checked for general health, damage, stress and disease.

Landscape Contractor is responsible for planting preparation, method and finishing. Planting shall be accomplished by industry standards relating to hole size, amendment within the hole, initial fertilization, backfill, staking and mulching.

All planting areas shall be mulched and otherwise prepared to reduce ground temperature and water evaporation.

Water quantity and watering frequency shall be matched to plant species, size, location and sun exposure.

Irrigation

Irrigation systems shown are schematic in nature and include valve location and zone identification only. The Landscape Contractor shall prepare detailed design of irrigation system, including size and type of heads, zone capacity, pipe size and location, and watering schedule.

System shall be designed to use minimum water quantities for plant materials specified. Where applicable, drip and low flow systems shall be provided. System shall be designed for flexibility so that water use can be reduced as plants mature and stabilize.

Irrigation systems shall be designed so that no water sprays on structures, fences or patio areas. (Minor overspray on shade brick patios is acceptable to facilitate growth of moss and lichens.)

Keynotes

Mark	Description
1	Align as indicated
2.0	Existing Paving, Flatwork to Remain
2.01	Existing Grades, Landscaping to Remain
2.02	Existing Utilities to remain
2.1	New concrete flatwork, curbs
2.11	Concrete stair
2.12	Pervious paving units
2.2	Drainage, stormwater features
2.21	Planting area
2.22	Drainage inlet
2.3	New AC paving
32.0	Existing hardscape and landscaping to remain
32.01	Existing decorative gravel
32.02	Existing public sidewalk and ramps
32.03	Existing fence to remain
32.04	Existing tree to be removed
32.8	Irrigation
32.81	Irrigation valve(s) and control location
32.9	Planting
32.90	Existing planting to remain
32.91	New planting areas

Plant Legend

Mark	Group Name	Common Names, Sci Name	Install Size
L1	Specimen Tree	Coast Redwood	15 gal
L2	Moderate Shrub	Toyon, Heteromeles arbutifolia	5 gal
		Manzanita, Howard McMinn	5 gal
		Oregon Grape, Mahonia Nervosa	5 gal
		Darwins Barberry, Barberis Darwinii	1 gal
		Coffeeberry, Frangula californica	5 gal
		Cotoneaster, Cotoneaster lacteus	
L3	Small Shrub	Service Berry, Amalanchier alnifolia	5 gal
		California Barberry, Mahonia pinnata	1 gal
		Bush Anenome, Carpenteria californica	1 gal
		Rock rose, Cistus	1 gal
L4	Incidental Ground Cover, fillers	Hookers Manzanita, Arctostaphylos hookeri	1 gal
		Bluebunch Fescue, Festuca Idahoensis	1 gal
		Western Sword Fern, Polystichum Munitum	1 gal
		Creeping Snowberry, Symphoricarpos Molis	1 gal
L5	Bark	Varried bark or shredded wood	
L6	Decorative Rock, Zone 0	Varried decorative rocks, match (E)	

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Station 1 Modernization

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Set Purpose

Planning Submittal

Minor Use Permit Mod

Design Review

Edition History

Schematic Design Set	1/9/2026
50% Progress Set	3/19/2026
Planning Submittal	4/20/2026

Sheet Subject

Program / Owner Proj #

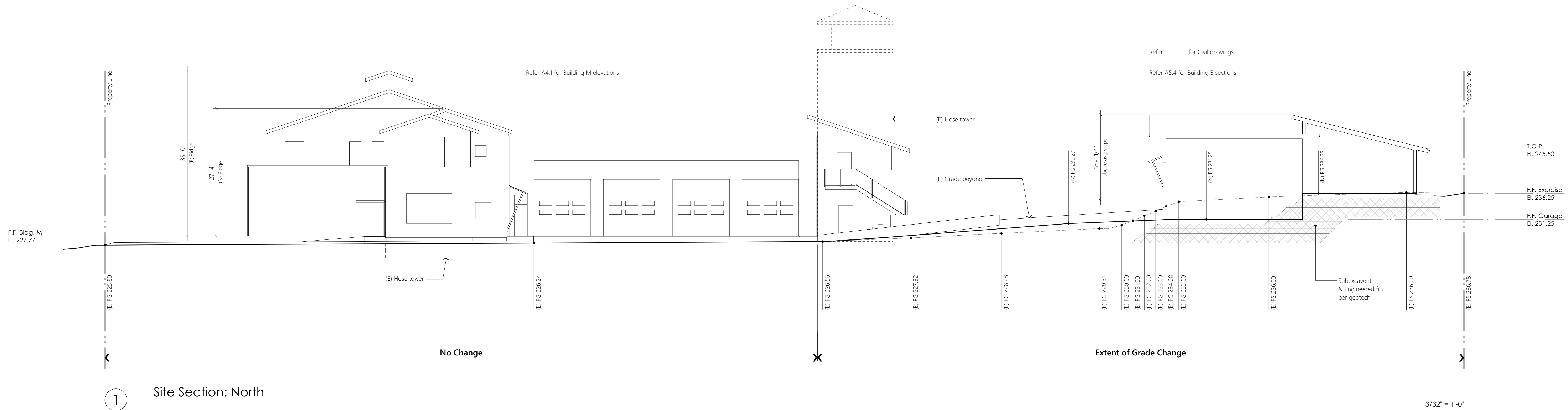
DTA Project #

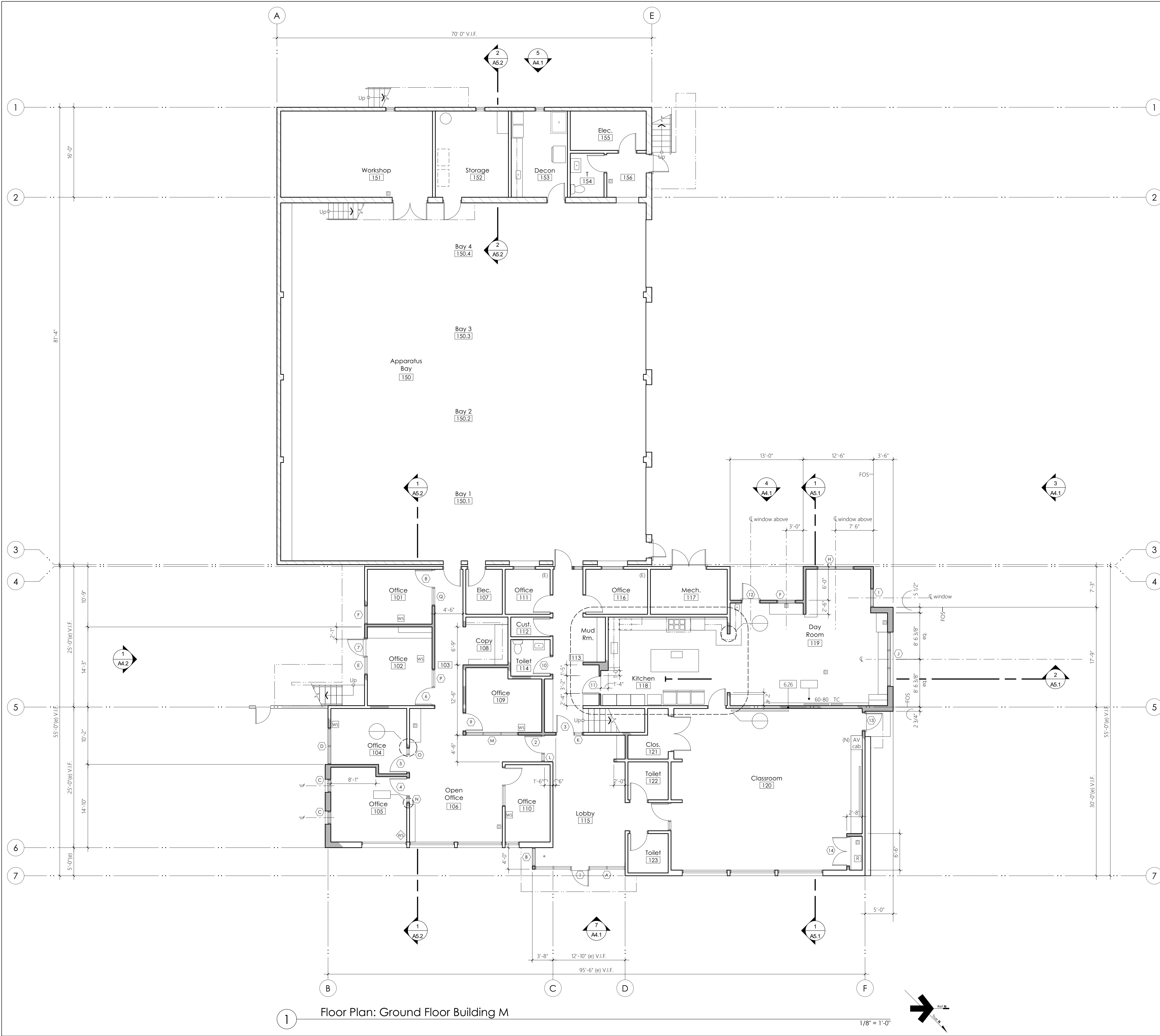
2525

Landscape Plan

A1.4

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1 Floor Plan: Ground Floor Building M

1/8" = 1'-0"

Floor Plan Notes

Keynote legend below contains information referenced throughout Documents. References are organized by standard CSI divisions and are flagged at various locations in the Documents by means of the following symbol:

Plans shall not be scaled. All dimensions given take precedence over scale. Contractor shall not scale drawings to determine dimensions without consulting with the Architect. Incorrect dimensions and resulting construction deficiencies due to scaling of Documents by the Contractor are the Contractor's responsibility and required corrections will be performed at the Contractor's expense.

Existing Construction Dimensions: All dimensions shown for existing construction are for convenience only and may not be accurate. Contractor shall verify all dimensions in field at start of construction and before any downstream work has been initiated, ordered or otherwise procured and advise Architect should any critical discrepancies exist. If discrepancies exist that affect other work Contractor and Architect shall coordinate necessary modifications. Such discrepancies shall not form the basis of a change to the contract.

Dimensions shown often refer to existing construction locations (existing FOS, for example). Contractor shall verify that such existing locations are accurately shown and that using such locations as starting points for new construction layout will not compromise downstream construction or installations. Should discrepancies be identified Contractor and Architect shall coordinate necessary modifications.

All Dimensions: Contractor shall review all dimensions shown herein for accuracy prior to construction. Contractor shall review any apparent discrepancies and either propose a resolution to, or request a resolution from, the Architect.

Dimensions given are to face of stud unless otherwise noted. Variations include:

FOC: Face of Concrete

FOM: Face of Masonry

Centerline

FOF: Face of Finish

Centerline Dimensions: Where centerline dimensions are used, they may indicate critical alignments with objects on other floors, other drawings or objects defined by other disciplines (such as plumbing, electrical, etc.). Contractor shall coordinate such dimensions throughout the set and across trades where they occur.

Plywood at Foundations: Tops of foundation walls shall receive exterior sheathing from walls framed above. Refer foundation details to verify whether a difference between foundation dimensions and wall framing dimensions is required. Adjust layout accordingly to ensure that foundation and framing are coordinated.

Demolition: Refer Demolition Plans for required demolition. Demolition plans may not define all required demolition. As systems are modified other areas may require selective demolition. Contractor shall be responsible for all demolition whether it is defined in the Demolition Plans, requested by a subcontractor or otherwise required for any aspect of the Work.

Existing Electrical Systems: Minor modifications are proposed for portions of the existing electrical, communications and signal systems. Modifications are defined by end locations of new equipment but may not address all necessary pipe runs, terminations, splice/J-box locations or other details that may be presently hidden from view. Contractor shall limit use of new J-boxes and shall begin modified runs either at existing panels, existing J-boxes, existing bus boxes or other locations compliant with codes. Refer Electrical Drawings for other requirements.

Critical Alignments: Critical alignments may occur between items installed by different trades. Contractor shall note all such items and notify affected trades. Sub-contractors shall review Documents and identify all such items that affect their work in any way.

Repeating items or assemblies may not be noted or dimensioned at all locations where repetition is obvious.

Refer Exterior Elevations for critical alignment of openings, architectural elements, assemblies and equipment.

Refer Interior Elevations for critical alignment of openings, fixtures, finishes, cabinet dimensions, vertical controls, millwork locations.

Refer Ceiling Plans for critical location of light fixtures, switches, etc.

Refer Clerestory Plans for location of windows that may have a critical alignment with openings or objects below.

Refer Electrical Plans for all other electrical devices. Contractor to provide framing, backing or other supports to accommodate recessed fixtures and other items with critical locations. Refer Structural Drawings for location of posts, double studs, special floor and wall framing, special connections, shear nailing.

Existing Critical Shear Walls: A portion of existing walls are critical shear walls and shall not be modified. Such walls are noted on the Demolition Plans and require surgical demolition. Should actual conditions occur that vary from noted critical shear walls Contractor shall advise Architect for resolution. Contractor shall not compromise critical shear walls in any way. Refer to Structural Plans for other limitations.

Keynotes

Mark	Description
1.0	Align as indicated.
1.01	Line of roof, floor above
1.02	Line of cabinet, assembly, structure above
1.03	Line of wall below
3.1	New conc slab, foundation, ref struct dwgs
3.14	Conc. Curb at exterior walls
4.0	Existing CMU wall
4.01	New opening in existing CMU wall
4.1	Existing mtl stair, landing, railing.
5.0	Existing mtl stair, landing, railing.
5.1	Structural steel framing, ref struc dwgs
5.3	Mtl stair, landing
5.5	Metal Fence, Gate
6.01	Existing framed wall/floor/ceiling to remain
6.02	Existing framed wall/floor/ceiling to be removed. Refer Demolition Plans
6.2	Wood framing, ref struc
6.24	2x4 STUDS @ 16
6.26	2x6 STUDS @ 16
6.28	2x8 STUDS @ 16
6.31	Decorative wood railing
6.4	New wood cabinets. Refer Interior Elevations
8.0	Existing door / window / skylight. to remain
8.01	Salvage existing door for new frame, opening
8.1	Aluminum storefront system
8.2	Aluminum windows
8.3	Passage doors
8.4	Overhead Door
8.41	Sectional Overhead Door
8.42	Coiled Overhead Door
9.4	Ceramic Tile
9.51	Floor Tile over setting bed on framed floor
9.52	Floor tile over setting bed in recessed slab
9.53	Sheet flooring: vinyl/linoileum
9.54	Vinyl composition tile
9.55	Finished concrete
9.57	Carpet
9.91	Conc floor sealer
10.95	Projection Screen
12.3	Epoxy Resin Countertop, Splash
15.2	Plumbing fixture, ref Plumbing dwgs
15.21	Sink, ref plumbing
15.22	Integral countertop sink, ref plumbing, int dwg
15.60	Hard Pipe Downspout, by Plumbing
16.1	Elec equip, ref elec dwgs

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Set Purpose

Planning Submittal
Minor Use Permit Mod
Design Review

Edition History

Schematic Design Set	1/9/2026
50% Progress Set	3/19/2026
Planning Submittal	4/20/2026

Sheet Subject

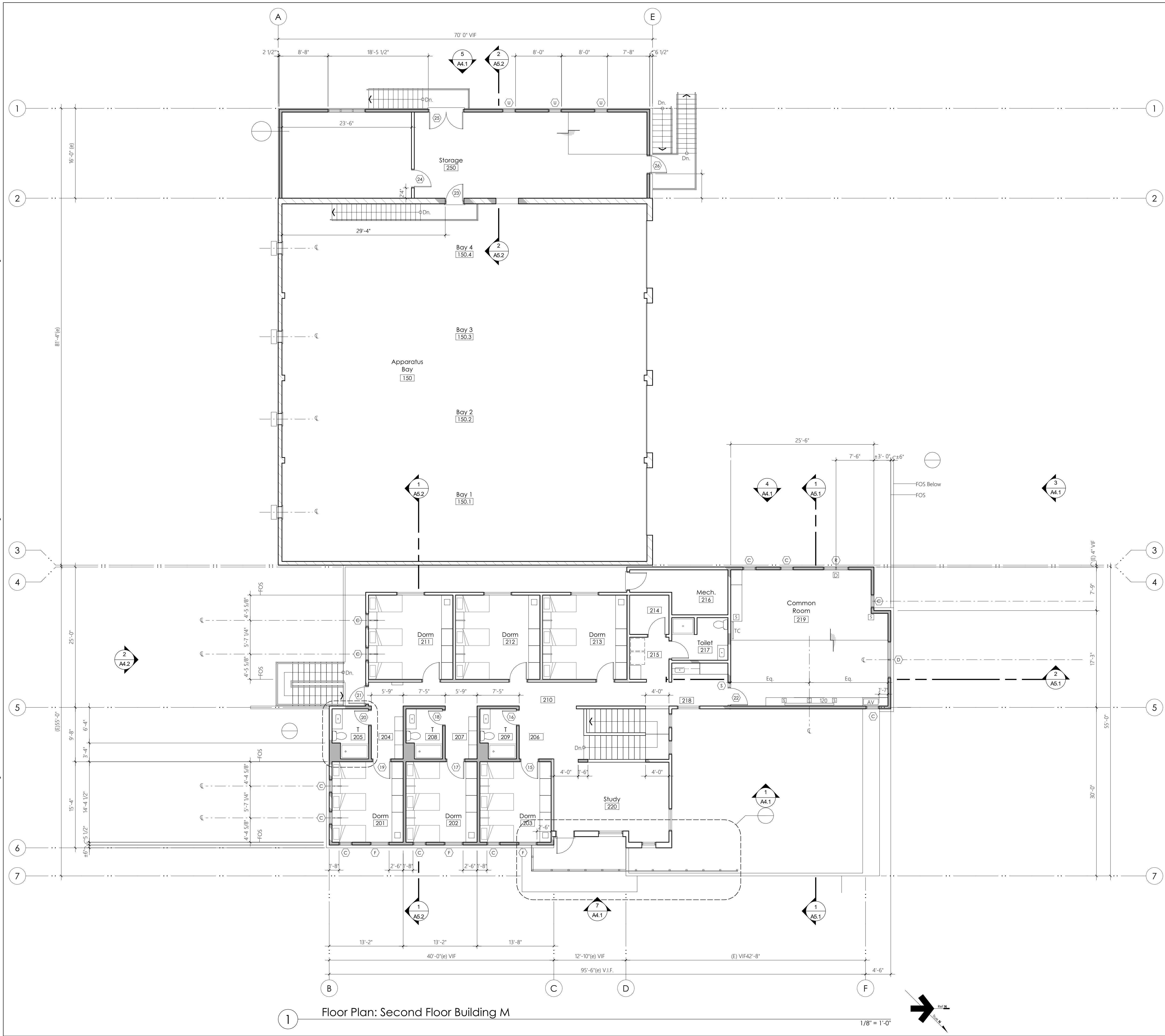
Building M Ground Floor
Floor Plan

Program / Owner Proj #

DTA Project #

2525

A2.1



Floor Plan Notes

Ref A2.1 for floor plan notes

Keynotes

Mark	Description
1.0	Align as indicated
1.01	Line of roof, floor above
1.02	Line of cabinet, assembly, structure above
1.03	Line of wall below
3.1	New conc slab, foundation, ref struct dwgs
3.14	Conc. Curb at exterior walls
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8.01	Salvage existing door for new frame, opening
8.1	Aluminum storefront system
8.2	Aluminum windows
8.3	Passage doors
8.4	Overhead Door
8.41	Sectional Overhead Door
8.42	Coiled Overhead Door
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9.91	Conc floor sealer
10.95	Projection Screen
12.3	Epoxy Resin Countertop, Splash
15.2	Plumbing fixture, ref Plumbing dwgs
15.21	Sink, ref plumbing
15.22	Integral countertop sink, ref plumbing, int dwg
15.60	Hard Pipe Downspout, by Plumbing
16.1	Elec equip, ref elec dwgs

Set Purpose

Planning Submittal

Minor Use Permit Mod Design Review

Edition History

Revision	Description	Date
1	Schematic Design Set	1/9/2026
2	50% Progress Set	3/19/2026
3	Planning Submittal	4/20/2026

Sheet Subject

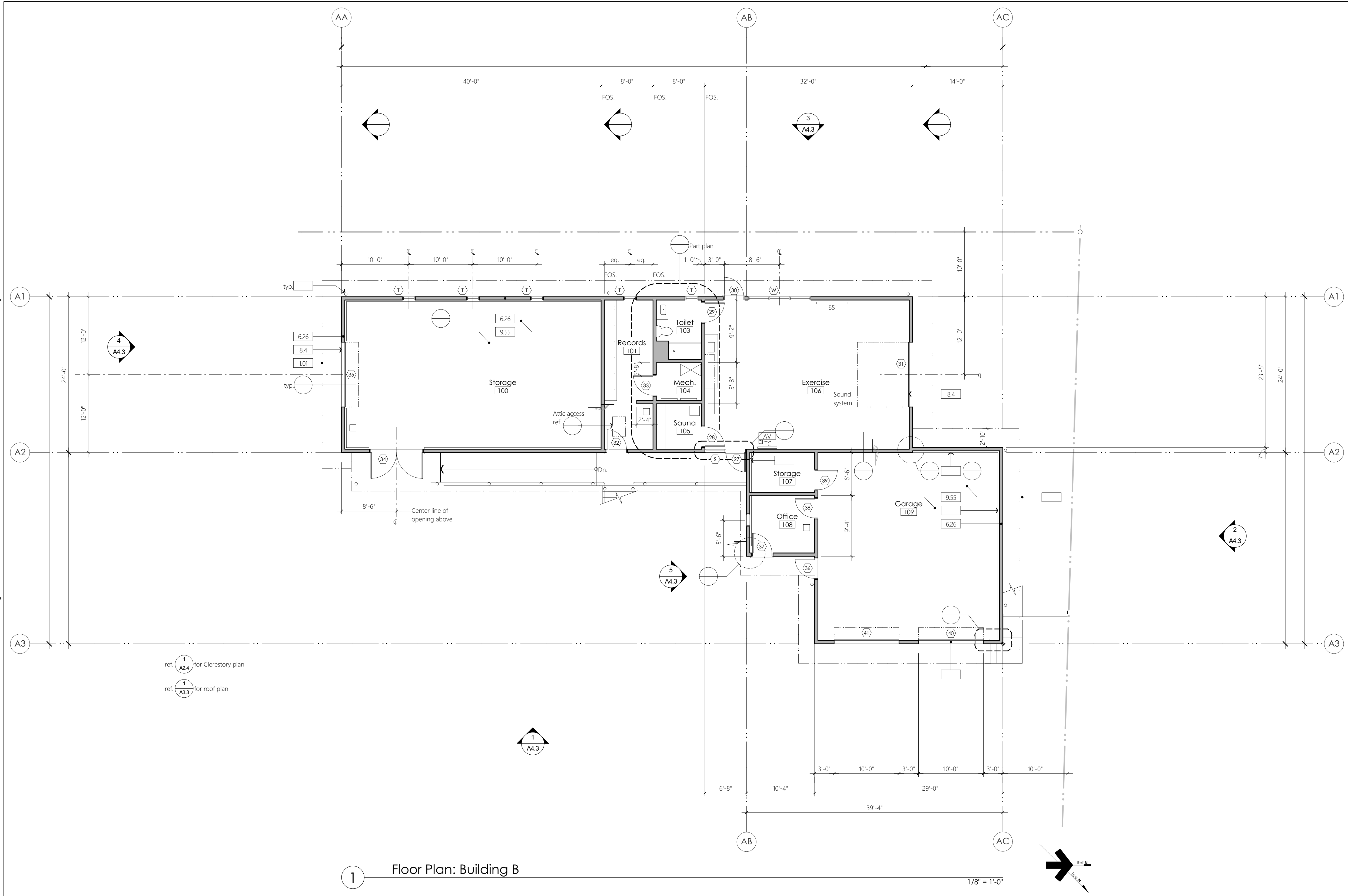
Building M Second Floor

Floor Plan

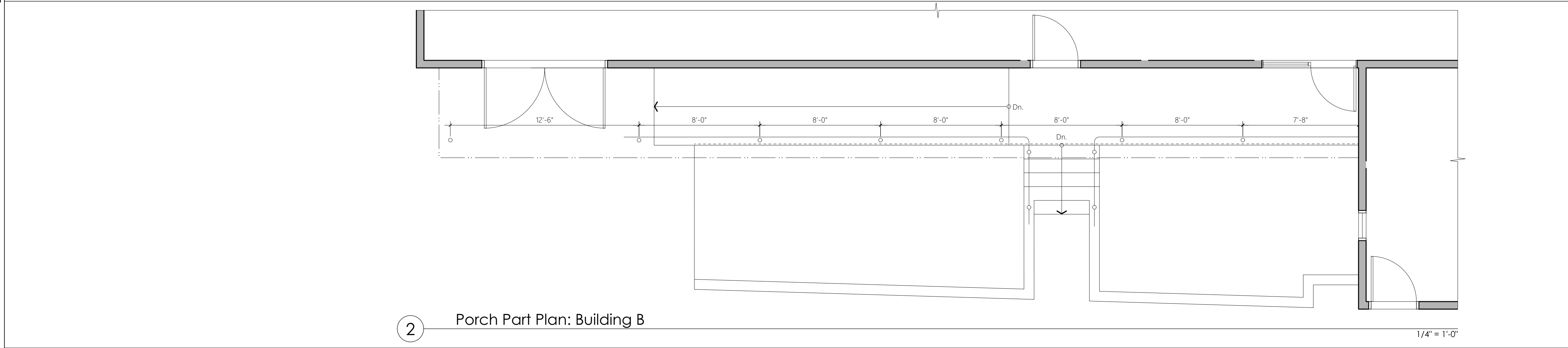
DTA Project #

2525

A2.2



1 Floor Plan: Building B



2 Porch Part Plan: Building B

Floor Plan Notes

Ref A2.1 for floor plan notes

Keynotes

Mark	Description
1.0	Align as indicated
1.01	Line of roof, floor above
1.02	Line of cabinet, assembly, structure above
1.03	Line of wall below
3.1	New conc slab, foundation, ref struct dwgs
3.14	Conc. Curb at exterior walls
4.0	Existing CMU wall
4.01	New opening in existing CMU wall
4.1	
5.0	Existing mtl stair, landing, railing
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5.3	Mtl stair, landing
5.5	Metal Fence, Gate
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6.26	2x6 STUDS @ 16
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Set Purpose
Planning Submittal
Minor Use Permit Mod
Design Review

Edition History	
Schematic Design Set	1/9/2026
50% Progress Set	3/19/2026
Planning Submittal	4/20/2026

Sheet Subject

Building B
Floor Plan

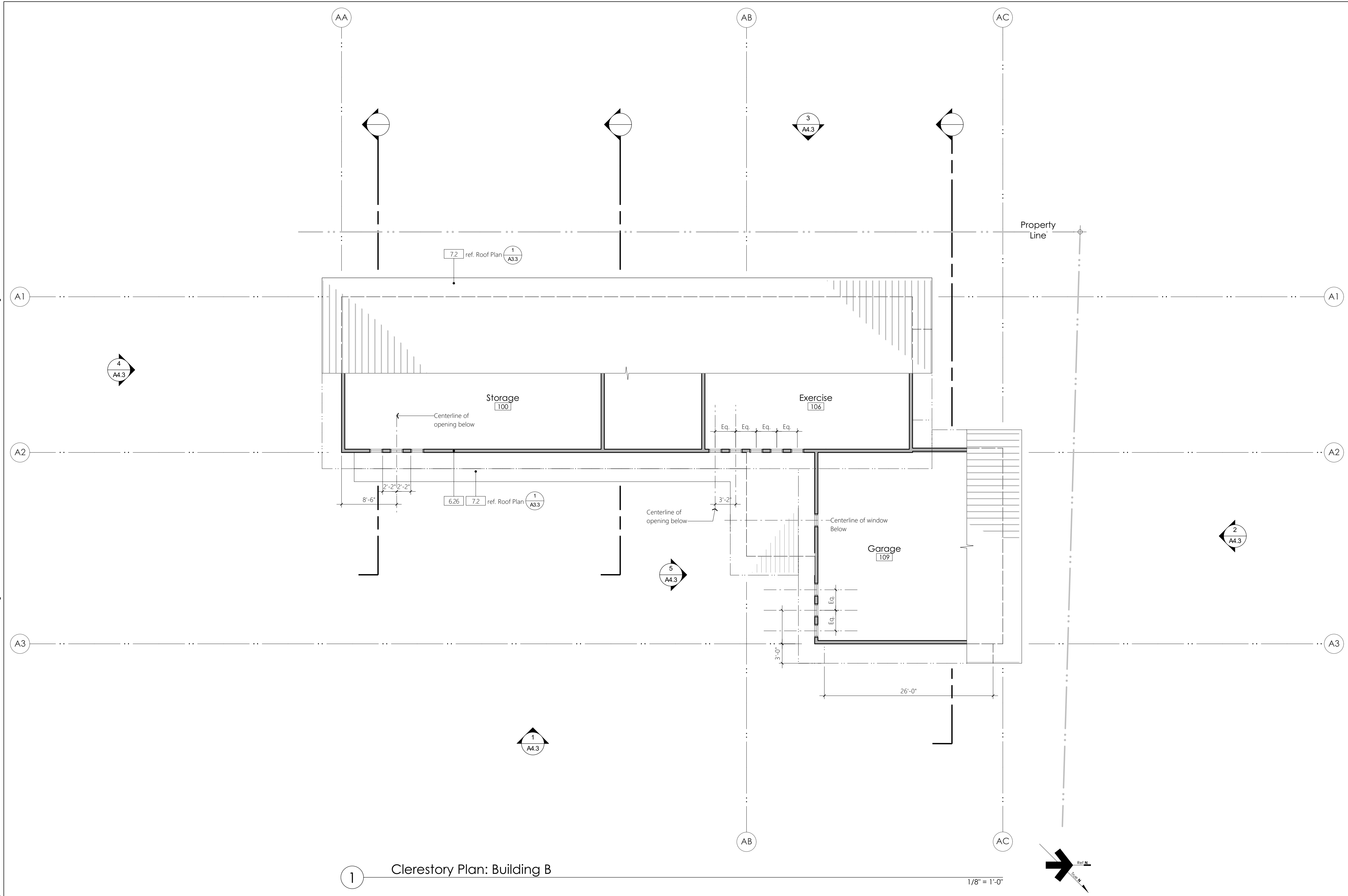
Program / Owner Proj #

DTA Project #

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Building B
Floor Plan

A2.3



Floor Plan Notes

Ref A2.1 for floor plan notes

Keynotes	
Mark	Description
1.0	Align as indicated
1.01	Line of roof, floor above
1.02	Line of cabinet, assembly, structure above
1.03	Line of wall below
3.1	New conc slab, foundation, ref struct dwgs
3.14	Conc. Curb at exterior walls
4.0	Existing CMU wall
4.01	New opening in existing CMU wall
4.1	
5.0	Existing mtl stair, landing, railing,
5.1	Structural steel framing, ref struc dwgs
5.3	Mtl stair, landing
5.5	Metal Fence, Gate
6.01	Existing framed wall/floor/ceiling to remain
6.02	Existing framed wall/ floor/ ceiling to be removed. Refer Demolition Plans
6.2	Wood framing, ref struc
6.24	2x4 STUDS @ 16
6.26	2x6 STUDS @ 16
6.28	2x8 STUDS @ 16
6.31	Decorative wood railing
6.4	New wood cabinets. Refer Interior Elevations
8.0	Existing door / window / skylight. to remain
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15.2	Plumbing fixture, ref Plumbing dwgs
15.21	Sink, ref plumbing
15.22	Integral countertop sink, ref plumbing, int dwg
15.60	Hard Pipe Downspout, by Plumbing
16.1	Elec equip, ref elec dwgs

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Design Review

Edition History

Schematic Design Set	1/9/2026
50% Progress Set	3/19/2026
Planning Submittal	4/20/2026

Sheet Subject

Program / Owner Proj #

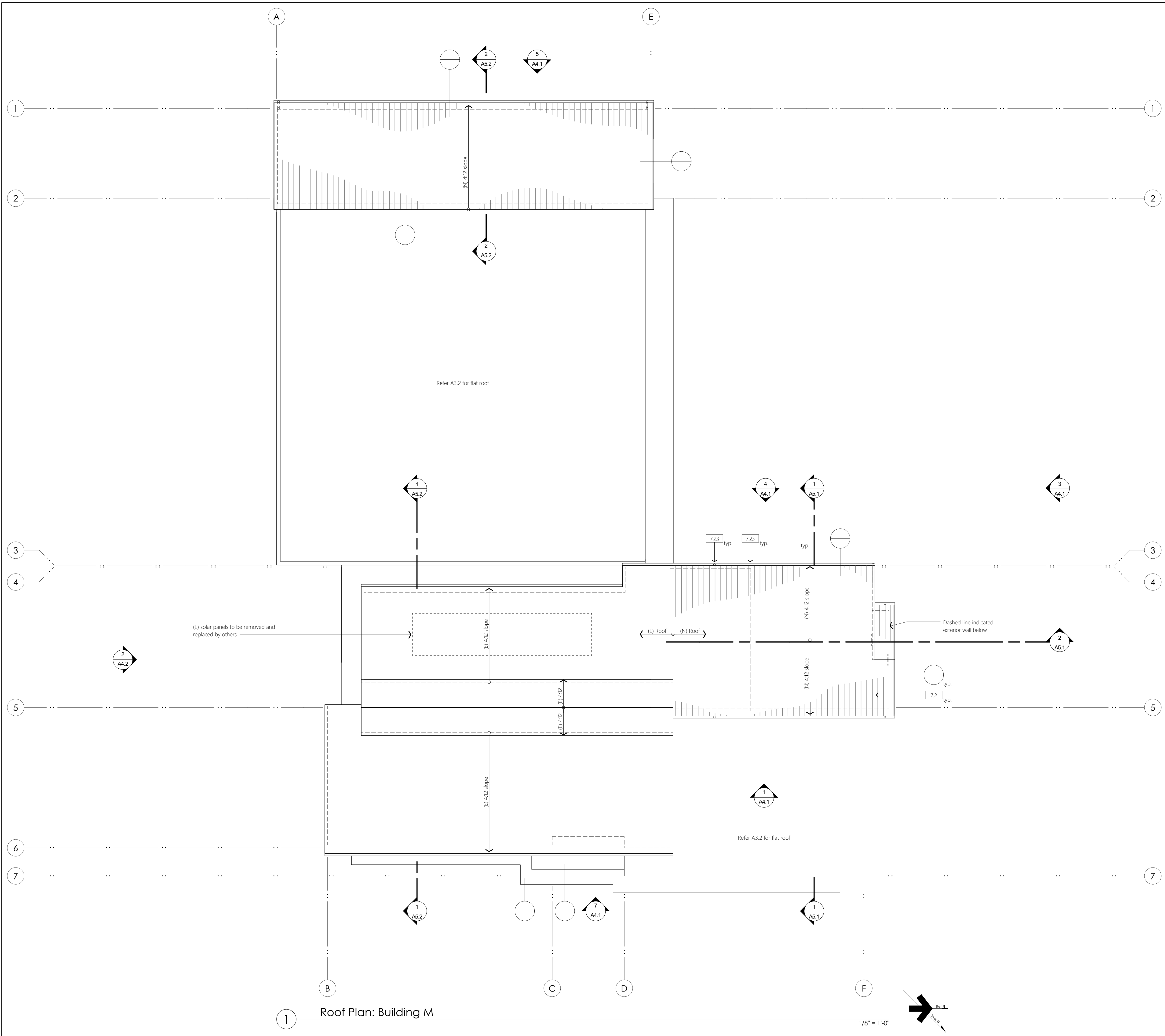
Building B
Clerestory Plan

DTA Project #

2525

Building B
Clerestory Plan

A2.4



Roof Notes

Keynote legend below contains information referenced throughout Documents. References are organized by standard CSI divisions and are flagged at various locations in the Documents by means of the following symbol:

All roof materials shall be part of a specified roofing system as defined by the system manufacturer specified. All materials shall be from a single source and shall be assembled and integrated as specified by the manufacturer. In no case may materials be substituted which are not listed in specified manufacturer information.

Roof shall be installed per Roof Manufacturers standards such that all warranties are maintained. Contractor shall note any conflicts between Roof manufacturer's instructions and requirements of these drawings.

Roofing substrate, base sheets and any installed components shall be fully protected from subsequent construction activities until completion of project. Contractor shall provide temporary protection for all roof components where subsequent work may occur.

Contractor shall protect all roof components from exposure to sunlight where such exposure may damage materials. Installation of felts or base sheets shall be covered immediately by either subsequent roof layers or protective materials to avoid exposure to sun or general drying of materials.

Contractor shall fully seal all roof penetrations per roof manufacturer's specifications. Provide flashing, counter flashing, coping and reglets as necessary to ensure positive drainage across all surfaces.

Provide gutters, rainwater leaders and downspouts to match existing. Integrated gutters shall be constructed in similar fashion to existing and shall provide identical visual results. Gutters shall be fully soldered and shall be fabricated in minimum 20' lengths to reduce number of joints.

Provide conc. splash block at grade at all downspouts not connected to subgrade drainage system. Refer Civil Drawings for additional requirements related to Stormwater Management.

Metal Roof
Metal Roof shall match existing in color, finish and dimension. Contractor shall insure that all details including flashing, counterflashing, panel terminations match existing unless specifically directed otherwise in plans and details.

Flat Roof
Flat roof shall be fully replaced. Replacement shall require lifting and removal of all roof mounted equipment, piping. Contractor shall coordinate which items shall be discarded and which items shall be retained for re-installation.

Contractor shall coordinate which trades perform which portions of removal and replacement based on specific items. Contractor shall insure that no items that require removal or replacement by a specific trade (such as plumbers) are removed, damaged, disposed of by other trades or forces.

Once existing roof is removed, Contractor shall coordinate all work required for new installations on new roof as though this were a new roof installation on a new building. Old roof removal and new roof installation shall be considered separate tasks and shall allow for construction by other trades as required between tasks.

Gutters
Provide self-flashing gutters. All gutters shall be continuous for entire length of eaves where length is 20' or less. Joints will be allowed for lengths greater than 20'. All joints shall be fully lapped and soldered. Corners shall be mitered and soldered. Do not use prefabricated connectors. Downspout leaders shall be cut into standard gutter sections and soldered. Expansion joints shall be provided at runs over 50' in length. Expansion joints shall be lap type per SMACNA figure 1-6.

Do not use prefabricated connectors. Downspout leaders shall be cut into standard gutter sections and soldered.

Downspouts shall connect to site drainage system via manufactured bushings and transitions. Downspouts shall connect between 6" and 12" above finish grade. Contractor shall coordinate final finish grade with downspout installer.

Provide conc. splash block at grade at all downspouts not connected to drainage system. Downspouts occurring over raised patios shall be connected to plastic flex pipe below patio and shall daylight at adjacent grade.

Provide metal dissipaters on roof where gutters or downspouts discharge onto roof surfaces. On shingle roofs, dissipaters shall be woven into shingle layout similar to step flashing.

Replacement Preparation
All roofing shall be installed such that it can be removed for regular replacement without removing flashing or other adjacent construction. Roofing shall not be attached below flashing such that it can be slipped out from under flashing and new roofing slipped back under flashing when roof replacement occurs.

Keynotes

Mark	Description
1	Align as indicated
1.01	Line of roof, floor above
1.03	Line of wall below
7.01	Existing Mtl roof
7.02	Existing Built Up Roof
7.03	Existing flashing, cover
7.2	New Metal Roof
7.21	Sheet Metal flashing, cricket
7.22	Metal coping, cover panel, edge flashing
7.23	Metal gutter, downspout
7.24	Metal Fascia, Painted
7.3	New Built up roof
7.31	New Traffic Membrane Roof
7.32	Traffic Pads
7.33	Cricket
7.34	Roof drain/overflow
8.5	Skylight
8.51	New Skylight in existing opening
16.2	PV Panels, applied to metal roof, ref elec.

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Set Purpose

Planning Submittal

Minor Use Permit Mod Design Review

Edition History

Schematic Design Set	1/9/2026
50% Progress Set	3/19/2026
Planning Submittal	4/20/2026

Sheet Subject

Building M
Roof Plan

Program / Owner Proj #

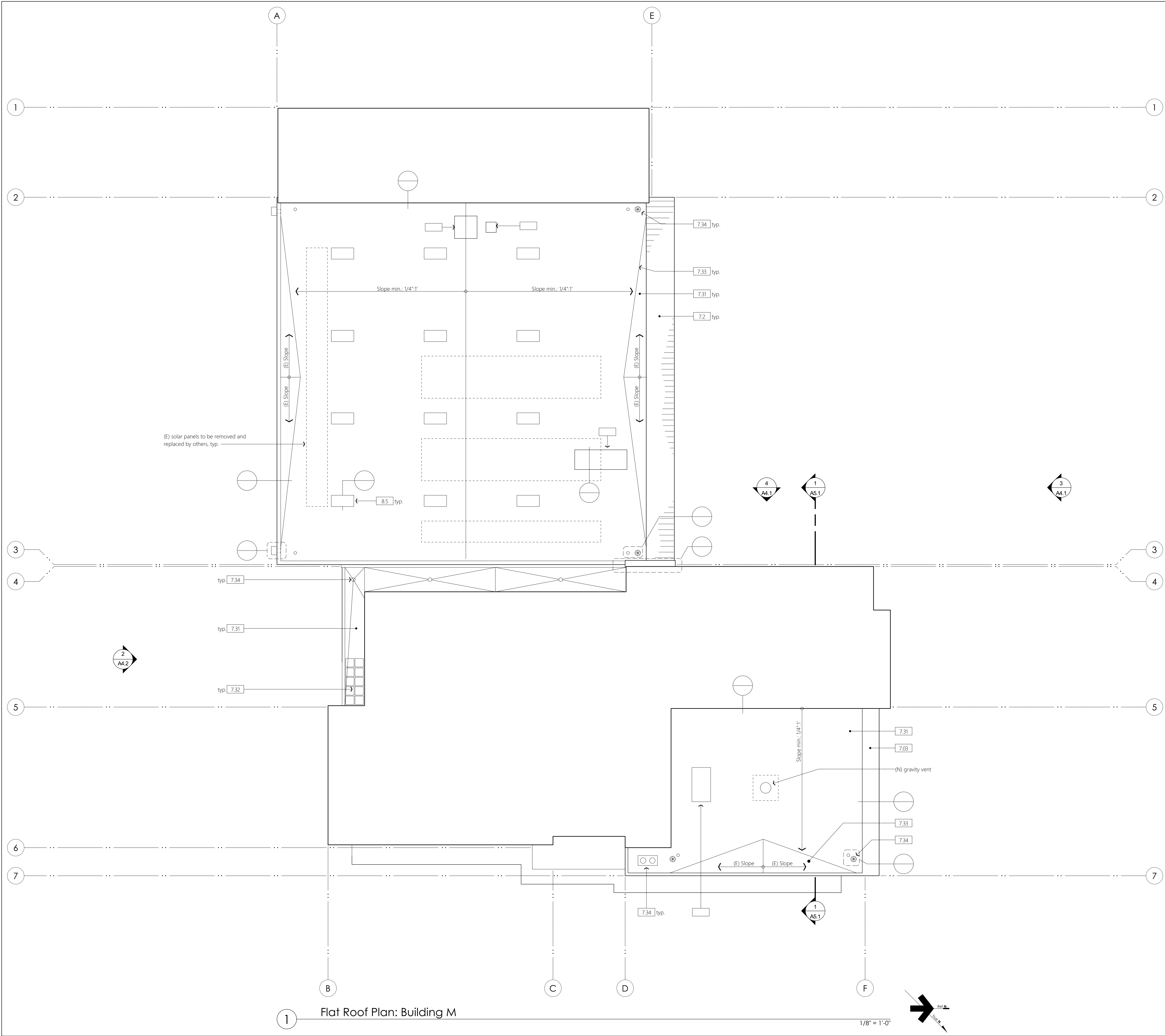
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Building M
Roof Plan

A3.1

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1 Flat Roof Plan: Building M

Roof Notes

Ref A3.1 for roof notes

Keynotes

Mark	Description
1	Align as indicated
1.01	Line of roof, floor above
1.03	Line of wall below
7.01	Existing Mtl roof
7.02	Existing Built Up Roof
7.03	Existing flashing, cover
7.2	New Metal Roof
7.21	Sheet Metal flashing, cricket
7.22	Metal coping, cover panel, edge flashing
7.23	Metal gutter, downspout
7.24	Metal Fascia, Painted
7.3	New Built up roof
7.31	New Traffic Membrane Roof
7.32	Traffic Pads
7.33	Cricket
7.34	Roof drain/overflow
8.5	Skylight
8.51	New Skylight in existing opening
16.2	PV Panels, applied to metal roof, ref elec.

Planning Submittal

Minor Use Permit Mod Design Review

Revision Table

Revision	Description	Date
1	Initial Design	1/9/2026
2	50% Progress Set	3/19/2026
3	Planning Submittal	4/20/2026

Sheet Subject

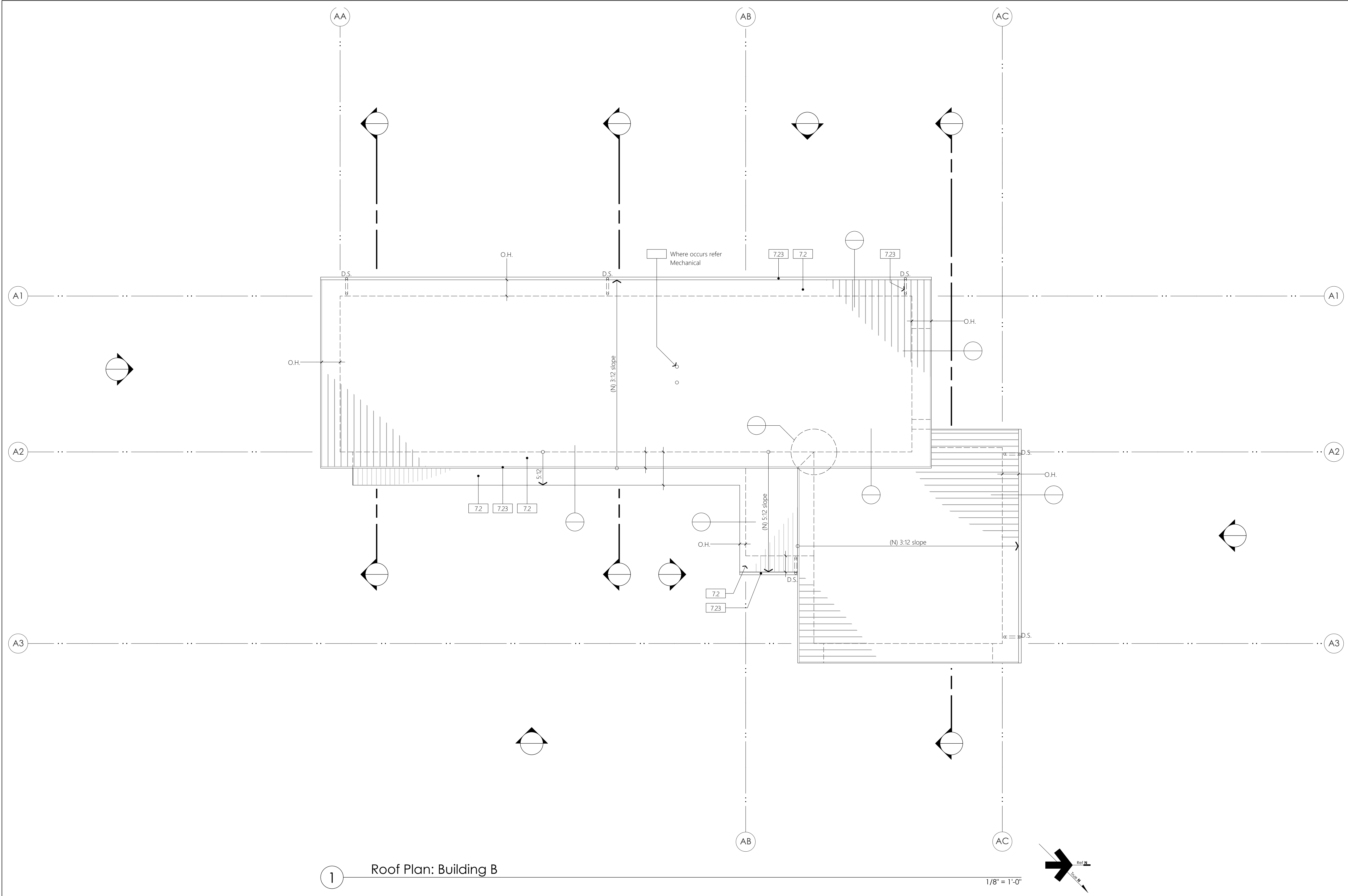
Building M
Roof Plan

DTA Project #

2525

DTA Project #

2525



1 Roof Plan: Building B

1/8" = 1'-0"

Roof Notes

Ref A3.1 for roof notes

Keynotes

Mark	Description
1	Align as indicated
1.01	Line of roof, floor above
1.03	Line of wall below
7.01	Existing Mtl roof
7.02	Existing Built Up Roof
7.03	Existing flashing, cover
7.2	New Metal Roof
7.21	Sheet Metal flashing, cricket
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Design Review

Edition History

Schematic Design Set	1/9/2026
50% Progress Set	3/19/2026
Planning Submittal	4/20/2026

Sheet Subject

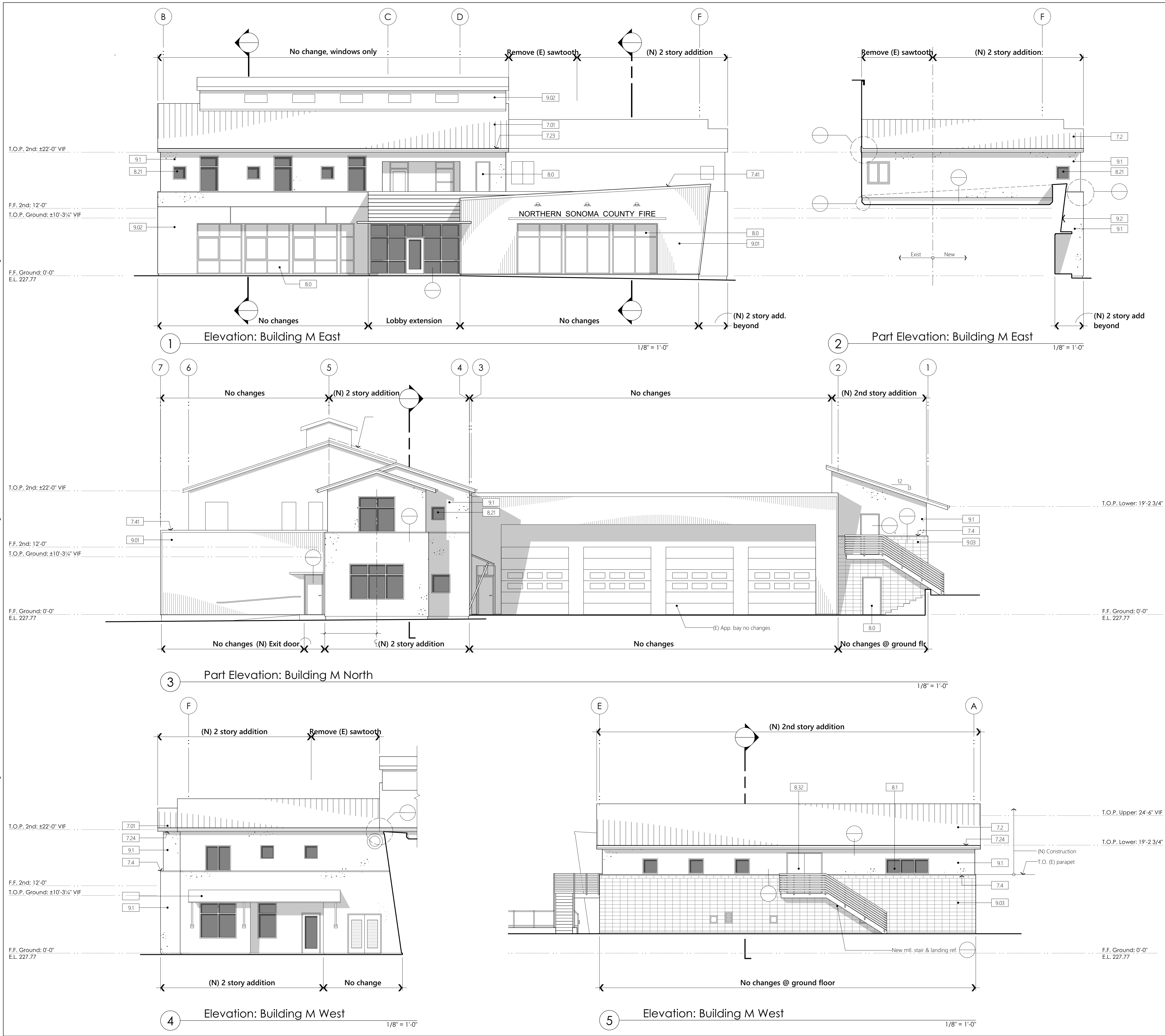
Building B
Roof Plan

Program / Owner Proj #

DTA Project #

2525

A3.3



Elevation Notes

Keynote legend below contains information referenced throughout Documents. References are organized by standard CSI divisions and are flagged at various locations in the Documents by means of the following symbol:

All new exterior surfaces shall be cleaned and painted. Existing surfaces shall be fully painted to ensure complete match with new surface colors, textures, quality.

All metal flashing, flues, coping, gutters, downspouts, and other mechanical appurtenances shall be fully primed and painted.

Exterior light fixtures and mechanical penetrations shall be fully caulked. All structural steel shall be cleaned, primed and painted, including existing.

Refer roof plans for roof detail callouts.

Refer floor plans for additional detail references.

Refer site plan/civil drawings for grading, drainage.

Building M: Renovation and Additions

The project includes alterations to an existing building. All exterior work shall seek to match existing where possible. New work shall meet or exceed the quality of existing work to remain.

Contractor shall match existing eaves, trims and other wood details and assemblies UON.

Contractor shall construct new openings to match existing. Consideration shall include exposure of metal opening frames, dimensions and configuration of sealant joints, dimension and configuration of flashings, sills, thresholds and operators.

Existing Cement Plaster finish shall be matched at all new plaster locations.

Where new plaster meets existing plaster:

- New paper/membrane shall lap under or over old paper, depending on relationship of old to new, minimum of 2".
- New wire mesh shall be tied to old mesh and lapped minimum of 2".
- At corners where existing plaster occurs at one side only, existing plaster shall be removed at least 6" from corner and new plaster, mesh and paper shall wrap around corner.
- Provide double layer building paper/membrane at all openings, corners, railing caps and roof to wall conditions.
- Install exterior plaster prior to installation of site paving at building perimeter.

Existing Metal Siding

Existing metal siding shall be protected from damage. New mtl siding details shall match existing at corners, openings and roofs UON.

Building B: New Construction

Maintenance Notes

Exterior finishes shall be cleaned and/or replaced regularly by Owner. Paint shall not be allowed to crack, peel, or otherwise fail in any way without immediate replacement.

Windows shall be cleaned and maintained by Owner per mfr.'s recommendations.

Insect debris (spider webs, etc.) shall be removed regularly.

Landscape irrigation shall not be allowed to spray on any portion of structure.

Downspouts and splash blocks shall be adjusted so that no water splashes onto structure.

Slope all finish grades away from structure at 2% for min. 18" to swale

Keynotes

Mark	Description
1	Align as indicated
1.05	Line of Finish Grade
1.06	Line of floor beyond
3	Existing conc slab, foundation, ref struct dwgs
3.1	New conc slab, foundation, ref struct dwgs
3.11	Structural slab on grade
3.12	Concrete retaining wall, plywd formed
3.13	Conc retaining wall, plank formed
3.14	Conc. Curb at exterior walls
4.0	Existing CMU wall
4.01	New opening in existing CMU wall
4.1	New CMU wall
5.0	Existing mtl stair, landing, railing.
5.1	Structural steel framing, ref struct dwgs
5.3	Mtl stair, landing
5.4	Metal Railing
5.5	Metal Fence, Gate
6.3	Exterior wood trim, fascia
6.31	Decorative wood railing
7.01	Existing Mtl roof
7.03	Existing flashing, cover
7.2	New Metal Roof
7.21	Sheet Metal flashing, cricket
7.22	Metal coping, cover panel, edge flashing
7.23	Metal gutter, downspout
7.24	Metal Fascia, Painted
7.34	Roof drain/overflow
7.4	Flashing
7.41	Mtl parapet coping
7.42	Sheet metal sill
8.0	Existing door / window / skylight to remain
8.1	Aluminum storefront system
8.11	Operable window
8.12	Aluminum trim by storefront provider
8.21	Operable aluminum window
8.31	Aluminum Door in Storefront
8.32	Mtl Door & Frame
8.33	Wood Door in Storefront Frame
8.34	Wood Door in mtl. Frame
8.4	Glazed Door
8.41	Sectional Overhead Door
8.42	Coiled Overhead Door
9.0	Existing finish to remain
9.01	Existing mtl siding
9.02	Existing ext cem plaster, trims, screeds
9.03	Existing CMU
9.04	Existing exposed concrete
9.1	Exterior Cement Plaster
9.11	Plaster control joint, trim, screed
9.2	Corrugated Metal Siding
9.21	Mtl trim, match siding
15.60	Hard Pipe Downspout, by Plumbing
16.1	Elec equip, ref elec dwgs
16.12	Communication Devices, refer Elec dwgs
16.52	Wall Mounted Light Fixture, refer elec dwgs
16.54	Surface Mounted Light Fixture, refer elec dwgs

Northern Sonoma County Fire Protection District

Station 1 Modernization

20975 Geyserville Ave, Geyserville, CA 95441

Set Purpose

Planning Submittal

Minor Use Permit Mod

Design Review

Edition History

Schematic Design Set	1/9/2026
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Sheet Subject

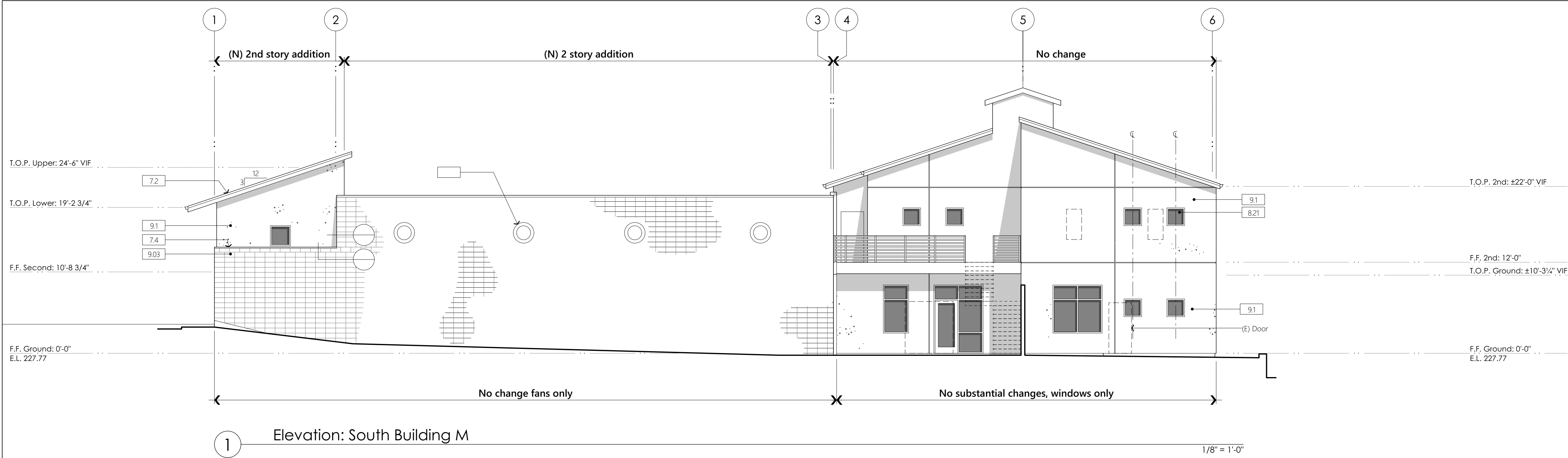
Building M Elevations

Program / Owner Proj #

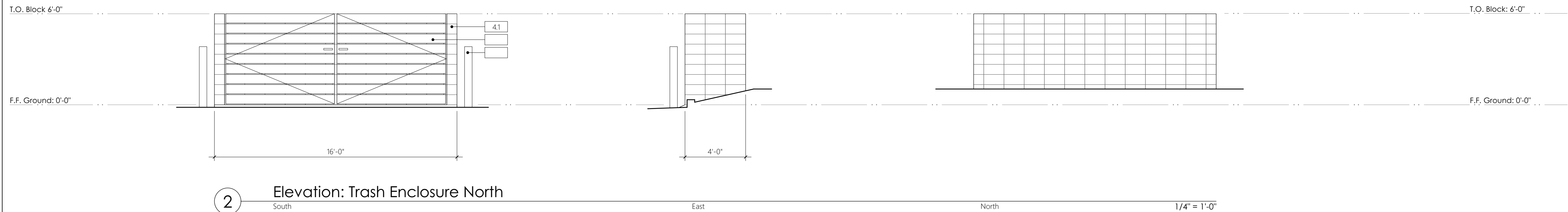
DTA Project #

2525

A4.1



Elevation: South Building M



Elevation: Trash Enclosure North

Elevation Notes

Ref A4.1 for elevation notes

Keynotes

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1	Align as indicated
1.05	Line of Finish Grade
1.06	Line of floor beyond
3	Existing conc slab, foundation, ref struct dwgs
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5.5	Metal Fence, Gate
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7.41	Mtl parapet coping
7.42	Sheet metal sill
8.0	Existing door / window / skylight, to remain
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8.11	Operable window
8.12	Aluminum trim by storefront provider
8.21	Operable aluminum window
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8.32	Mtl Door & Frame
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8.34	Glazed Door
8.41	Sectional Overhead Door
8.42	Coiled Overhead Door
9.0	Existing finish to remain
9.01	Existing mtl siding
9.02	Existing ext cem plaster, trims, screeds
9.03	Existing CMU
9.04	Existing exposed concrete
9.1	Exterior Cement Plaster
9.11	Plaster control joint, trim, screed
9.2	Corrugated Metal Siding
9.21	Mtl trim, match siding
15.60	Hard Pipe Downspout, by Plumbing
16.1	Elec. equip, ref elec dwgs
16.12	Communication Devices, refer Elec dwgs
16.52	Wall Mounted Light Fixture, refer elec dwgs
16.54	Surface Mounted Light Fixture, refer elec dwgs

DTA Dreiling Terrones Architecture Inc.

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Environments

1103 Juanita Avenue
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95448
707.431.1305

Northern Sonoma County Fire Protection District

Station 1 Modernization

20975 Geyserville Ave, Geyserville, CA 95441

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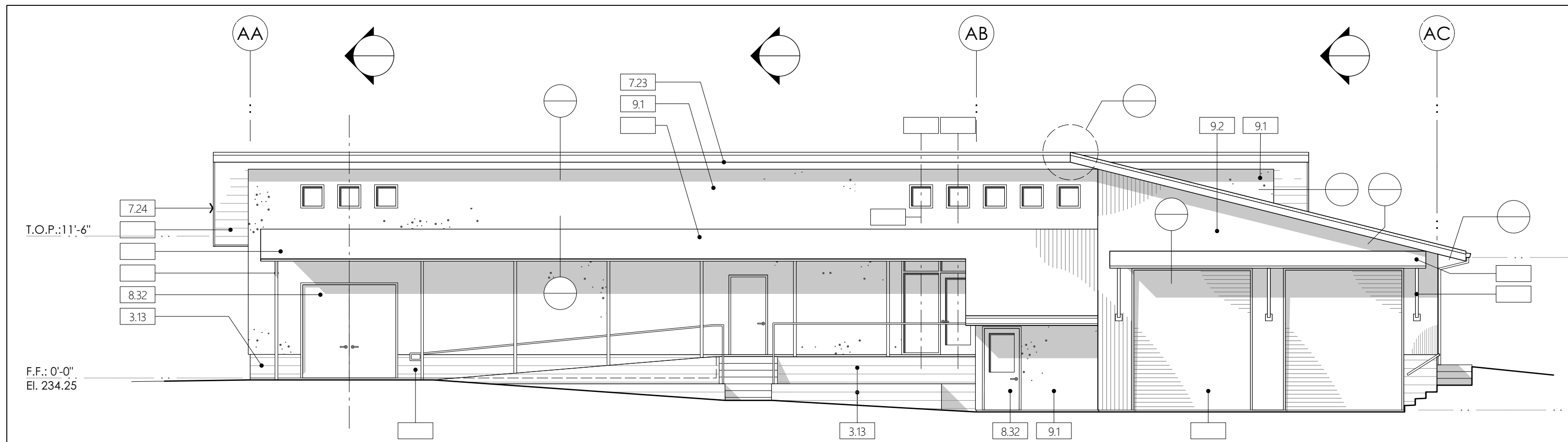
Program / Owner Proj #

DTA Project #

2525

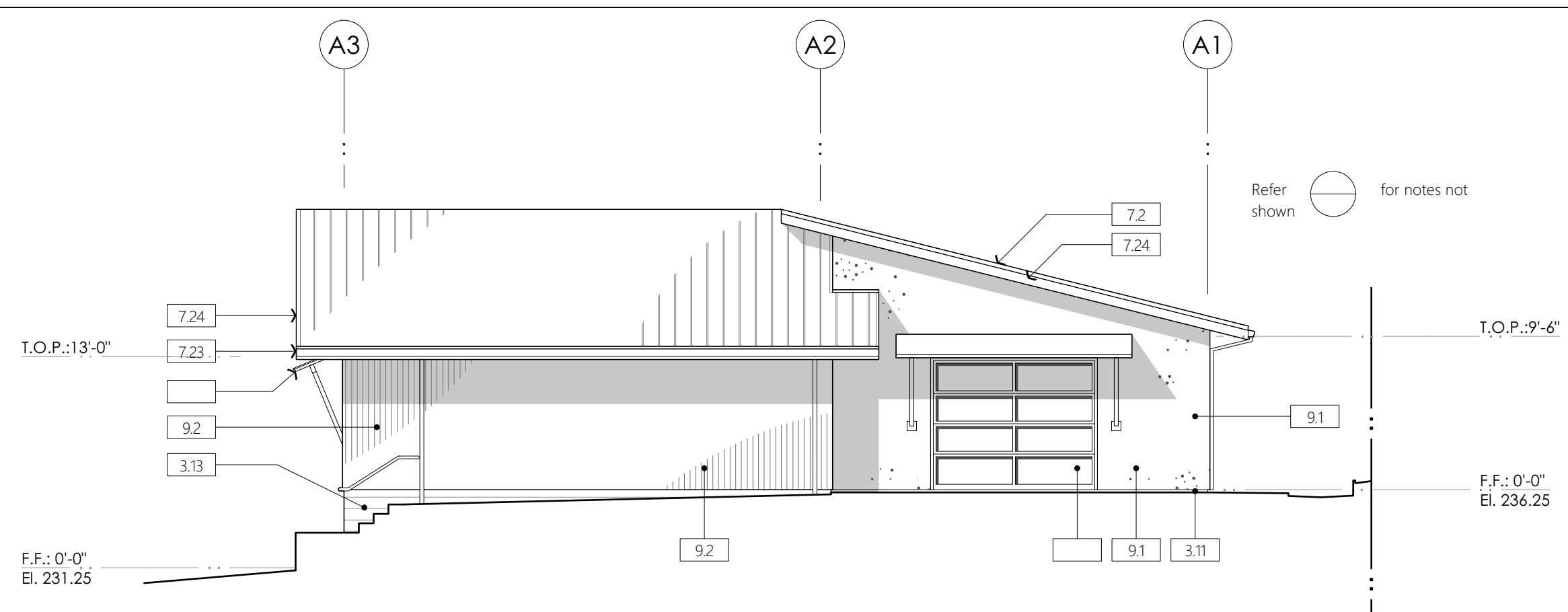
Building M
Elevations

A4.2



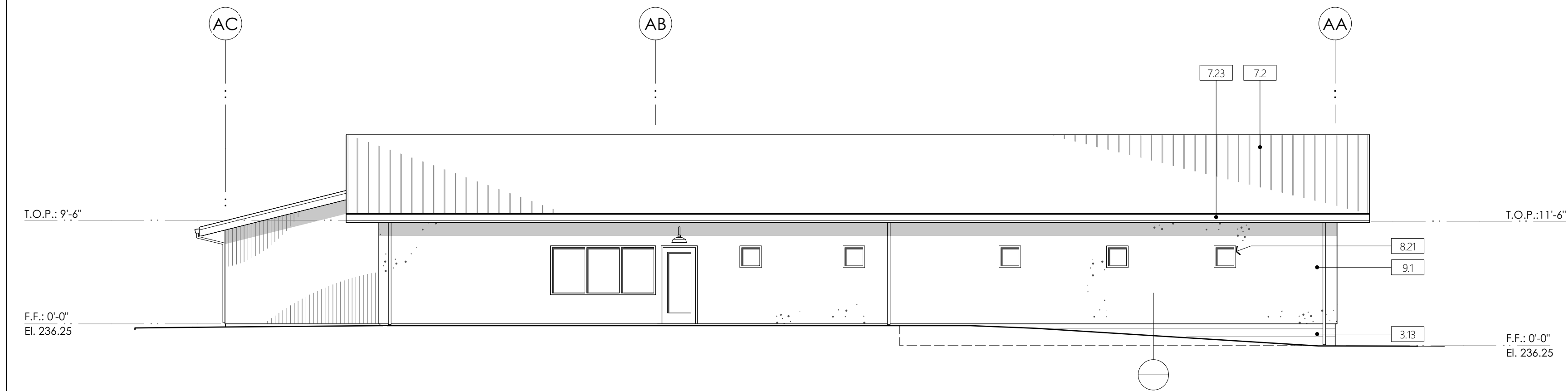
1 Elevation: Building B East

1/8" = 1'-0"



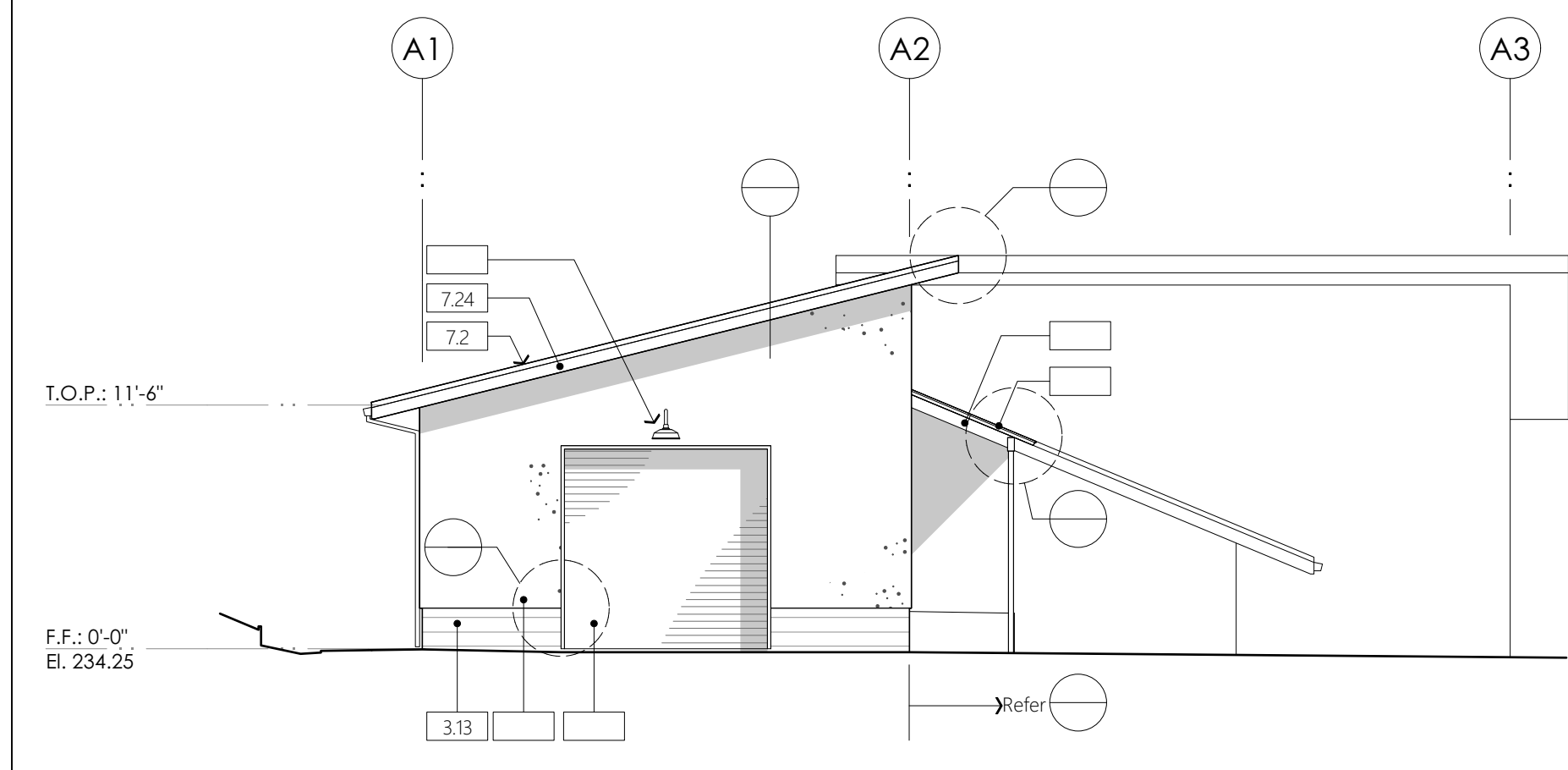
2 Elevation: Building B North

1/8" = 1'-0"



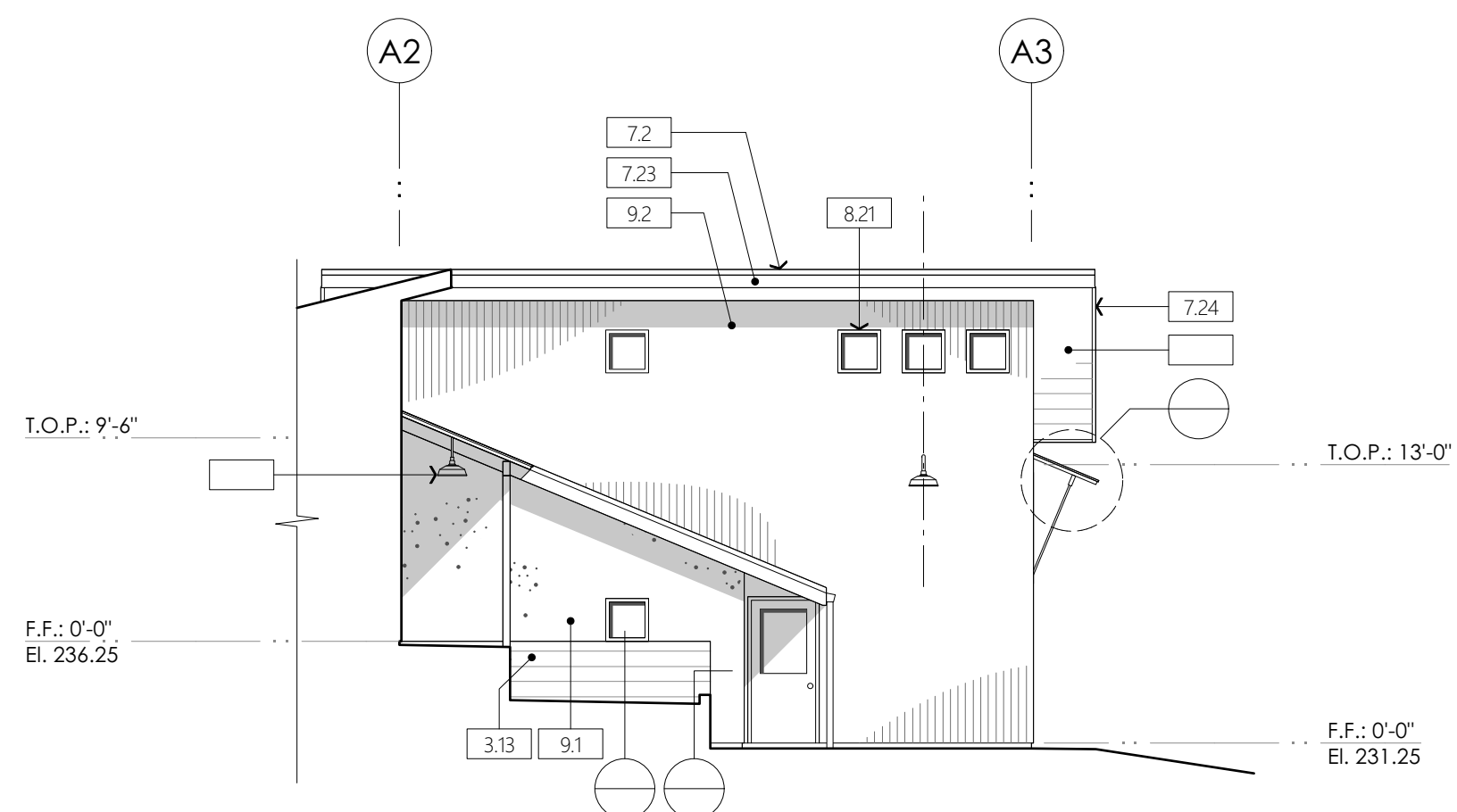
3 Elevation: Building B West

1/8" = 1'-0"



4 Elevation: Building B South

1/8" = 1'-0"



5 Part Elevation: Building B South

1/8" = 1'-0"

Elevation Notes

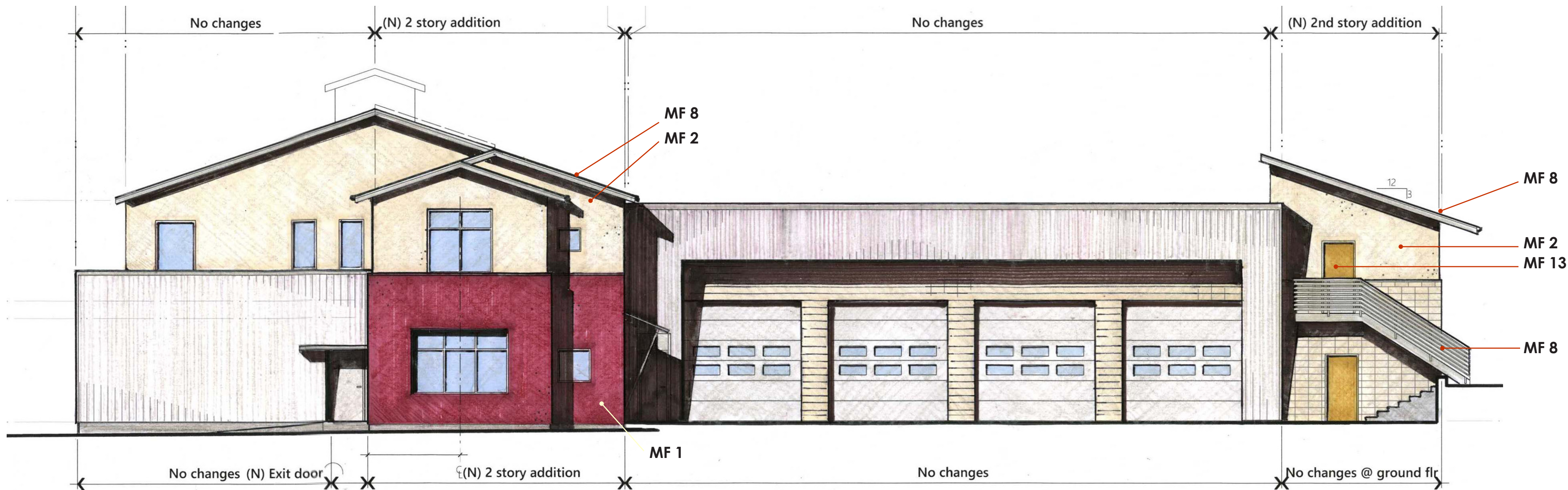
Ref A4.1 for elevation notes

Keynotes

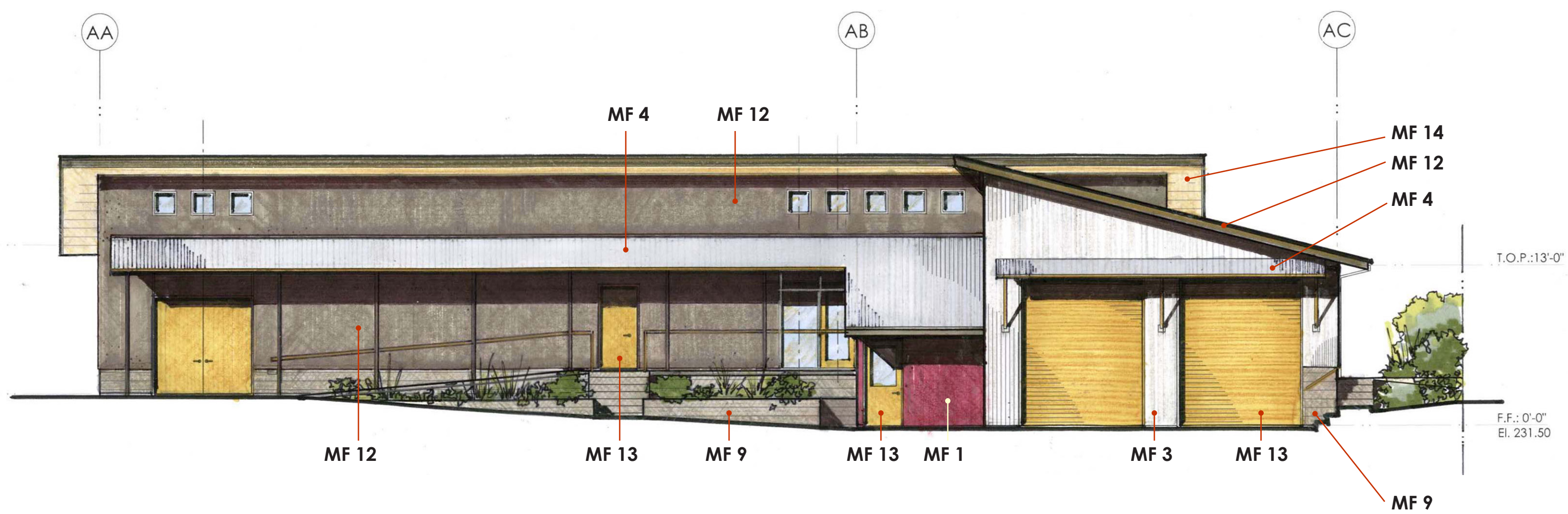
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16.54	Surface Mounted Light Fixture, refer elec dwgs



Elevation: Bldg M: East



Elevation: Bldg M: East



Elevation: Bldg B: East

SW 7046

Anonymous

FULL DETAILS

Gray-green undertones stir in the depths of this warm, hazy neutral. Pair with an off-white shade like Muslin for an earthy effect.

SW 9020

Rayo de Sol

FULL DETAILS

Sample this color:

SW 9015

They call it Mellow

FULL DETAILS

Recharge your batteries with this bright yellow that brings a soft, strong energy to any room. Often works well in bathrooms and kitchens.

- MF 12**

Ext Plaster, Wood Fascia:
Bldg B
- MF 13**

New and existing service Doors:
Yellow Ochre sim. to MF 11
- MF 14**

Soffits at Bldg B
- MF 10**

Wood Railing: Redwood,
stained Rosewood
- MF 11**

New Entry Door:
VG Doug Fir, Clear coat



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Healdsburg, California
95448
707.431.1305

Northern Sonoma County Fire Protection District

Station 1 Modernization

20975 Geyserville Avenue, Geyserville, CA 95441

APN: 140-100-070

Sel Purpose

Planning Submittal:
Minor Use Permit Mod
Design Review

Edition History

Zoning Submittal 4/21/26

Sheet Subject

Design Review
Colors - Materials

Program / Owner Proj #

NCF 1.001

DTA Project #

dtg 2525

A4.4

LEGEND

	UTILITY POLE W/ GUY WIRE
	GAS METER
	COMMUNICATION BOX
	MAIL BOX
	TRANSFORMER VAULT
	SEWER MANHOLE
	STORM DRAIN MANHOLE
	DI, DROP INLET
	FIRE HYDRANT
	WATER VALVE
	WATER BLOWOFF VALVE
	WELL
	PERC HOLE W/DEPTH & MINUTES PER INCH
	GROUNDWATER HOLE W/DEPTH & DATED TESTED
	SOILS PROFILE PIT W/DEPTH
	ELECTRIC METER
	WATER METER
	SEWER CLEANOUT
	EXISTING TREE
	TREE TO BE REMOVED
	DOWNSPOUT WITH SPLASHBLOCK
	BOUNDARY LINE
	ADJOINING BOUNDARY LINE
	EASEMENT(APPROX)
	CENTERLINE
	CONTOUR MAJOR
	CONTOUR MINOR
	EDGE OF LANDSCAPING
	FENCE
	FLOWLINE
	EXISTING GAS LINE
	EXISTING OVERHEAD WIRE
	EXISTING SEWER LINE
	EXISTING WATER LINE
	EXISTING DRY UTILITIES
	PROPOSED GAS LINE
	PROPOSED OVERHEAD WIRE
	PROPOSED SEWER LINE
	PROPOSED WATER LINE
	PROPOSED DRY UTILITIES
	TREE DRIP LINE
	CULVERT/STORMDRAIN W/SIZE & TYPE
	EXISTING GRAVEL
	EXISTING ASPHALT
	EXISTING CONCRETE
	EXISTING DECK
	EXISTING BUILDING
	TOP OF BANK
	LEACHFIELD AREA(APPROX)
	PROPOSED GRAVEL SURFACE
	PROPOSED AC PAVEMENT
	ROCKWALL
	DECKING
S.A.D.	SEE ARCHITECTURAL DWGS.
S.S.D.	SEE STRUCTURAL DWGS.
EG	EXISTING GRADE
FG	FINISH GRADE
TG	TOP GRATE
TW	TOP RETAINING WALL

GRD26-0000

Station 1 Modernization

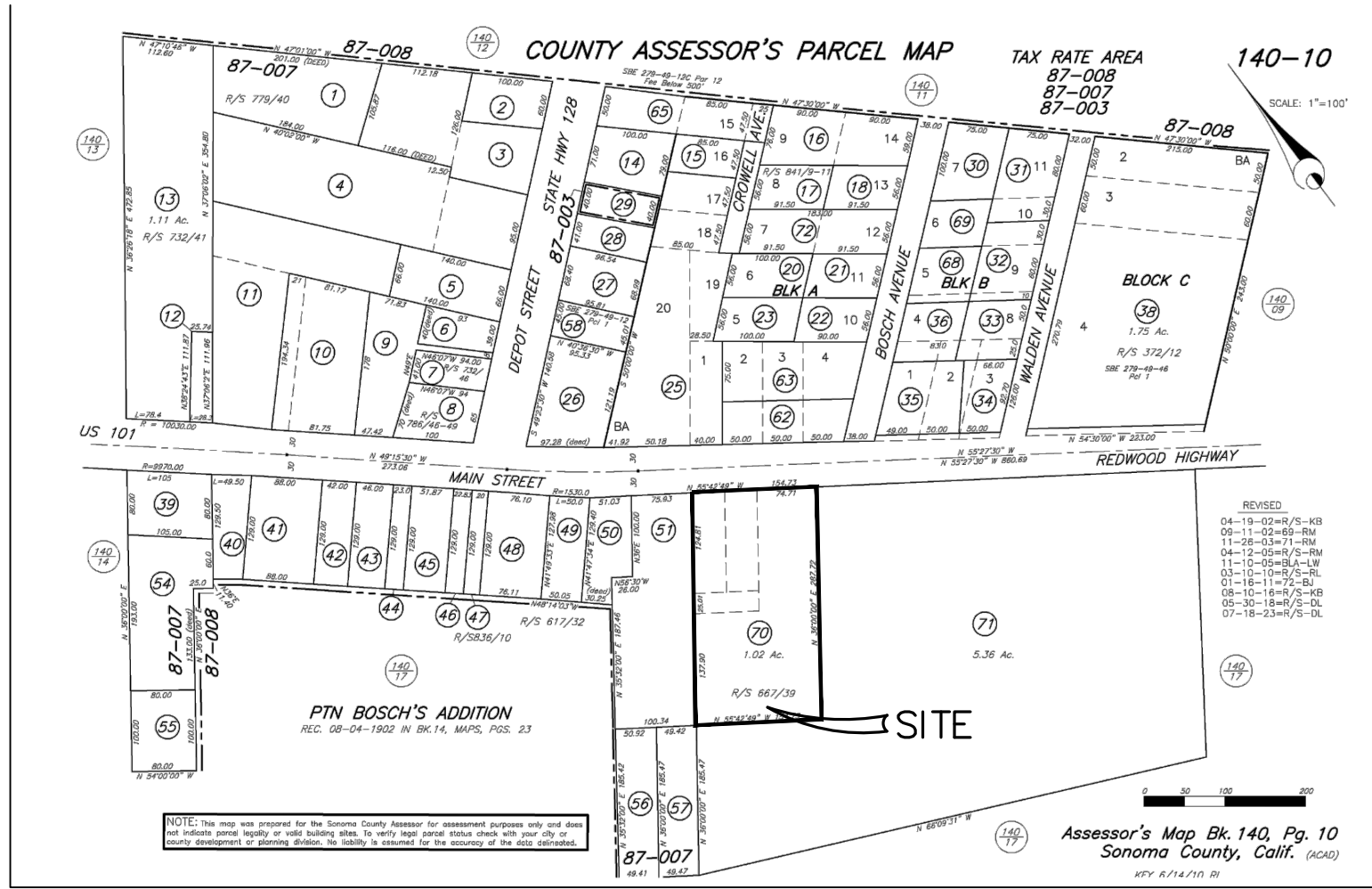
Northern Sonoma County
Fire Protection District
20975 Geyserville Avenue
Geyserville, CA 95441

ASSESSOR'S PARCEL NUMBER 140-100-070

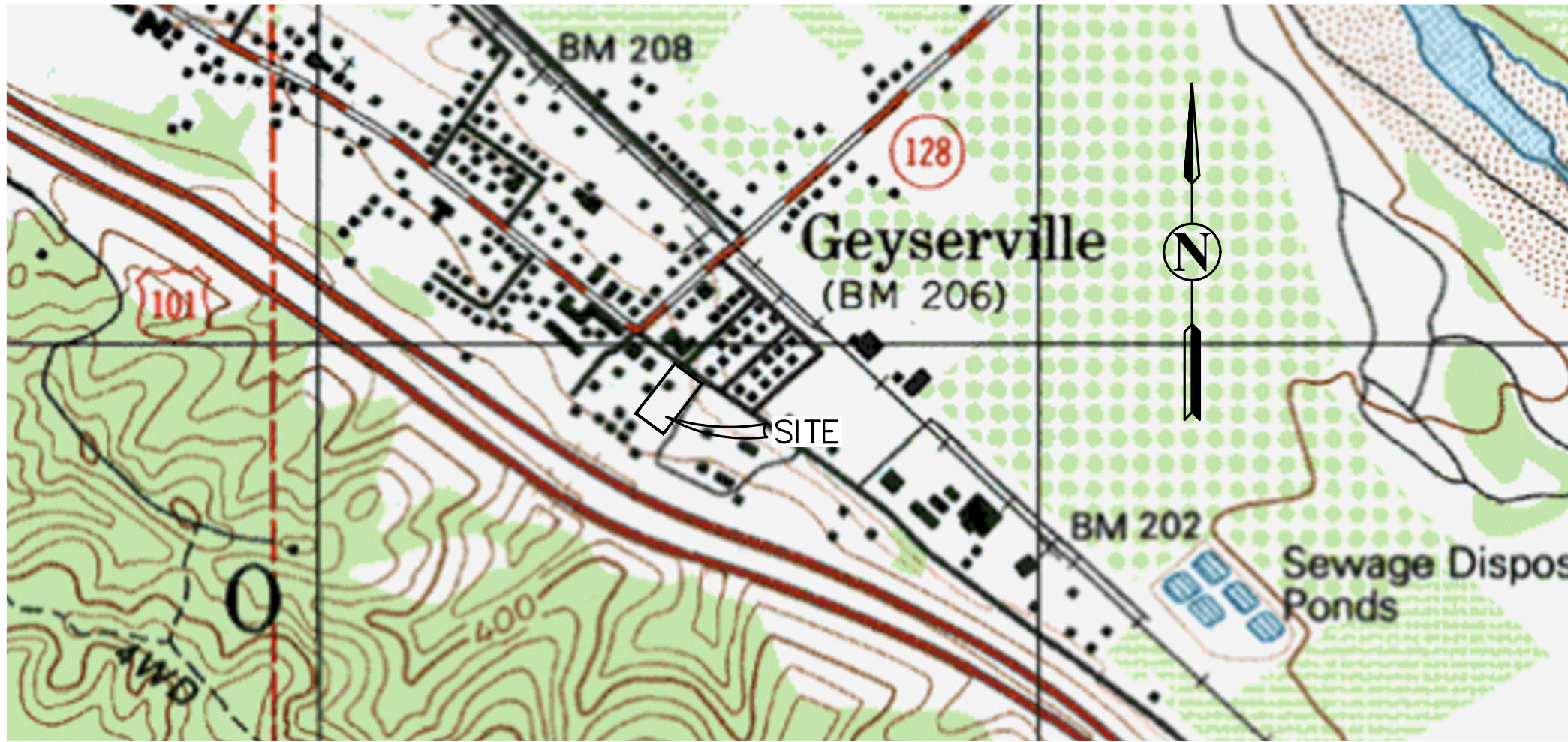
PROJECT SCOPE: Construct a new 2-bay apparatus garage with fire personnel accommodations, and main firehouse addition.

Sheet Index

C1	TITLE SHEET
C2	SITE PLAN
C3	GRADING PLANS
C4	UTILITY PLANS
C5	DRIVEWAY PROFILE AND SECTIONS
C6	UTILITY DETAILS
C7	GRADING DETAILS
C8	CONSTRUCTION NOTES



ASSESSOR'S PARCEL MAP
NTS



ASTI QUAD MAP
NTS

Project Information

Project Site Address:	20975 Geyserville Avenue
Owner:	Northern Sonoma County Fire Protection District 20975 Geyserville Avenue Geyserville, CA 95441
Architect:	Martin Dreiling, aia Dreiling Terrones Architecture Inc. 314 Center Street, #220 Healdsburg, CA 95448
Civil Engineer:	TOM ATTERBURYI, P.E. ATTERBURY & ASSOCIATES, INC. 16190 HEALDSBURG AVE, SUITE D HEALDSBURG, CA 95448 (707) 433-0134 ta@atterburyandassociates.com
Surveyor:	Mathew Dudley, L.S. Cinquini & Passarino, Inc. 117 West North Street Healdsburg, CA 95448 (707) 433-9760 mdudley@cinquiniepassarino.com
Acres:	1.02 ACRES

EARTHWORK

TOTAL CUT = 269 CY
TOTAL FILL = 126 CY

APPROXIMATE VOLUME OF FILL TO BE PLACED WITHIN THE EXCAVATION OF THE 8,000 GALLON WATER TANK IS 40 CY.

NET TOTAL = 103 CY TOTAL (CUT).
(TO BE OFF-HAULED TO AN APPROVED AND PERMITTED SITE.)

DISTURBED AREA = 0.25 ACRES

CREATED OR REPLACED IMPERVIOUS SURFACE = 7,185 SF
PROPOSED PERVIOUS PAVERS SURFACE AREA = 777 SF
(AREAS SHOWN REPRESENT EXPOSED PLAN-VIEW SURFACES ONLY. NO OVERLAP EXISTS BETWEEN THE TWO CALCULATIONS.)

SURFACE GRADING ONLY. CONTRACTOR RESPONSIBLE FOR THEIR OWN EARTHWORK ESTIMATE.

CONTRACTOR SHALL INCLUDE ALL REQUIRED EARTHWORK HAULING ON OR OFFSITE IN BID.

DTA
Dreiling Terrones Architecture Inc.

Atterbury & Associates, Inc.

Civil Engineering – Land Planning
16109 Healdsburg Avenue, Suite D, Healdsburg, CA 95448
Phone: (707) 433-0134. Email: ta@atterburyandassociates.com

Northern Sonoma County Fire Protection District
Station 1 Modernization
20975 Geyserville Ave. Geyserville, CA 95441

Set Purpose

Edisons History	
Schematic Design Set	1/9/2026
50% Progress Set	3/19/2026

Sheet Subject

TITLE SHEET

C1

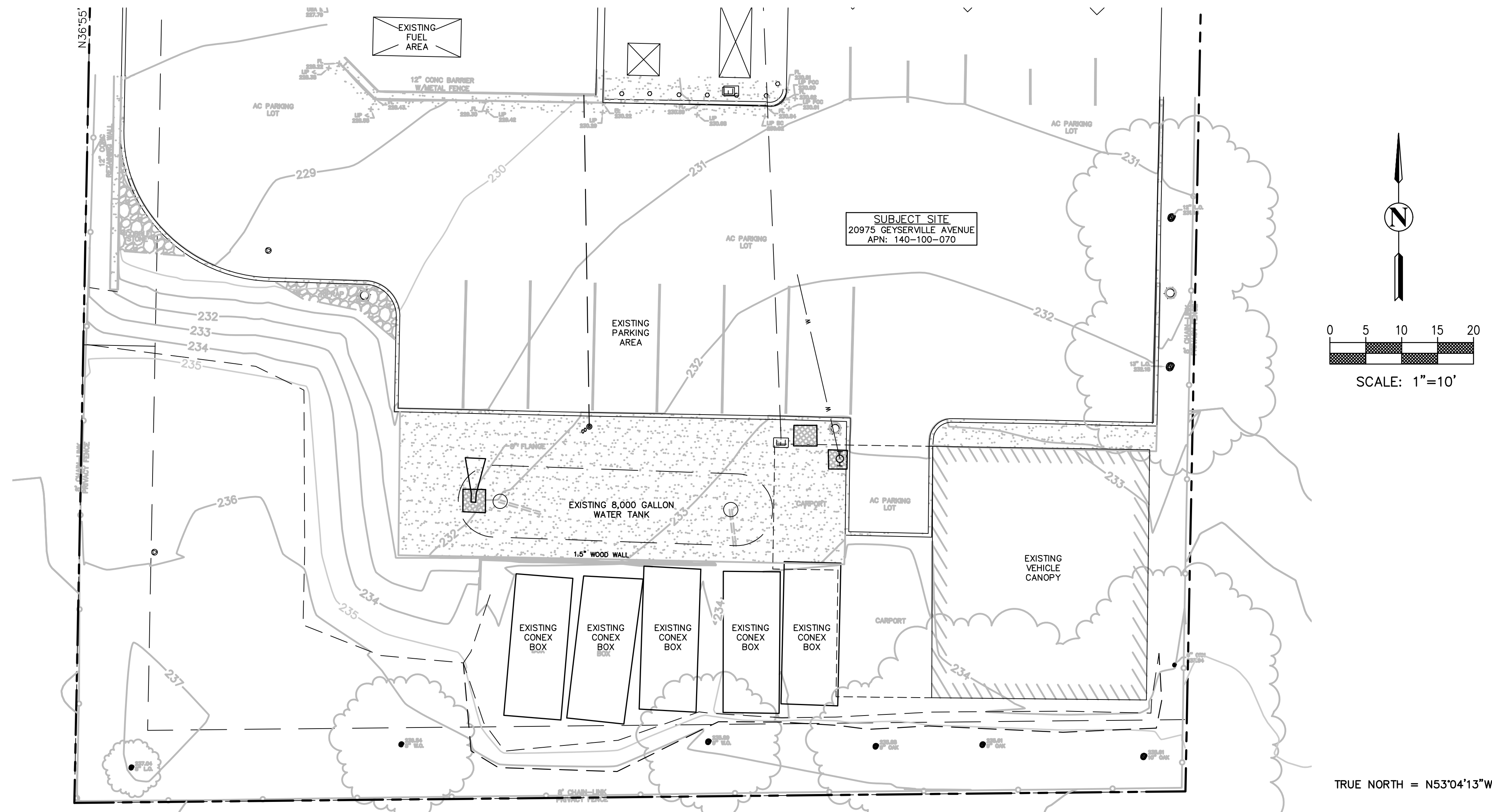
Program / Owner Proj #

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2525

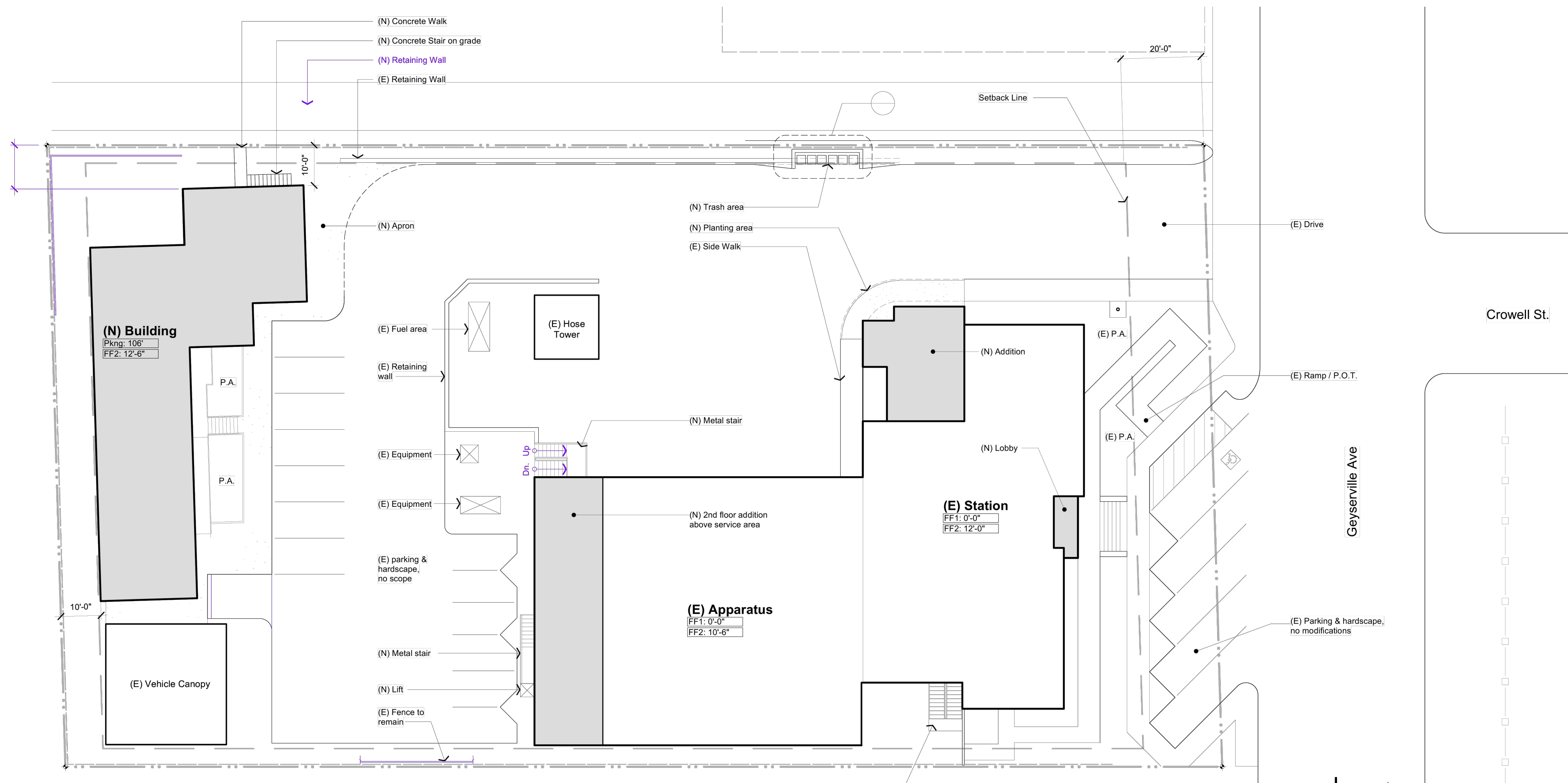
PRELIMINARY PLANSET
(NOT FOR CONSTRUCTION)
04/20/2026

AX.X



TOPOGRAPHIC MAP BY CINQUINI & PASSARINO

AS NOTED



OVERALL SITE PLAN

AS NOTED

PRELIMINARY PLANSET
(NOT FOR CONSTRUCTION)
04/20/2026

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Sheet Subject

SITE PLAN

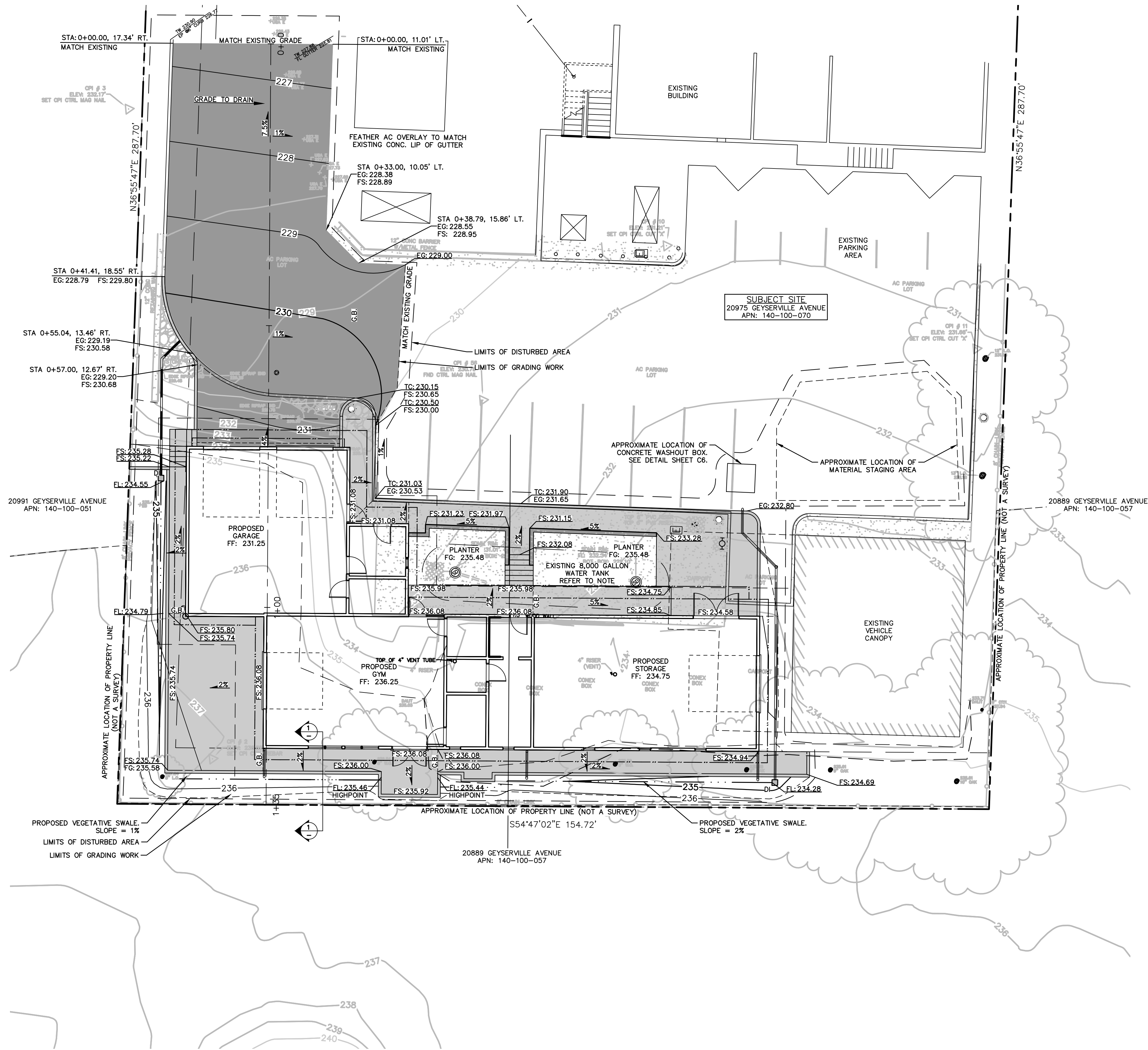
C2

Program / Owner Proj #

DTA Project #

2525

AX.X



PROPERTY LINE

10' SETBACK

2' MIN. SWALE

2' LANDSCAPE STRIP

PERMEABLE PAVERS

GYM

EX.GND.

6"

2"

2%

FF 236.25

PAD SUBGRADE

EX.GND.

PROPOSED CONCRETE CURB

4" OF CONSOLIDATED CRUSHED ROCK

PAD ~ FIRM UNDISTURBED FILL AT 90% R.C.
GEOTECHNICAL ENGINEER TO VERIFY SUBGRADE PRIOR TO FURTHER CONSTRUCTION

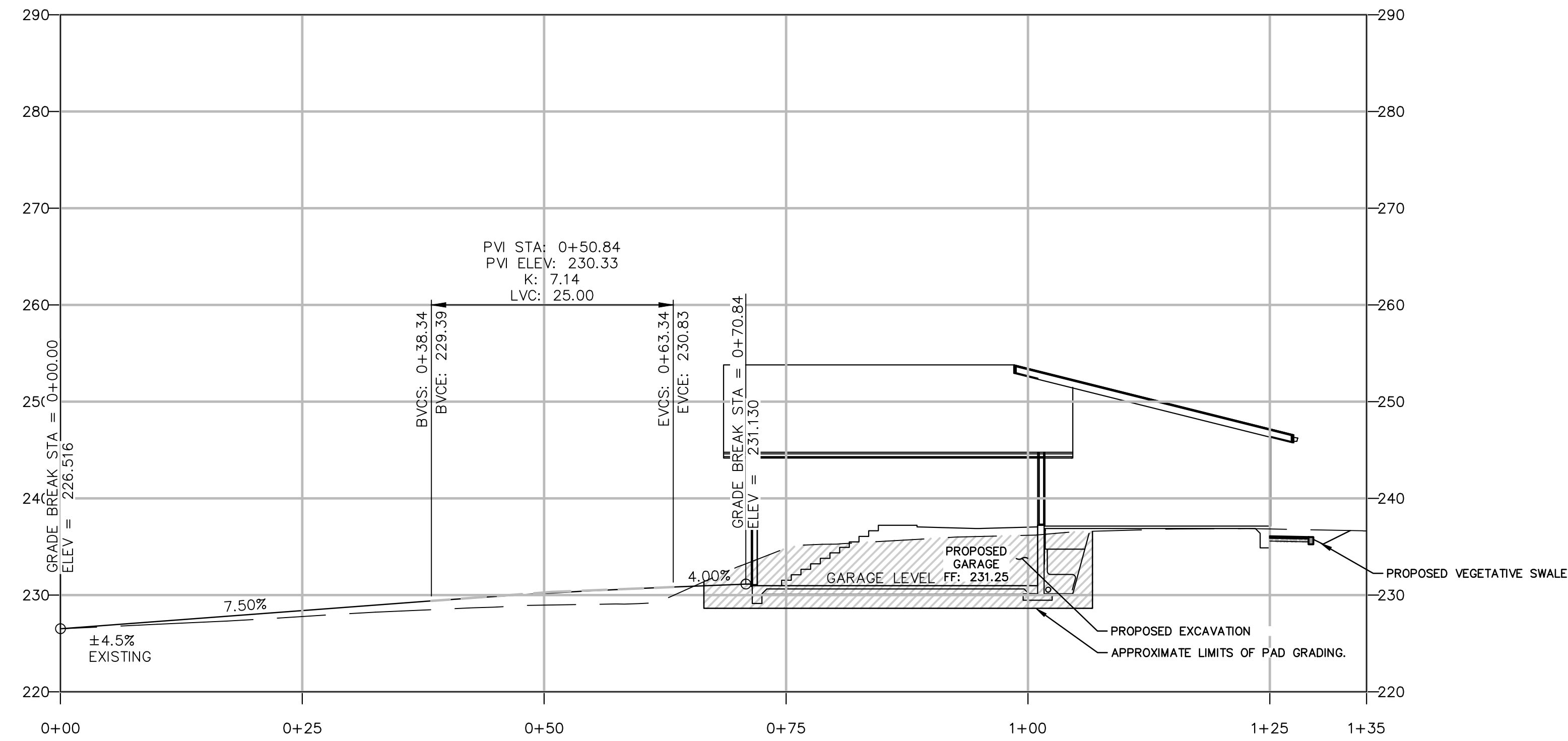
REFER TO DETAIL SHT. C6

SECTION AT GYM

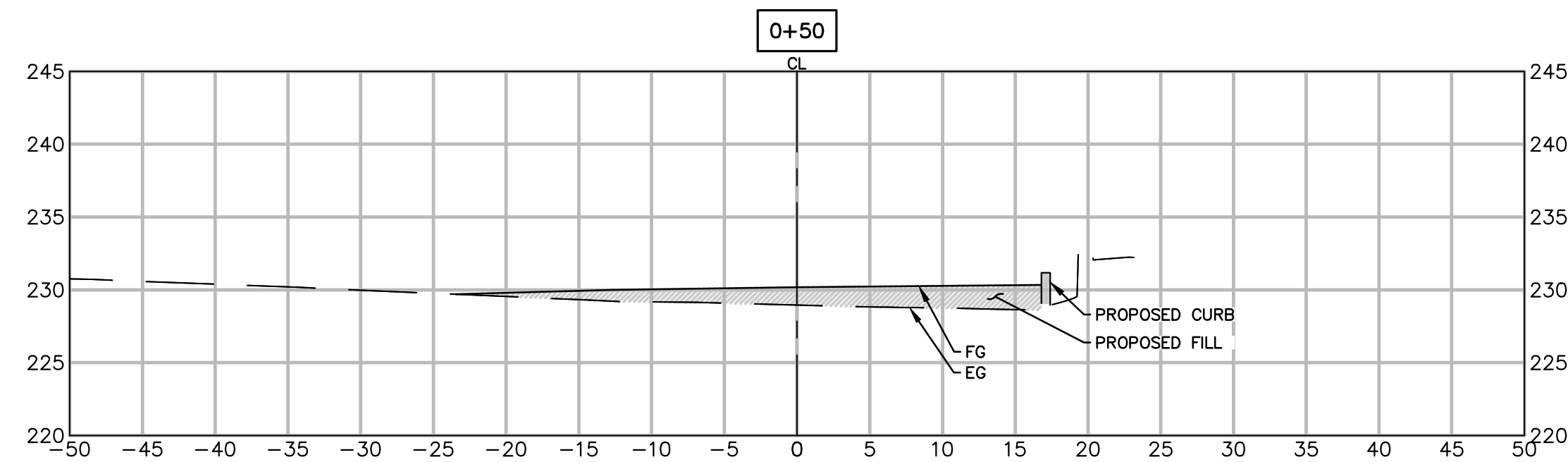
NTS

LIDAR DATA AND ORTHOPHOTOGRAPHY WERE PROVIDED BY THE UNIVERSITY OF MARYLAND UNDER GRANT NNX13AP69G FROM NASA'S CARBON MONITORING SYSTEM (DR. RALPH DUBAYAH AND DR. GEORGE HURTT (PRINCIPAL INVESTIGATORS)). THIS GRANT ALSO FUNDED THE CREATION OF DERIVED FOREST COVER AND LAND COVER INFORMATION, INCLUDING A COUNTYWIDE BIOMASS AND CARBON MAP, A CANOPY COVER MAP, AND DEMS. THE SONOMA COUNTY VEGETATION MAPPING AND LIDAR PROGRAM FUNDED LIDAR DERIVED PRODUCTS IN THE CALIFORNIA STATE PLANE COORDINATE SYSTEM SUCH AS DEMS, HILLSHADES, BUILDING FOOTPRINTS, 1-FOOT CONTOURS, AND OTHER DERIVED LAYERS. THE ENTIRETY OF THIS DATA IS FREELY LICENSED FOR UNRESTRICTED PUBLIC USE, UNLESS OTHERWISE NOTED. ANY USE OF THIS DATA, INCLUDING VALUE-ADDED PRODUCTS, WITHIN REPORTS, PAPERS, AND PRESENTATIONS MUST ACKNOWLEDGE NASA GRANT NNX13AP69G, THE UNIVERSITY OF MARYLAND AND THE SONOMA VEGETATION MAPPING AND LIDAR PROGRAM AS THEIR SOURCES.

AX.X



DRIVEWAY PROFILE VIEW
SCALE: 1"=10' (H,V)



DRIVEWAY CROSS SECTIONS
SCALE: 1"=10' (H,V)

PRELIMINARY PLANSET
(NOT FOR CONSTRUCTION)
04/20/2026

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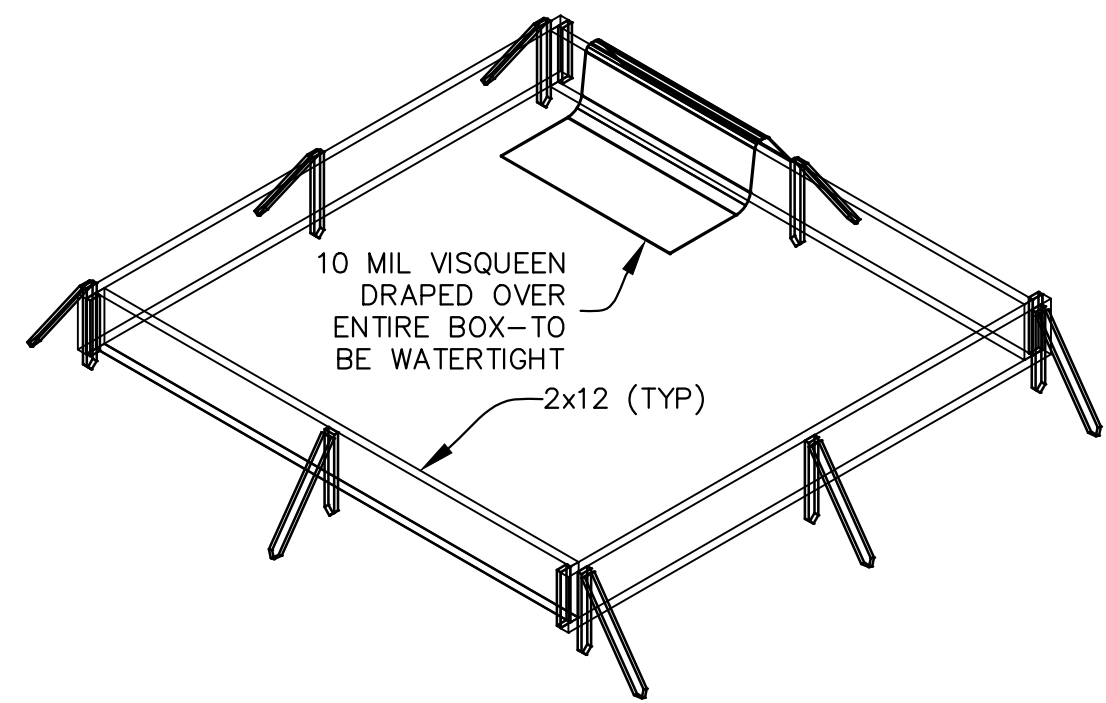
**DRIVEWAY PROFILE
AND SECTIONS
C5**

Program / Owner Proj #

DTA Project #

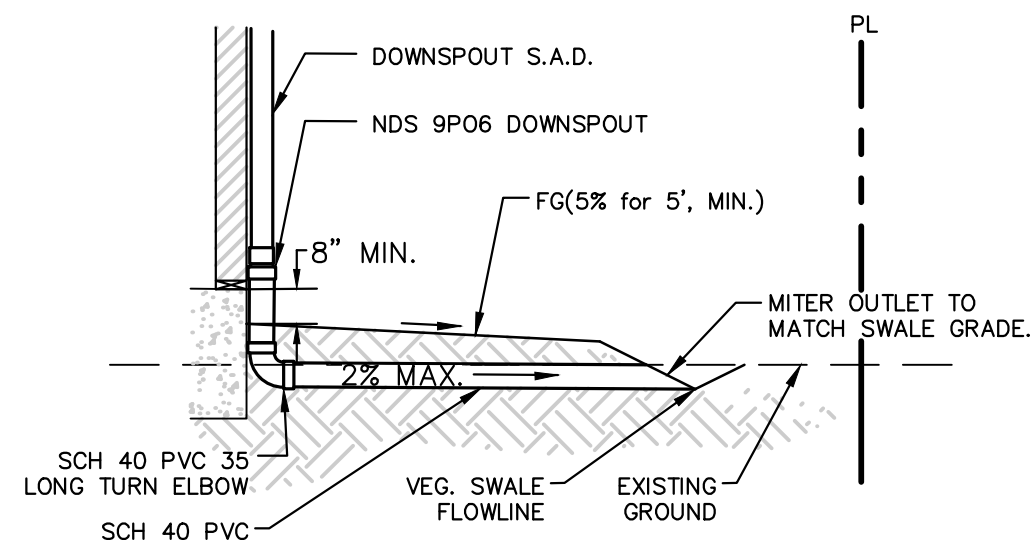
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AX.X

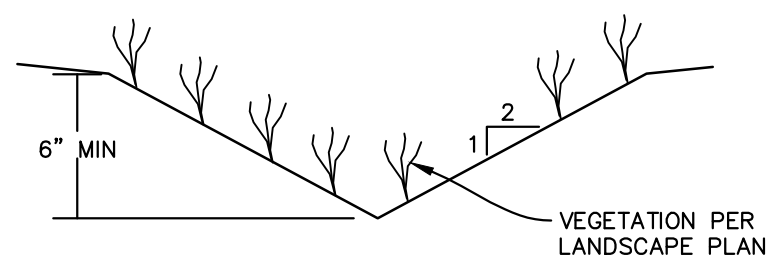


1. CONTRACTOR TO ADJUST SIZE AND STAKING TO JOB QUANTITY AND FREQUENCY OF POURS.
2. NO SHARP OBJECTS, NAIL ENDS, ROCKS, ETC ON INSIDE OF BOX
3. AT COMPLETION OF WORK, BOX AREAS SHALL BE RESTORED TO THE ORIGINAL CONDITION, OR TO THE SPECIFIED FINISH ON THE DWGS. BY THE CONTRACTOR.

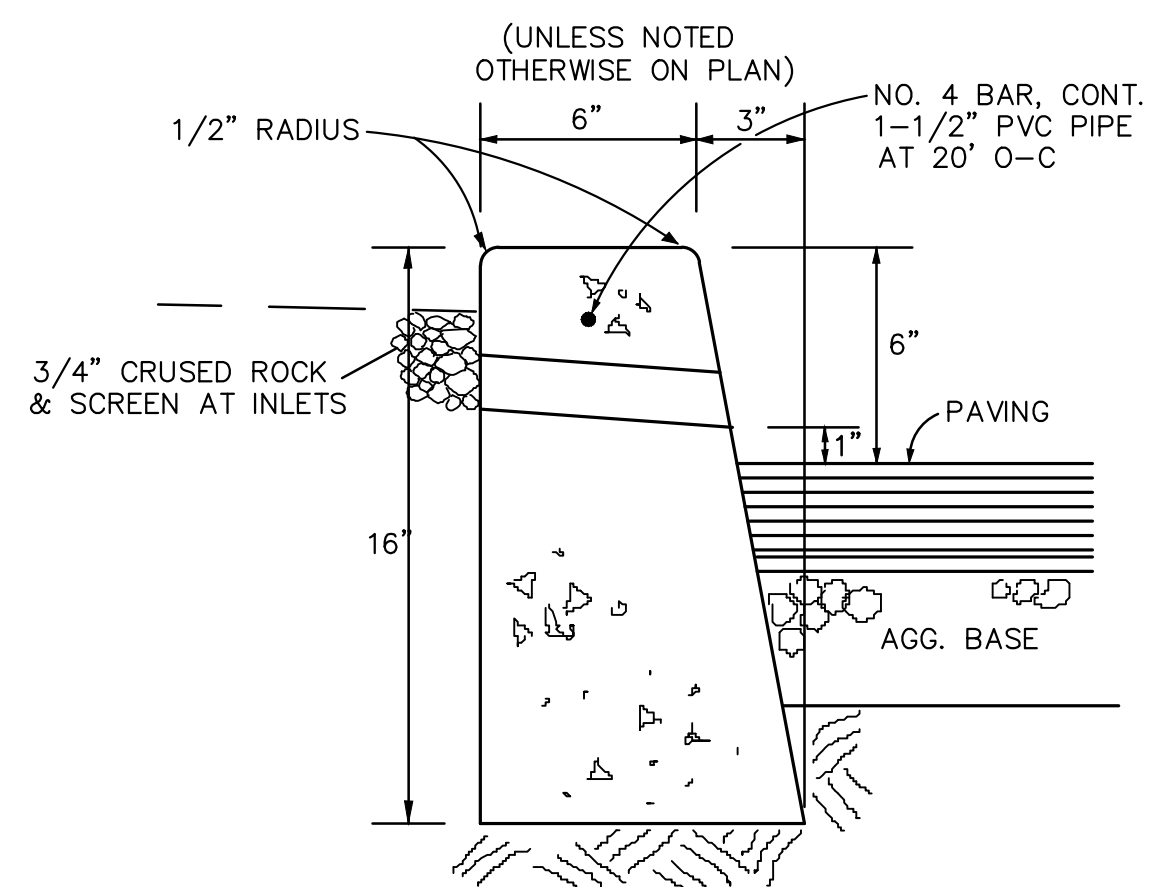
CONCRETE WASHOUT BOX
NTS



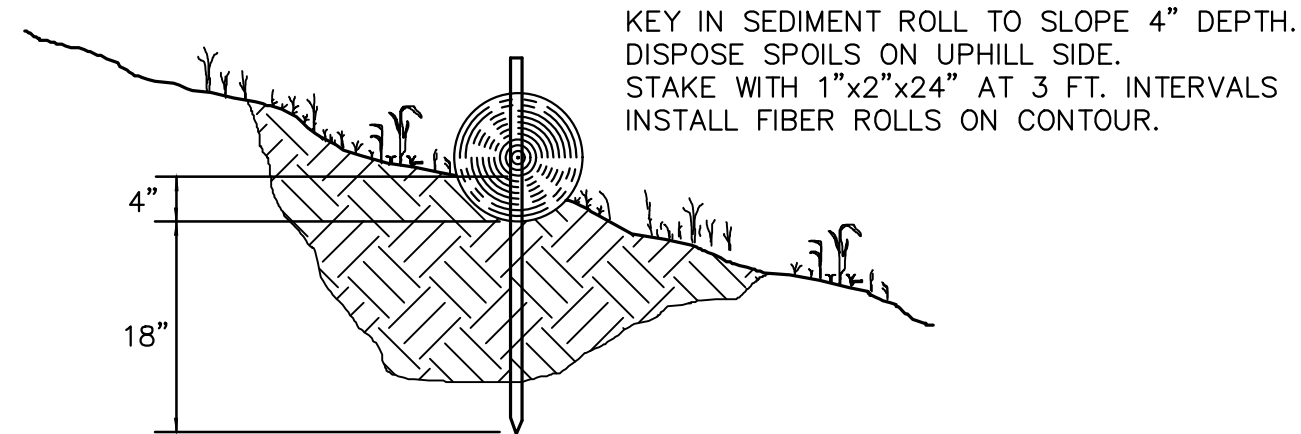
DOWNSPOUT DAYLIGHT
IN SIDEYARD SWALE DETAIL
NTS



VEGETATED SWALE
NTS

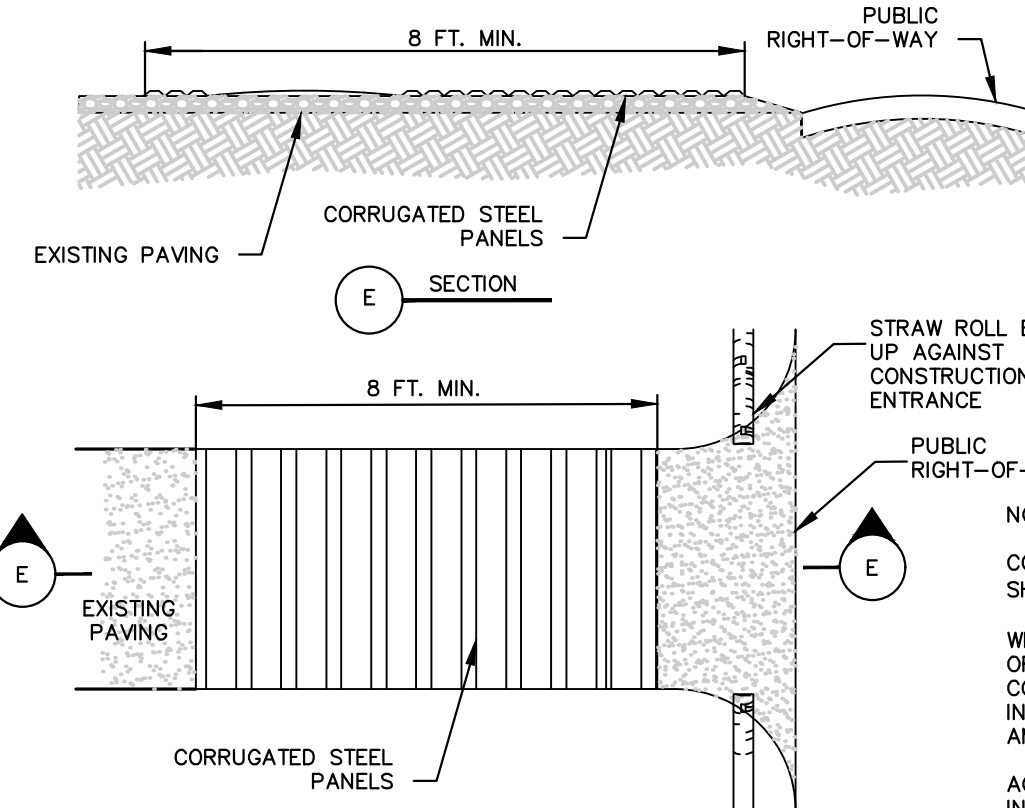
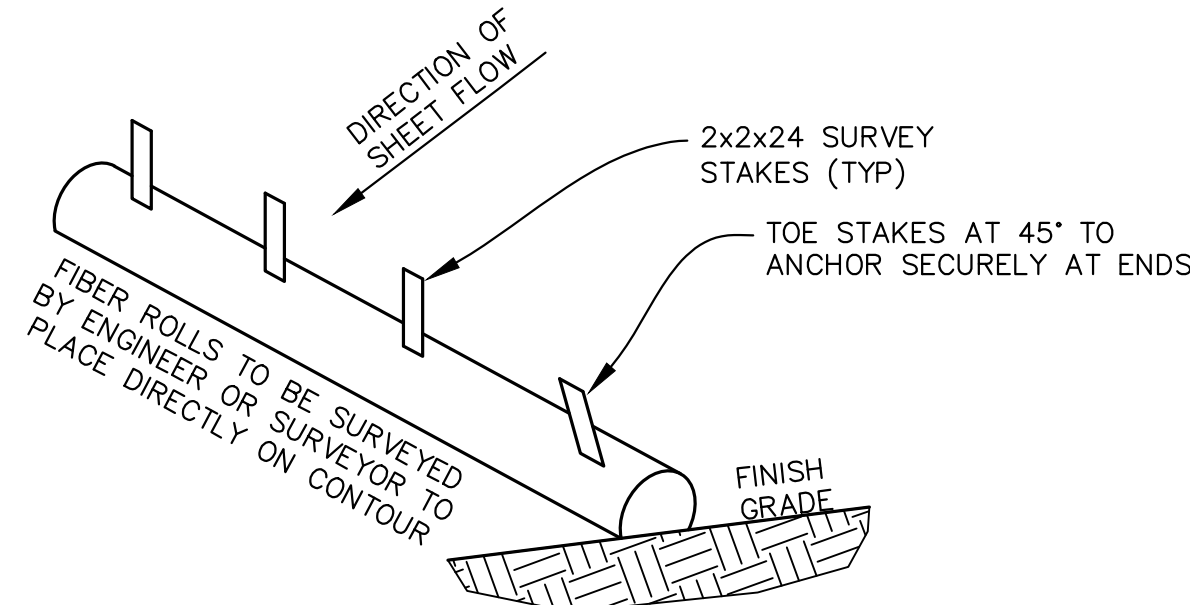


CONCRETE CURB
NTS



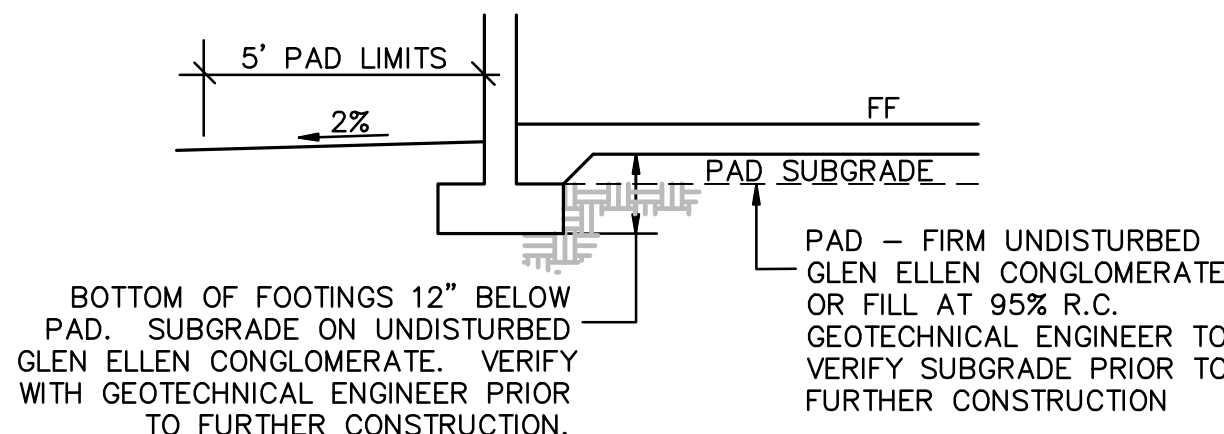
KEY IN SEDIMENT ROLL TO SLOPE 4" DEPTH.
DISPOSE SPOILS ON UPHILL SIDE.
STAKE WITH 1"x2"x24" AT 3 FT. INTERVALS
INSTALL FIBER ROLLS ON CONTOUR.

SEDIMENT ROLL INSTALLATION
NTS



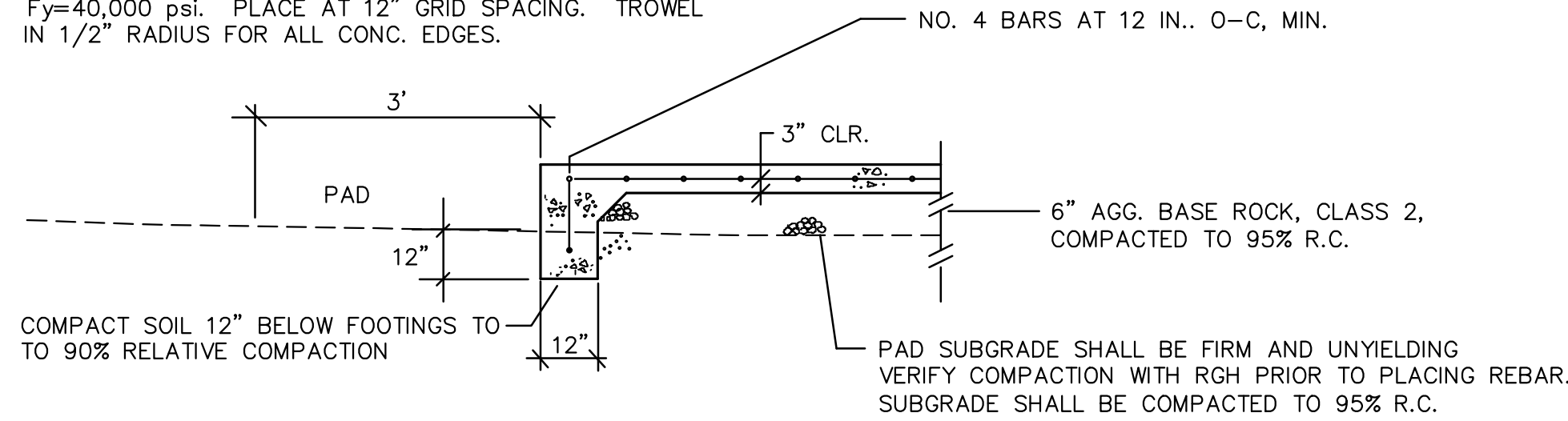
STEEL PLATED CONSTRUCTION ENTRANCE
NTS

NOTES:
CORRUGATED STEEL PANEL
SHALL BE A MINIMUM OF 8'.
WIDTH SHALL BE A MIN. OF 15'
OR GREATER IF NECESSARY TO
COVER ALL VEHICULAR
INGRESS AND EGRESS. PROVIDE
AMPLE TURNING RADII.
ACCESSES SHALL BE
INSPECTED WEEKLY DURING
PERIODS OF HEAVY USAGE,
MONTHLY DURING NORMAL
USAGE, AND AFTER EACH
RAINFALL, WITH MAINTENANCE
PROVIDED AS NECESSARY.



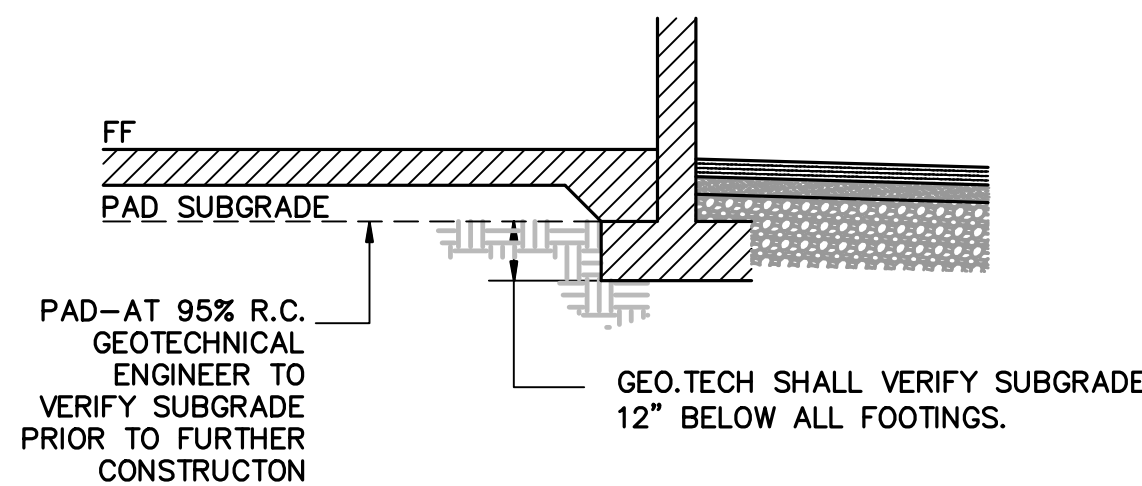
BUILDING SLAB/FOOTING
NTS

CONSTRUCT 6" CONCRETE SLAB W/ 3,000 psi. CONCRETE, FINISH
FINISH PER ARCH. SPECS. NO. 4 REINFORCEMENT BARS,
Fy=40,000 psi. PLACE AT 12" GRID SPACING. TROWEL
IN 1/2" RADIUS FOR ALL CONC. EDGES.

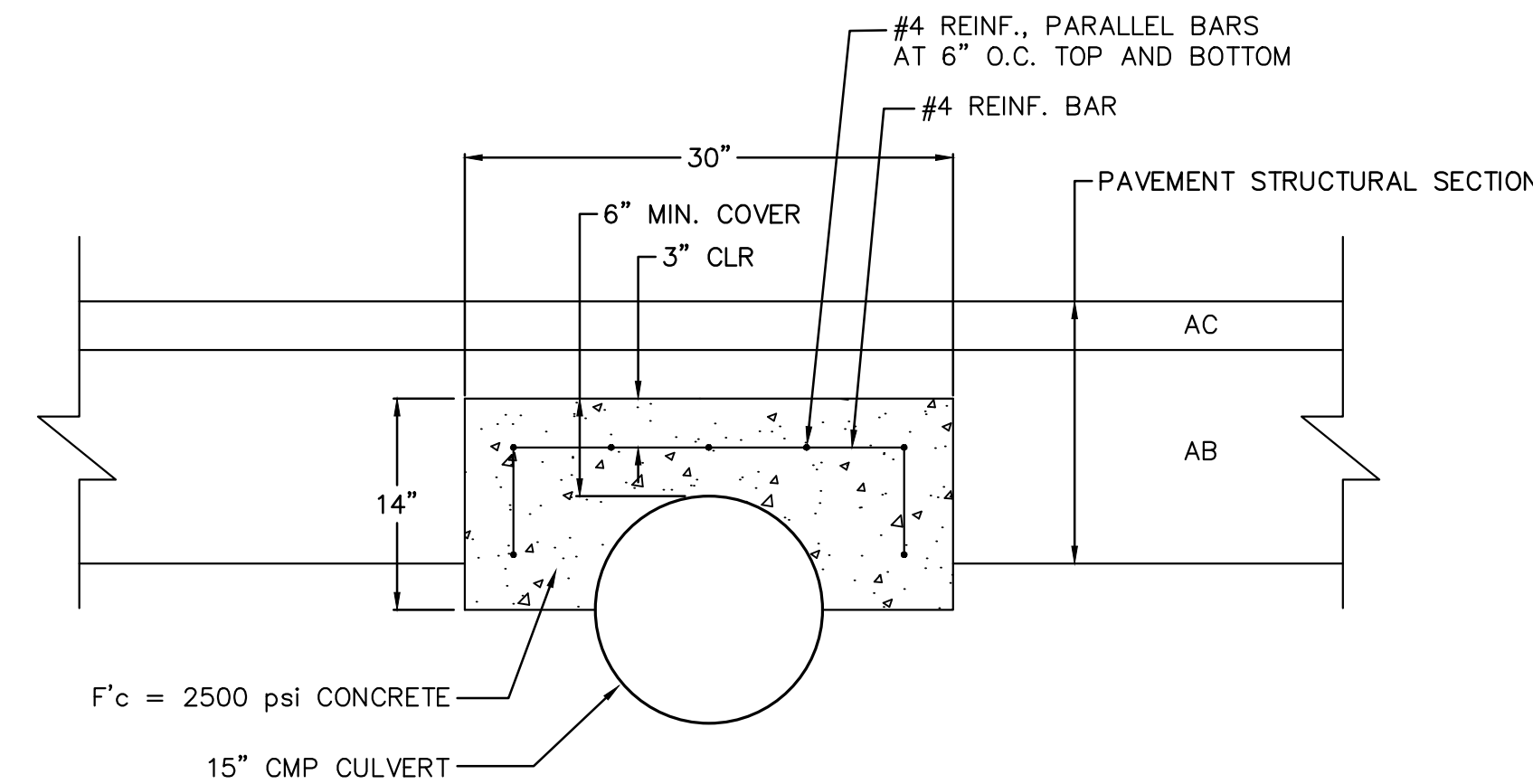


VERIFY SUB-GRADE WITH GEOTEC. ENGR. PRIOR TO FORMING AND CONC. POUR.

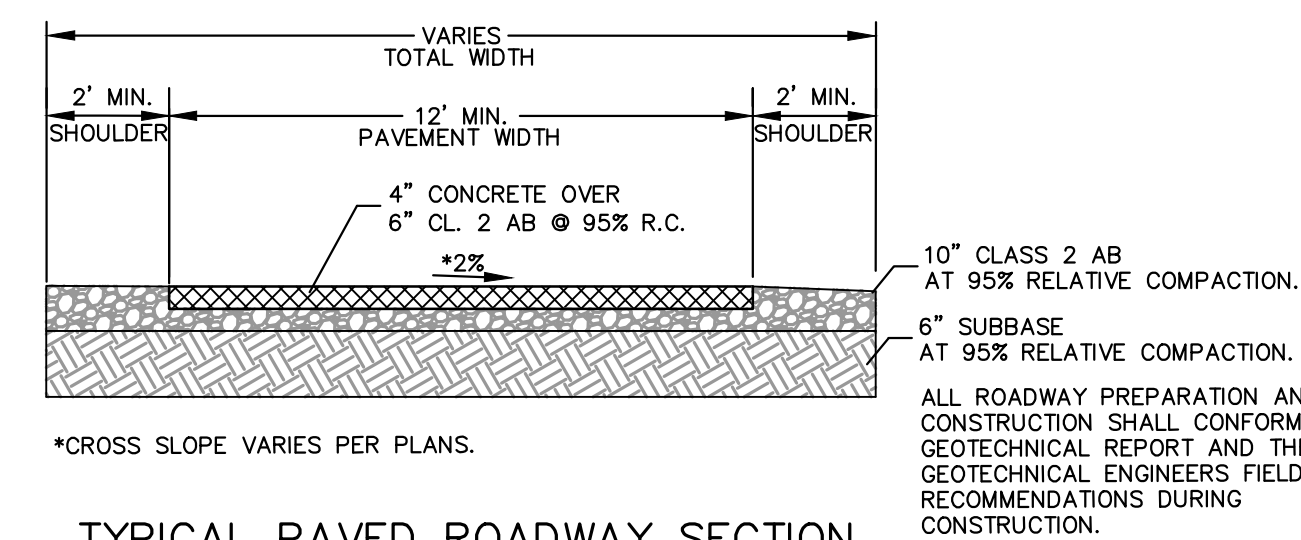
EXTERIOR CONCRETE SLAB
NTS



GARAGE SLAB/FOOTING
NTS



SHALLOW PIPE CONCRETE CAP
NTS

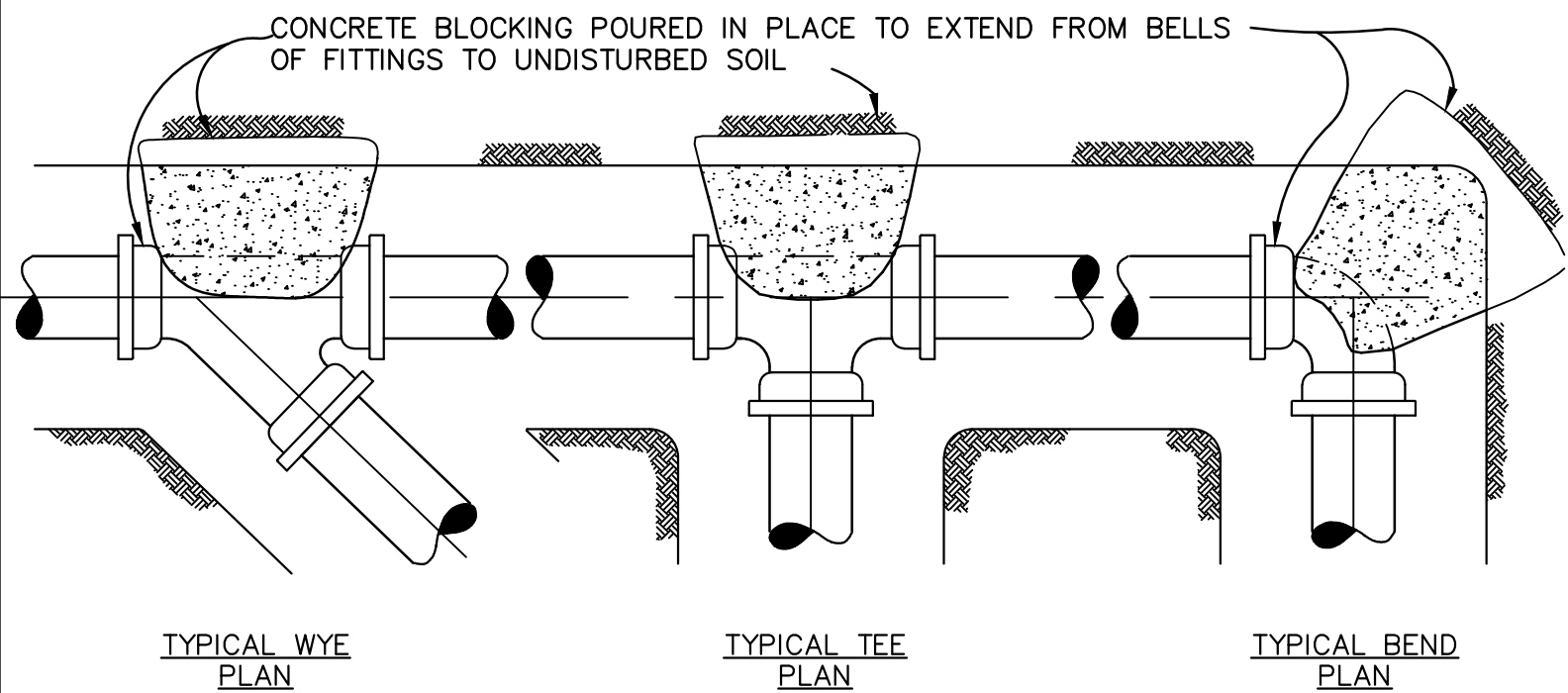


TYPICAL PAVED ROADWAY SECTION
NTS

PRELIMINARY PLANSET
(NOT FOR CONSTRUCTION)
04/20/2026

FIRE PROTECTION CONSTRUCTION

1. CONTRACTOR SHALL COORDINATE FINAL WATER SYSTEM INSPECTIONS AND START UP WITH THE LOCAL FIRE PROTECTION DISTRICT AND THE COUNTY FIRE MARSHAL.
2. ALL FIRE PROTECTION CONSTRUCTION SHALL CONFORM TO CALIFORNIA AMERICAN WATER SYSTEM STANDARDS. ALL PIPE AND FIXTURES CONFORM TO AWWA C900.
3. ON SITE FIRE PROTECTION MAIN CONSTRUCTION SHALL CONFORM TO NATIONAL FIRE PROTECTION ASSOCIATION, NFPA 24, "STANDARD FOR THE INSTALLATION OF PRIVATE FIRE SERVICE MAINS AND THEIR APPURTENANCES."
4. WATER VALVES AND HYDRANT ASSEMBLIES SHALL BE INSTALLED PER C.A. WATER SYSTEM STDs.



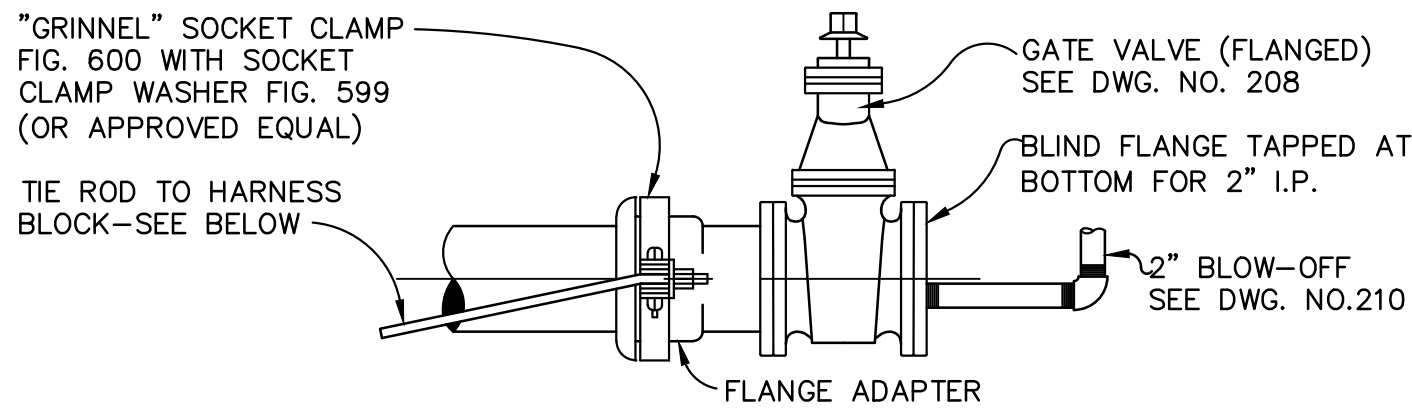
TYPICAL CONCRETE THRUST BLOCKING

PRESSURE	PIPE DIA.	FITTING				
		11 1/4'	22 1/2'	45' AND WYE	90' AND TEE	
150 P.S.I.	4" & 6"	1	1	2	3	
	8"	1	2	3	5	
	10" & 12"	2	4	8	14	
	14"	3	5	9	17	
200 P.S.I.	4" & 6"	1	1	2	4	
	8"	1	2	4	7	
	10" & 12"	3	5	10	19	
	14"	3	6	12	22	

NOTE: THE ABOVE TABLE IS BASED ON AN ASSUMED SOIL BEARING VALUE OF 2000 POUNDS PER SQ. FT. IF BEARING VALUE FOR SOIL IS LESS, THEN AREA SHOWN HEREIN SHOULD BE INCREASED ACCORDINGLY.

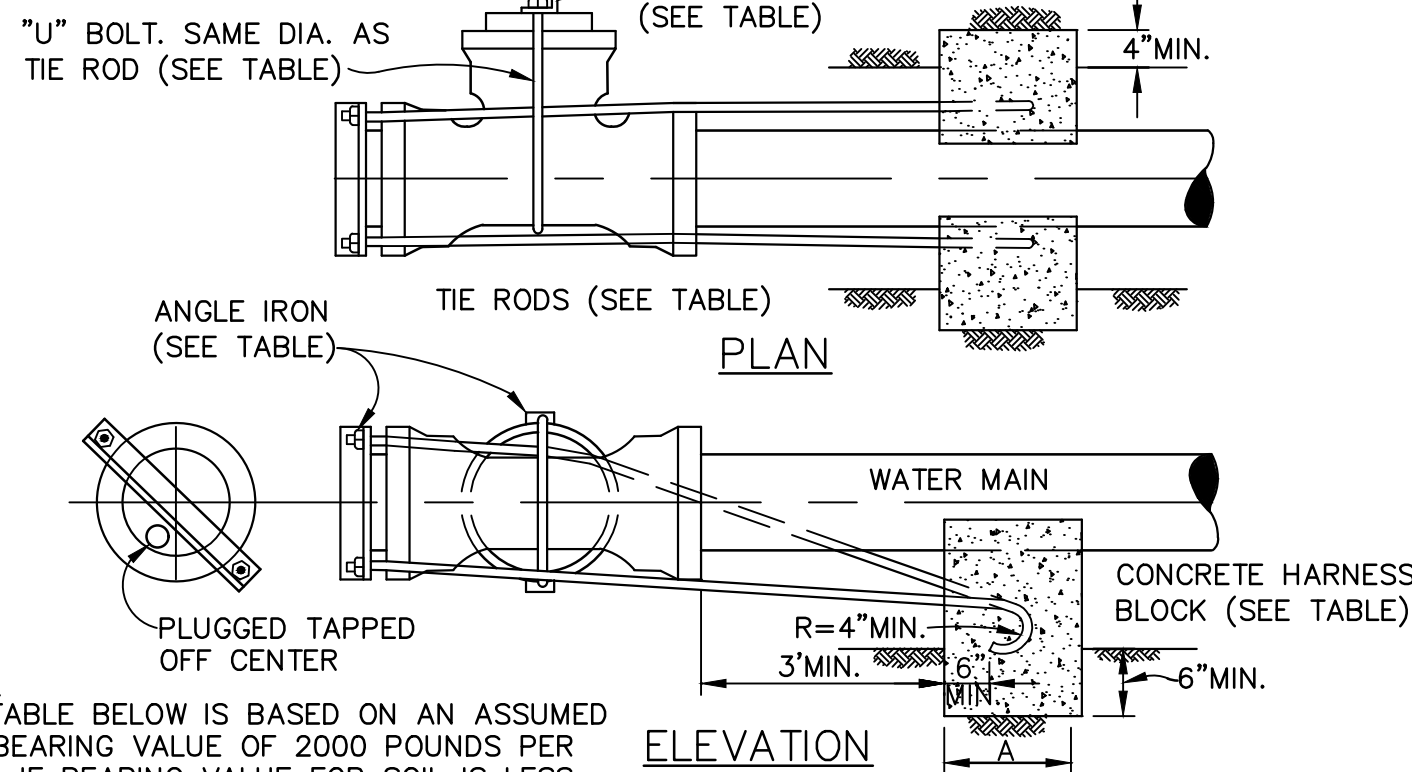
CONCRETE THRUST BLOCKING TABLE

SQ. FT. OF BEARING AREA



HARNESS FOR VALVE WITH PLUG

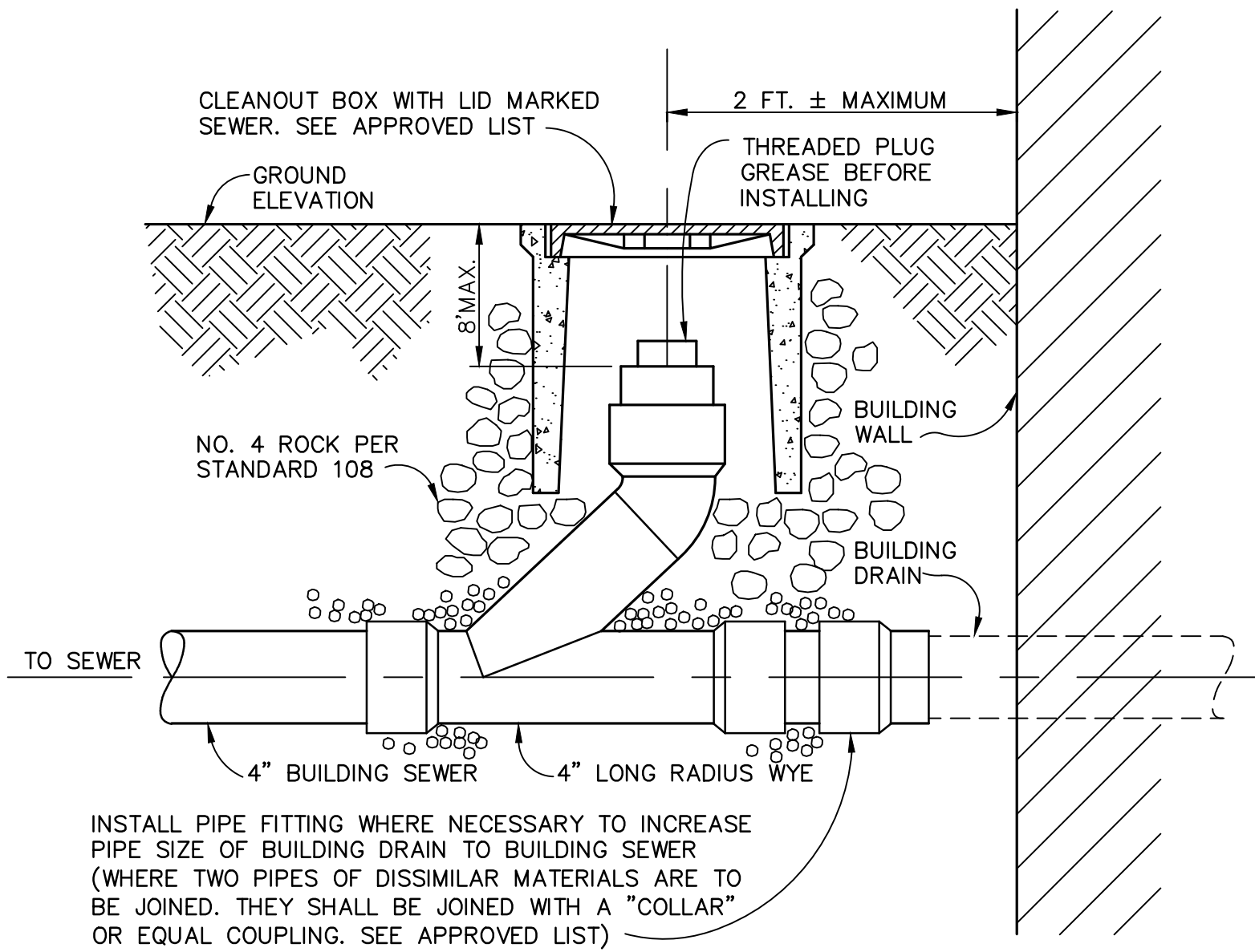
NOTE: AFTER ASSEMBLY ALL IRON PIPE AND MISC. METALS SHALL BE PAINTED WITH A BITUMINOUS PAINT (OR AN APPROVED EQUAL) AS PER MANUFACTURERS RECOMMENDATIONS.



NOTE: THE TABLE BELOW IS BASED ON AN ASSUMED SOIL BEARING VALUE OF 2000 POUNDS PER SQ.FT. IF BEARING VALUE FOR SOIL IS LESS, THAN AREA SHOWN HEREIN SHOULD BE INCREASED ACCORDINGLY.

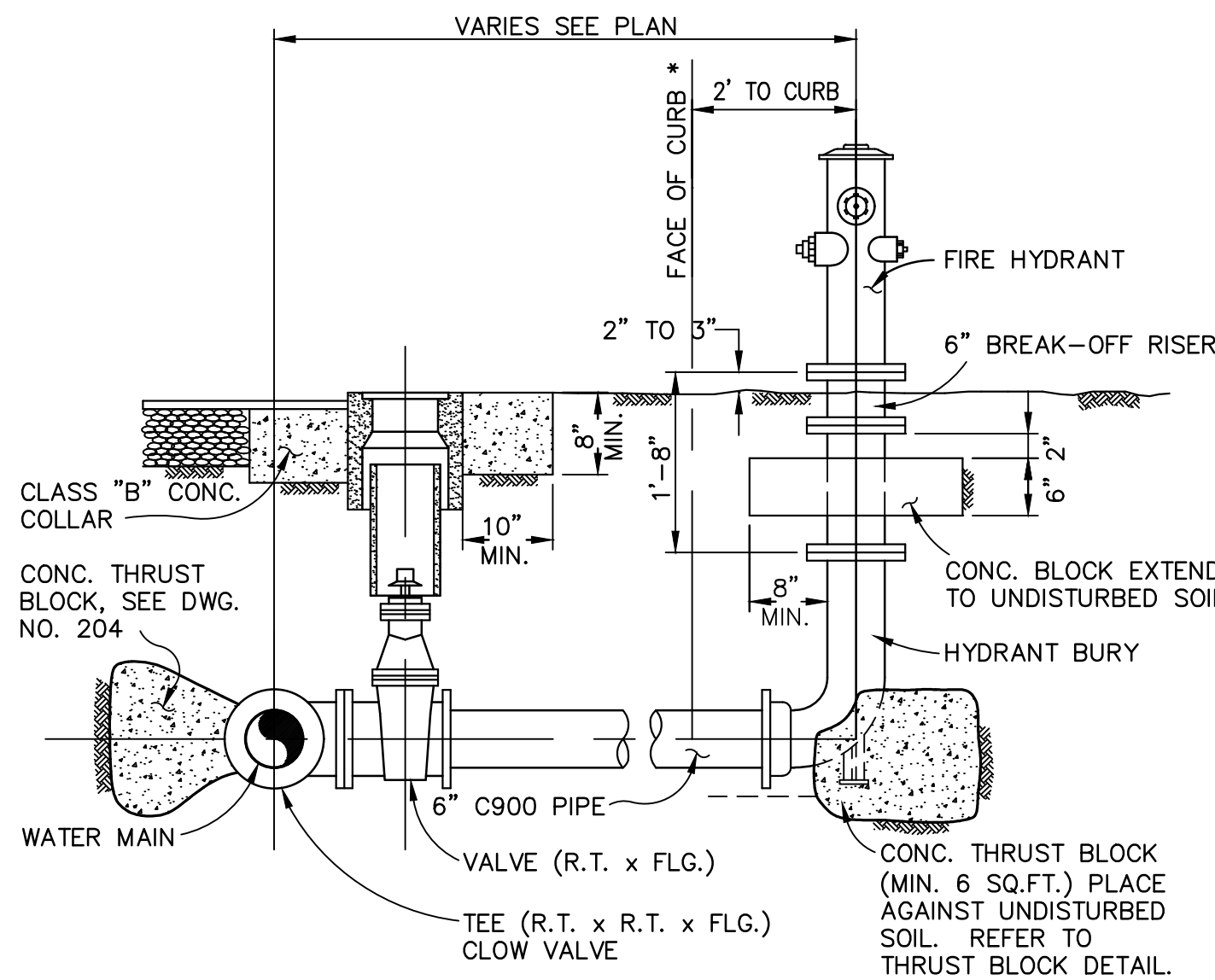
MINIMUM DIMENSIONS				HARNESS BLK SQ. FT. "A"
PIPE SIZE	TIE RODS	ANGLE IRON		
4"	1/2"	2 1/2 x 2 x 1/4	2	2'
6"	5/8"	3 x 3 x 1/4	4	2'
8"	3/4"	3 1/2 x 3 x 1/4	7	3'
10"	1"	4 x 3 x 5/16	11	3'
12"	1 1/8"	4 x 3 x 1/2	15	3'

TYPICAL HARNESS



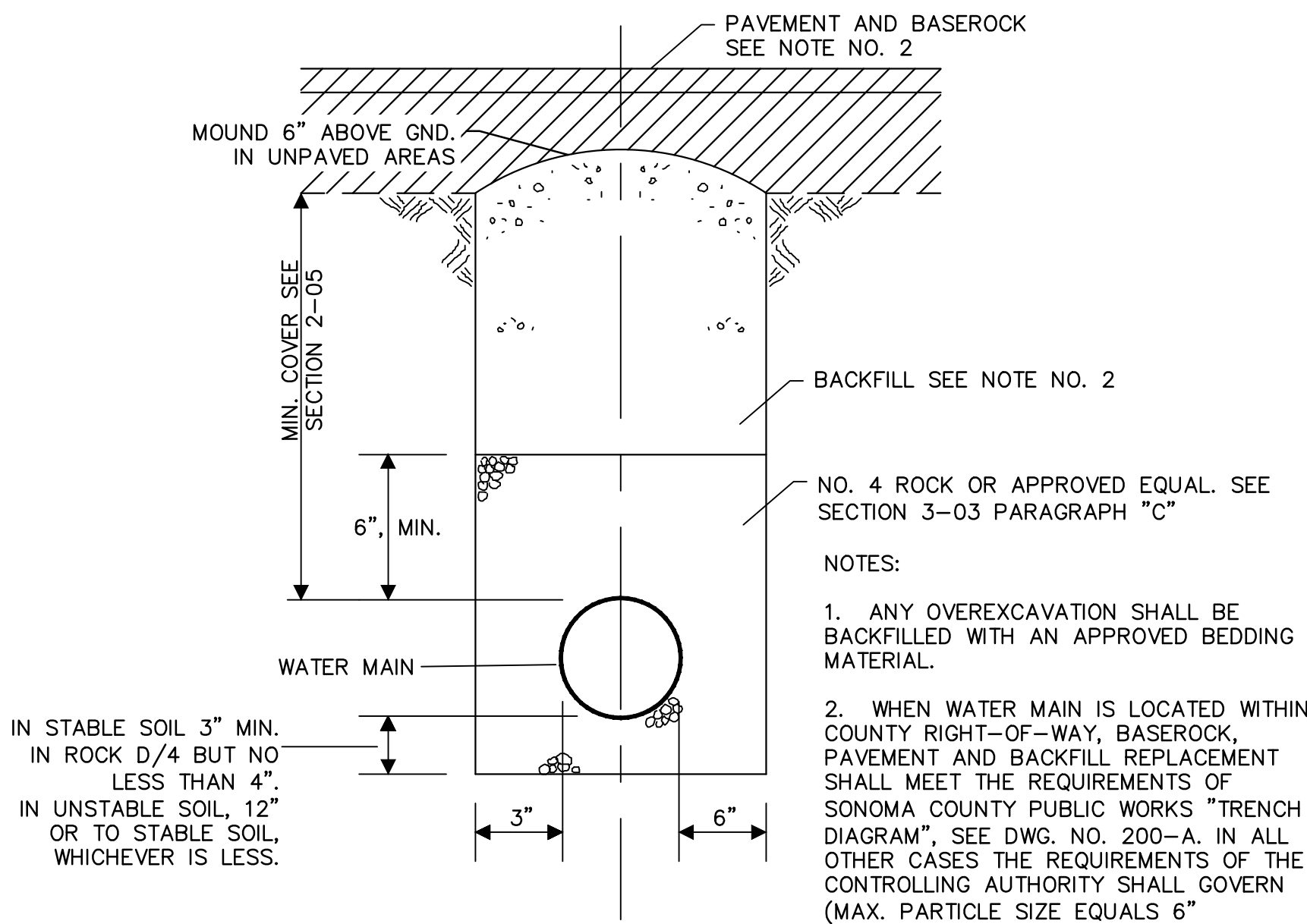
BUILDING CLEANOUT DETAIL

NTS



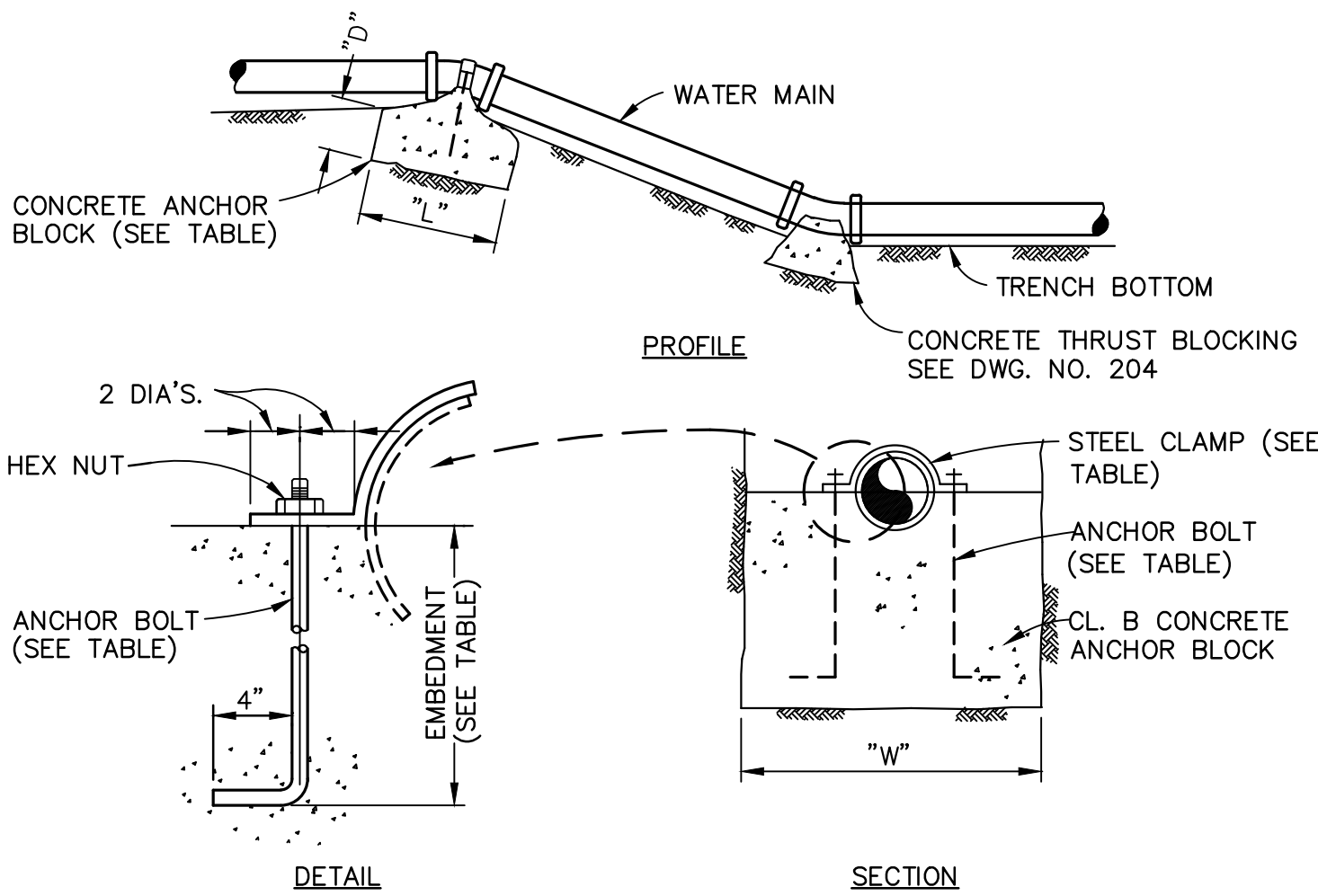
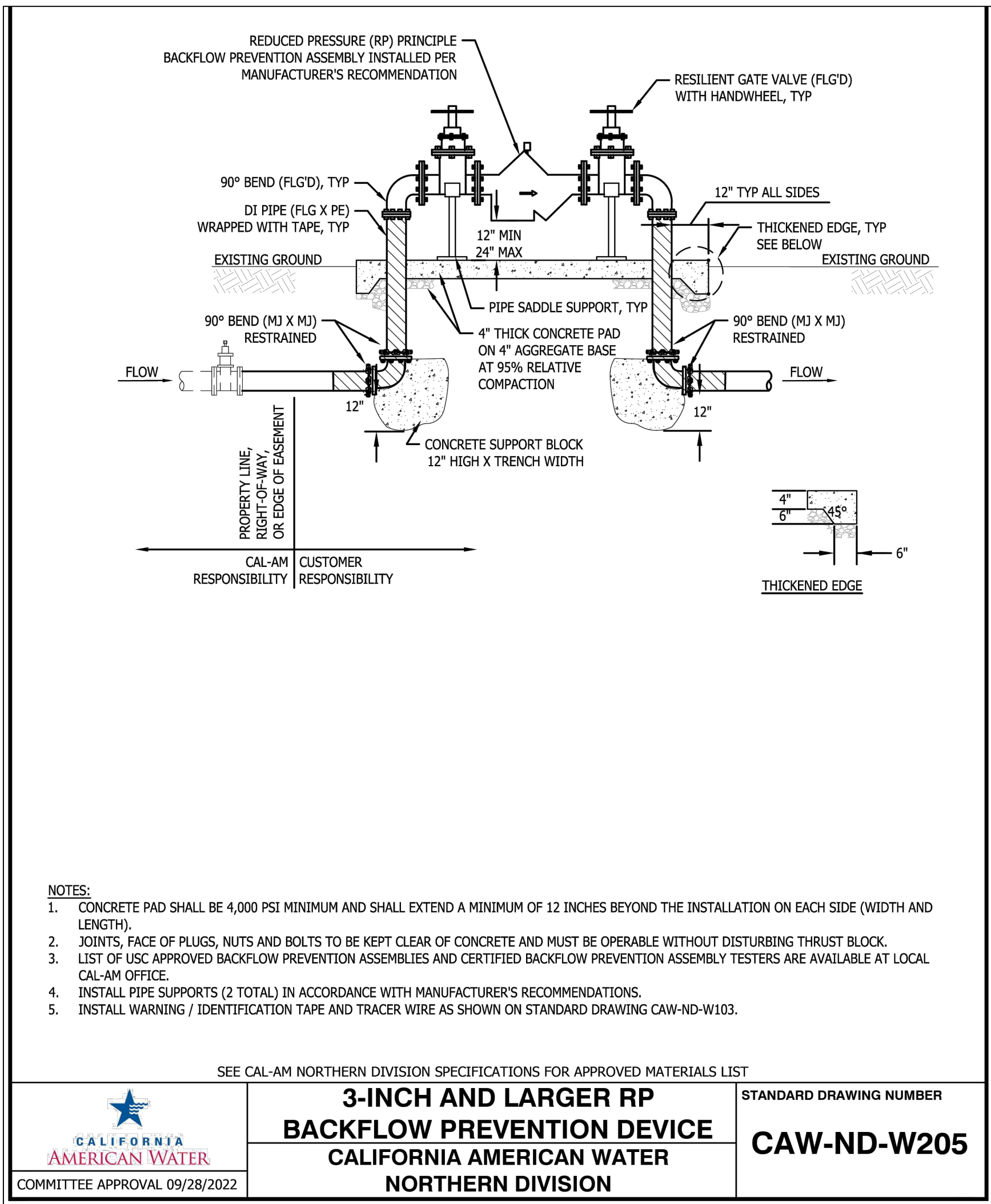
TYPICAL FIRE HYDRANT INSTALLATION

NTS



TYPICAL PIPE BEDDING AND TRENCH DETAILS

NTS



TYPICAL CONCRETE ANCHOR BLOCKS FOR VERTICAL BENDS

PIPE SIZE	ANCHOR BOLT SIZE DIA. x MIN EMBEDMENT		STEEL CLAMP WIDTH x THICKNESS	ANCHOR BLOCK W x D x L	
	45°ELL	22 1/2°ELL		45°ELL	22 1/2°ELL
3"	5/8" x 8"	5/8" x 8"	3" x 1/4"	2'x 1'x 1.5'	2'x 1'x 1'
4"	↓	↓	↓	2'x 1'x 2'	2'x 1'x 1.5'
6"	↓	↓	↓	3'x 2'x 2.5'	3'x 2'x 2.5'
8"	3/4" x 10"	3/4" x 10"	3 1/4" x 1/4"	4'x 2'x 3'	3'x 2'x 2.5'
10"	1" x 17"	1" x 17"	4" x 5/16"	4'x 3'x 4'	3'x 2'x 3.5'

NOTES: THE ABOVE TABLE IS BASED ON AN ASSUMED SOIL BEARING VALUE OF 2000 POUNDS PER SQ.FT. IF BEARING VALUE FOR SOIL IS LESS, THAN AREA SHOWN HEREIN SHOULD BE INCREASED ACCORDINGLY.

ALL METAL PARTS SHALL BE PAINTED WITH A BITUMINOUS PAINT (OR AN APPROVED EQUAL) AS PER MANUFACTURERS RECOMMENDATIONS. PAINT STEEL CLAMP BEFORE INSTALLATION, PAINT THREADS AND NUTS AFTER INSTALLATION.

PRELIMINARY PLANSET
(NOT FOR CONSTRUCTION)
04/20/2026

DTA
Drilling Terrones Architecture Inc.

Atterbury & Associates, Inc.
Architecture
Infrastructure
Environments
Civil Engineering - Land Planning
16109 Healdsburg Avenue, Suite D, Healdsburg, CA 95448
Phone: (707) 433-0134, Email: ta@atterburyandassociates.com

Northern Sonoma County Fire Protection District
Station 1 Modernization
20975 Geyserville Ave, Geyserville, CA 95441

Set Purpose

Edits History
Schematic Design Set 1/9/2026
50% Progress Set 3/19/2026

Sheet Subject

UTILITY DETAILS

Program / Owner Proj #

DTA Project #

2525

AX.X

GENERAL NOTES

1.

CONTRACTOR AGREES THAT HE SHALL ASSUME SOLE AND COMPLETE RESPONSIBILITY FOR JOB SITE CONDITIONS DURING THE COURSE OF CONSTRUCTION OF THIS PROJECT, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY; THAT THIS REQUIREMENT SHALL APPLY CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS; AND THAT THE CONTRACTOR SHALL DEFEND, INDEMNIFY AND HOLD THE OWNER AND SWICEGOOD CIVIL ENGINEERING, INC HARMLESS FROM ANY AND ALL LIABILITY REAL OR ALLEGED, IN CONNECTION WITH THE PERFORMANCE OR WORK ON THIS PROJECT, EXCEPTING FOR LIABILITY ARISING FROM THE SOLE NEGLIGENCE OF THE OWNER OR ENGINEER.

2.

THE ENGINEER ASSUMES NO RESPONSIBILITY EXCEPT FOR THE ADEQUACY OF HIS DESIGN CONTAINED HEREIN.

3.

NO GUARANTEE IS INTENDED THAT UNDERGROUND OBSTRUCTIONS NOT SHOWN ON THE PLAN MAY NOT BE ENCOUNTERED. THOSE SHOWN ON THE PLAN ARE BASED ON THE BEST INFORMATION AVAILABLE AND THE CONTRACTOR IS CAUTIONED THAT THE OWNER AND THE ENGINEER ASSUME NO RESPONSIBILITY FOR ANY OBSTRUCTIONS, EITHER SHOWN OR NOT SHOWN ON THESE PLANS. UNDERGROUND SERVICE ALERT (USA) SHALL BE NOTIFIED OF UNDERGROUND WORK 48 HOURS IN ADVANCE AS NECESSARY FOR UNDERGROUND UTILITY LOCATION.

4.

ANY DISCREPANCY DISCOVERED BY THE CONTRACTOR IN THESE PLANS OR ANY FIELD CONDITIONS DISCOVERED BY THE CONTRACTOR THAT MY DELAY OR OBSTRUCT THE PROPER COMPLETION OF THE WORK PER THESE PLANS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER AND THE OWNER IMMEDIATELY UPON DISCOVERY. SAID NOTIFICATION SHALL BE IN WRITING.

5.

THE CONTRACTOR SHALL SUBMIT ALL EQUIPMENT SPECIFICATIONS TO THE ENGINEER FOR APPROVAL PRIOR TO INSTALLATION. ALL MATERIALS TO BE USED ARE TO BE APPROVED BY THE ENGINEER PRIOR TO THEIR PLACEMENT. ANY MATERIALS INSTALLED PRIOR TO APPROVAL ARE SUBJECT TO REMOVAL.

6.

ALL EARTHWORK AND GRADING WORK SHALL CONFORM WITH THE GEOTECHNICAL INVESTIGATION PREPARED BY RGH GEOTECHNICAL CONSULTANTS, DATED JULY 15, 2003, JOB NO. 2057.01.04.1. THIS INVESTIGATION IS MADE A PART OF THIS GRADING PLAN.

GRADING AND DRAINAGE NOTES

1.

PERFORM GRADING AND DRAINAGE IMPROVEMENTS IN ACCORDANCE WITH CHAPTER 11 AND 11A OF THE SONOMA COUNTY CODE (SCC), APPLICABLE SONOMA COUNTY REGULATIONS, AND TO THE RECOMMENDATIONS OF THE SOILS REPORT PREPARED BY RGH CONSULTANTS, TITLED: GEOTECHNICAL STUDY GEYSERVILLE FIRE PROTECTION DISTRICT HEADQUARTERS, 20975 GEYSERVILLE AVENUE, GEYSERVILLE, CALIFORNIA DATED JULY 15, 2023.

2.

ALL WORK SHALL BE DONE IN COMPLIANCE WITH THE APPROVED PLANS AND SPECIFICATIONS. THE APPROVED PLANS AND SPECIFICATIONS SHALL NOT BE CHANGED WITHOUT THE WRITTEN APPROVAL OF THE SONOMA COUNTY PERMIT AND RESOURCE MANAGEMENT DEPARTMENT (PRMD). PROPOSED MODIFICATIONS TO THE APPROVED PLANS AND SPECIFICATIONS SHALL BE SUBMITTED TO PRMD IN WRITING, TOGETHER WITH ALL NECESSARY TECHNICAL INFORMATION AND DESIGN DETAILS. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE PROPERTY OWNER AND ENGINEER OF RECORD, IF APPLICABLE, UPON DISCOVERING DISCREPANCIES, ERRORS, OR OMISSIONS IN THE APPROVED PLANS. PRIOR TO PROCEEDING, THE PROPERTY OWNER SHALL HAVE THE APPROVED PLANS REVISED TO CLARIFY IDENTIFIED DISCREPANCIES, ERRORS, OR OMISSIONS. PRMD MAY REQUIRE UNAUTHORIZED WORK TO BE REDONE OR REMOVED TO VERIFY COMPLIANCE WITH SCC. PRMD MAY INITIATE ENFORCEMENT ACTION AND SEEK THE IMPOSITION OF CIVIL PENALTIES FOR VIOLATIONS OF SCC.

3.

THE GRADING OR DRAINAGE PERMIT AND A COPY OF THE APPROVED PLANS SHALL BE MAINTAINED ON THE PROJECT SITE THROUGHOUT THE DURATION OF CONSTRUCTION ACTIVITIES.

4.

PRMD MAY ORDER THAT ANY WORK STOP IMMEDIATELY IF IT IS PERFORMED CONTRARY TO CHAPTER 11 AND 11A OF THE SCC, THE APPROVED PLANS AND SPECIFICATIONS, PERMIT CONDITIONS, OR ANY WORK THAT HAS BECOME HAZARDOUS TO PROPERTY OR THE PUBLIC. A GRADING OR DRAINAGE PERMIT MAY BE SUSPENDED, REVOKED, OR MODIFIED BY PRMD IN ACCORDANCE WITH SCC 11.24.080.

5.

ISSUANCE OF A GRADING OR DRAINAGE PERMIT BY PRMD DOES NOT ELIMINATE THE RESPONSIBILITY OF THE PROPERTY OWNER TO SECURE PERMITS FROM OTHER AGENCIES WITH REGULATORY RESPONSIBILITIES FOR THE USES AND CONSTRUCTION ACTIVITIES ASSOCIATED WITH THE WORK SHOWN ON THE APPROVED PLANS. FAILURE TO OBTAIN ALL REQUIRED PERMITS MAY RESULT IN FINES FROM OTHER AGENCIES.

6.

EXISTING DRAINAGE COURSES RECEIVING WATERS FROM THE PROJECT SITE AND LOCATED THROUGHOUT THE PROJECT SITE SHALL REMAIN OPEN AND CLEAR OF DEBRIS TO PROPERLY CONVEY STORM WATER. IF EXISTING DRAINAGE COURSES RECEIVING WATERS FROM THE PROJECT SITE ARE LOCATED IN THE COUNTY RIGHT-OF-WAY AND NEED MAINTENANCE, CONTACT THE DEPARTMENT OF TRANSPORTATION AND PUBLIC WORKS AT (707) 565-2231 FOR FURTHER ASSISTANCE. IN ANY EVENT, THE PROPERTY OWNER AND/OR CONTRACTOR SHALL BE HELD LIABLE FOR ANY DAMAGE DUE TO OBSTRUCTING NATURAL DRAINAGE PATTERNS.

7.

THE CONTRACTOR SHALL CONTACT THE UNDERGROUND SERVICE ALERT (USA), AT 811, AT LEAST TWO WORKING DAYS, BUT NOT MORE THAN 14 CALENDAR DAYS, PRIOR TO EXCAVATION. THE CONTRACTOR SHALL UNCOVER RELEVANT UTILITIES TO VERIFY THEIR LOCATION AND ELEVATION. IF UNEXPECTED OR CONFLICTING UTILITIES ARE ENCOUNTERED DURING EXCAVATION, NOTIFY USA, THE UTILITY OWNER, AND/OR THE ENGINEER OF RECORD, IF APPLICABLE, IMMEDIATELY. UTILITIES INCLUDE BUT ARE NOT LIMITED TO WATER, SEWER, ELECTRICAL, GAS, TELEPHONE, AND CABLE/TV. THE EXCAVATOR SHALL DELINEATE WITH PAINT OR OTHER SUITABLE MARKINGS THE AREA TO BE EXCAVATED.

8.

IN THE EVENT CULTURAL RESOURCES (SUCH AS HISTORICAL, ARCHAEOLOGICAL, AND PALEONTOLOGICAL RESOURCES, AND HUMAN REMAINS) ARE DISCOVERED DURING GRADING OR OTHER CONSTRUCTION ACTIVITIES, WORK SHALL IMMEDIATELY BE HALTED WITHIN THE VICINITY OF THE FIND. THE NORTHWEST INFORMATION CENTER SHALL BE NOTIFIED AT (707) 568-8455. A QUALIFIED ARCHEOLOGIST SHALL BE CONSULTED FOR AN ON-SITE EVALUATION. ADDITIONAL MITIGATION MAY BE REQUIRED BY THE COUNTY PER THE ARCHEOLOGIST'S RECOMMENDATIONS AND SCC 11.16.050. IF HUMAN BURIALS OR HUMAN REMAINS ARE ENCOUNTERED, THE CONTRACTOR SHALL ALSO NOTIFY THE COUNTY CORONER AT (707) 565-5070.

9.

SHOULD GRADING OPERATIONS ENCOUNTER HAZARDOUS MATERIALS, OR WHAT APPEAR TO BE HAZARDOUS MATERIALS, STOP WORK IMMEDIATELY IN THE CONTAMINATED AREA AND CONTACT 911 OR THE APPROPRIATE AGENCY FOR FURTHER INSTRUCTION.

10.

RETAINING WALLS, UNLESS EXEMPTED PER SCC 7.13(A)(3)4, ARE NOT APPROVED UNDER A GRADING PERMIT. A SEPARATE BUILDING PERMIT IS REQUIRED.

11.

EQUIPMENT SHALL NOT CROSS OR DISTURB CHANNELS OF ACTIVELY FLOWING STREAMS WITHOUT A PRMD APPROVED ROILING PERMIT AND BEST MANAGEMENT PRACTICES (SCC 23.1 AND 11.16.060.D).

12.

GRADING AND DRAINAGE IMPROVEMENTS SHALL BE SET BACK FROM LAKES, PONDS, STREAMS, AND WETLANDS IN COMPLIANCE WITH THE REQUIREMENTS OF SCC 11.16.100, 11.16.120, AND 11.16.130. EXISTING VEGETATION SHALL BE RETAINED IN STREAM SETBACK AREAS TO FILTER SOIL AND OTHER POLLUTANTS CARRIED IN STORM WATER.

13.

EXCESS SOIL SHALL BE REMOVED FROM THE PROJECT SITE UNLESS DEPICED TO REMAIN ON SITE PER THE APPROVED PLAN. THE SITE RECEIVING SOIL MAY REQUIRE A GRADING PERMIT UNLESS EXEMPTED BY SCC 11.04.010.C.

14.

CONTOURS, ELEVATIONS, AND SHAPES OF FINISHED SURFACES SHALL BE BLENDED WITH ADJACENT NATURAL TERRAIN TO ACHIEVE A CONSISTENT GRADE AND NATURAL APPEARANCE. BORDERS OF CUT SLOPES AND FILLS SHALL BE ROUNDED OFF TO A MINIMUM RADIUS OF FIVE FEET TO BLEND WITH THE NATURAL TERRAIN.

15.

FILL MATERIAL SHALL NOT INCLUDE ORGANIC, FROZEN, OR OTHER DELETERIOUS MATERIALS. NO ROCK OR SIMILAR IRREDUCIBLE MATERIAL GREATER THAN SIX INCHES IN ANY DIMENSION SHALL BE INCLUDED IN FILLS EXCEPT WHERE APPROVED BY THE SOILS ENGINEER. FILLS SHALL BE CONSTRUCTED IN LIFTS NOT EXCEEDING EIGHT INCHES IN DEPTH. COMPLETED FILLS SHALL BE STABLE, WELL-INTEGRATED, AND BONDED TO ADJACENT MATERIALS AND THE MATERIALS ON WHICH THEY REST. FILLS SHALL BE COMPETENT TO SUPPORT ANTICIPATED LOADS AND BE STABLE AT THE DESIGN SLOPES SHOWN ON THE APPROVED PLANS AND SPECIFICATIONS OR AS DIRECTED BY THE SOILS ENGINEER.

16.

GROUND SURFACES SHALL BE PREPARED TO RECEIVE FILL BY REMOVING VEGETATION, TOPSOIL, AND OTHER UNSUITABLE MATERIALS, AND SCARIFYING THE GROUND TO PROVIDE A BOND WITH THE FILL MATERIAL.

17.

FILL SHALL NOT BE PLACED ON NATURAL SLOPES STEEPER THAN 2H:1V (50 PERCENT).

18.

FILLS INTENDED TO SUPPORT STRUCTURES OR SURCHARGES SHALL BE COMPACTED TO A MINIMUM OF 90 PERCENT OF MAXIMUM DRY DENSITY, AS DETERMINED BY ASTM D 1557, MODIFIED PROCTOR. A HIGHER COMPACTION PERCENTAGE MAY BE REQUIRED BY THE SOILS ENGINEER.

19.

FILLS NOT INTENDED TO SUPPORT STRUCTURES OR SURCHARGES SHALL BE COMPACTED AS FOLLOWS:

(1)

FILL GREATER THAN THREE FEET IN DEPTH SHALL BE COMPACTED TO THE DENSITY SPECIFIED BY THE SOILS ENGINEER.

(2)

FILLS NO GREATER THAN THREE FEET IN DEPTH SHALL BE COMPACTED TO THE DENSITY NECESSARY FOR THE INTENDED USE OR AS DIRECTED BY THE SOILS ENGINEER.

EROSION PREVENTION AND SEDIMENT CONTROL NOTES

1.

PERFORM EROSION PREVENTION AND SEDIMENT CONTROL IN ACCORDANCE WITH CHAPTER 11 AND 11A OF THE SONOMA COUNTY CODE (SCC).

2.

THE APPROVED PLANS SHALL CONFORM TO THE PERMIT AND RESOURCE MANAGEMENT DEPARTMENT'S (PRMD) EROSION PREVENTION AND SEDIMENT CONTROL BEST MANAGEMENT PRACTICES (BMP'S) GUIDE AS POSTED ON THE PRMD WEBSITE.

3.

THE PROPERTY OWNER IS RESPONSIBLE FOR PREVENTING STORM WATER POLLUTION GENERATED FROM THE CONSTRUCTION SITE YEAR ROUND. WORK SITES WITH INADEQUATE EROSION PREVENTION AND/OR SEDIMENT CONTROL MAY BE SUBJECT TO A STOP WORK ORDER AND/OR ADDITIONAL INSPECTION FEES TO VERIFY COMPLIANCE WITH SCC.

4.

IF DISCREPANCIES OCCUR BETWEEN THESE NOTES, MATERIAL REFERENCED ON THE APPROVED PLANS OR MANUFACTURER'S RECOMMENDATIONS, THEN THE MOST PROTECTIVE SHALL APPLY.

5.

AT ALL TIMES THE PROPERTY OWNER IS RESPONSIBLE FOR OBTAINING AND COMPLYING WITH THE STATE OF CALIFORNIA NATIONAL POLLUTANT DISCHARGE ELIMINATION SYSTEM (NPDES) GENERAL PERMIT FOR STORM WATER DISCHARGES ASSOCIATED WITH CONSTRUCTION AND LAND DISTURBING ACTIVITIES SUCH AS CLEARING, GRADING, EXCAVATION, STOCKPILING, AND RECONSTRUCTION OF EXISTING FACILITIES INVOLVING REMOVAL AND REPLACEMENT.

6.

THE PROPERTY OWNER MUST IMPLEMENT AN EFFECTIVE COMBINATION OF EROSION PREVENTION AND SEDIMENT CONTROL ON ALL DISTURBED AREAS DURING THE RAINY SEASON (OCTOBER 1 - APRIL 30). GRADING AND DRAINAGE IMPROVEMENT SHALL BE PERMITTED DURING THE RAINY SEASON ONLY WHEN ON-SITE SOIL CONDITIONS PERMIT THE WORK TO BE PERFORMED IN COMPLIANCE WITH SCC.

7.

DURING THE RAINY SEASON, STORM WATER BMP'S REFERENCED OR DETAILED IN PRMD'S BMP GUIDE SHALL BE IMPLEMENTED AND FUNCTIONAL ON THE SITE AT ALL TIMES AND THE AREA OF ERODIBLE LAND EXPOSED AT ANY ONE TIME DURING THE WORK SHALL NOT EXCEED ONE ACRE OR 20 PERCENT OF THE PERMITTED WORK AREA, WHICHEVER IS GREATER, AND THE TIME OF EXPOSURE SHALL BE MINIMIZED TO THE MAXIMUM EXTENT PRACTICABLE.

8.

DURING THE NON-RAINY SEASON, ON ANY DAY WHEN THE NATIONAL WEATHER SERVICE FORECAST IS A CHANCE OF RAIN OF 30 PERCENT OR GREATER WITHIN THE NEXT 24 HOURS, STORM WATER BMP'S REFERENCED OR DETAILED IN PRMD'S BMP GUIDE SHALL BE IMPLEMENTED AND FUNCTIONAL ON THE SITE TO PREVENT SOIL AND OTHER POLLUTANT DISCHARGES. AT ALL OTHER TIMES, BMP'S SHOULD BE STORED ON SITE IN PREPARATION FOR INSTALLATION PRIOR TO RAIN EVENTS.

9.

EROSION PREVENTION AND SEDIMENT CONTROL BMP'S SHALL BE INSPECTED BY THE PROPERTY OWNER BEFORE FORECASTED STORM EVENTS AND AFTER STORM EVENTS TO ENSURE BMP'S ARE FUNCTIONING PROPERLY. EROSION PREVENTION AND SEDIMENT CONTROL BMP'S THAT HAVE FAILED OR ARE NO LONGER EFFECTIVE SHALL BE PROMPTLY REPLACED. EROSION PREVENTION AND SEDIMENT CONTROL BMP'S SHALL BE MAINTAINED UNTIL DISTURBED AREAS ARE STABILIZED.

10.

THE LIMITS OF GRADING SHALL BE DEFINED AND MARKED ON SITE TO PREVENT DAMAGE TO SURROUNDING TREES AND OTHER VEGETATION. PRESERVATION OF EXISTING VEGETATION SHALL OCCUR TO THE MAXIMUM EXTENT PRACTICABLE. ANY EXISTING VEGETATION WITHIN THE LIMITS OF GRADING THAT IS TO REMAIN UNDISTURBED BY THE WORK SHALL BE IDENTIFIED AND PROTECTED FROM DAMAGE BY MARKING, FENCING, OR OTHER MEASURES.

11.

CHANGES TO THE EROSION PREVENTION AND SEDIMENT CONTROL PLAN MAY BE MADE TO RESPOND TO FIELD CONDITIONS IF THE ALTERNATIVE BMP'S ARE EQUIVALENT OR MORE PROTECTIVE THAN THE BMP'S SHOWN ON THE APPROVED PLANS. ALTERNATIVE BMP'S ARE SUBJECT TO REVIEW AND APPROVAL BY PRMD STAFF.

12.

DISCHARGES OF POTENTIAL POLLUTANTS FROM CONSTRUCTION SITES SHALL BE PREVENTED USING SOURCE CONTROLS TO THE MAXIMUM EXTENT PRACTICABLE. POTENTIAL POLLUTANTS INCLUDE BUT ARE NOT LIMITED TO: SEDIMENT, TRASH, NUTRIENTS, PATHOGENS, PETROLEUM HYDROCARBONS, METALS, CONCRETE, CEMENT, ASPHALT, LIME, PAINT, STAINS, GLUES, WOOD PRODUCTS, PESTICIDES, HERBICIDES, CHEMICALS, HAZARDOUS WASTE, SANITARY WASTE, VEHICLE OR EQUIPMENT WASH WATER, AND CHLORINATED WATER.

13.

ENTRANCE(S) TO THE CONSTRUCTION SITE SHALL BE MAINTAINED IN A CONDITION THAT WILL PREVENT TRACKING OR FLOWING OF POTENTIAL POLLUTANTS OFFSITE. POTENTIAL POLLUTANTS DEPOSITED ON PAVED AREAS WITHIN THE COUNTY RIGHT-OF-WAY, SUCH AS ROADWAYS AND SIDEWALKS, SHALL BE PROPERLY DISPOSED OF AT THE END OF EACH WORKING DAY OR MORE FREQUENTLY AS NECESSARY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR CLEANING CONSTRUCTION VEHICLES LEAVING THE SITE ON A DAILY BASIS TO PREVENT DUST, SILT, AND DIRT FROM BEING RELEASED OR TRACKED OFFSITE. ALL SEDIMENT DEPOSITED ON PAVED ROADWAYS SHALL BE REMOVED AT THE END OF EACH WORKING DAY OR MORE OFTEN, AS NECESSARY.

MATERIALS

SEED MIX

APPLICATION RATE (POUNDS PER ACRE)

Bromus mollis (BLANDO BROME)

40

Trifolium hirtum (HYKON ROSE CLOVER)

20

FERTILIZER

16-20-0 & 15% SULPHUR

500

MULCH

STRAW

4000

HYDRAULIC STABILIZING*

M-BINDER OR SENTINEL

75-100

EQUIVALENT MATERIAL

PER MANUFACTURER

*NON-ASPHALTIC, DERIVED FROM PLANTS

14.

ALL DISTURBED AREAS SHALL BE PROTECTED BY USING EROSION PREVENTION BMP'S TO THE MAXIMUM EXTENT PRACTICABLE, SUCH AS ESTABLISHING VEGETATION COVERAGE, HYDROSEEDING, STRAW MULCH, GEOTEXTILES, PLASTIC COVERS, BLANKETS, OR MATS. TEMPORARY REVEGETATION SHALL BE INSTALLED AS SOON AS PRACTICAL AFTER VEGETATION REMOVAL, BUT IN ALL CASES PRIOR TO OCTOBER 1. PERMANENT REVEGETATION OR LANDSCAPING SHALL BE INSTALLED PRIOR TO FINAL INSPECTION.

15.

WHENEVER IT IS NOT POSSIBLE TO USE EROSION PREVENTION BMP'S ON EXPOSED SLOPES, SEDIMENT CONTROL BMP'S SUCH AS FIBER ROLLS AND SILT FENCES SHALL BE INSTALLED TO PREVENT SEDIMENT MIGRATION. FIBER ROLLS AND SILT FENCES SHALL BE TRENCHED AND KEYED INTO THE SOIL AND INSTALLED ON CONTOUR. SILT FENCES SHALL BE INSTALLED APPROXIMATELY 2 TO 5 FEET FROM TOE OF SLOPE.

16.

HYDROSEEDING SHALL BE CONDUCTED IN A THREE STEP PROCESS. FIRST, EVENLY APPLY SEED MIX AND FERTILIZER TO THE EXPOSED SLOPE. SECOND, EVENLY APPLY MULCH OVER THE SEED AND FERTILIZER. THIRD, STABILIZE THE MULCH IN PLACE. AN EQUIVALENT SINGLE STEP PROCESS, WITH SEED, FERTILIZER, WATER, AND BONDED FIBERS IS ACCEPTABLE.

APPLICATIONS SHALL BE BROADCASTED MECHANICALLY OR MANUALLY AT THE RATES SPECIFIED BELOW. SEED MIX AND FERTILIZER SHALL BE WORKED INTO THE SOIL BY ROLLING OR TAMPING. IF STRAW IS USED AS MULCH, STRAW SHALL BE DERIVED FROM WHEAT, RICE, OR BARLEY AND BE APPROXIMATELY SIX TO EIGHT INCHES IN LENGTH. STABILIZATION OF MULCH SHALL BE DONE HYDRAULICALLY BY APPLYING AN EMULSION OR MECHANICALLY BY CRIMPING OR PUNCHING THE MULCH INTO THE SOIL. EQUIVALENT METHODS AND MATERIALS MAY BE USED ONLY IF THEY ADEQUATELY PROMOTE VEGETATION GROWTH AND PROTECT EXPOSED SLOPES.

GRADING AND DRAINAGE INSPECTION NOTES

1.

CONTRACTOR AND THE PROPERTY OWNER SHALL BE RESPONSIBLE FOR THE WORK TO BE PERFORMED IN COMPLIANCE WITH THE APPROVED PLANS AND SPECIFICATIONS, CHAPTER 11 AND 11A OF THE SONOMA COUNTY CODE (SCC), AND ANY PERMIT CONDITIONS. WORK SHALL BE SUBJECT TO INSPECTION AS REQUIRED BY THE SONOMA COUNTY PERMIT AND RESOURCE MANAGEMENT DEPARTMENT (PRMD) TO VERIFY COMPLIANCE. THE CONTRACTOR SHALL CONSULT THE PROJECT JOB CARD FOR COORDINATION OF INSPECTION REQUESTS.

2.

PRIOR TO THE START OF ANY GRADING OR DRAINAGE WORK, THE CONTRACTOR SHALL HAVE A PRE-CONSTRUCTION CONSULTATION WITH PRMD STAFF TO DISCUSS THE SCOPE OF THE PROJECT, PERMIT CONDITIONS, REQUIRED INSPECTIONS, APPROPRIATE APPLICATION OF BEST MANAGEMENT PRACTICES (BMP'S) AND ANY OTHER CONSTRUCTION ISSUES.

3.

INSPECTION REQUESTS SHALL BE MADE THROUGH THE SONOMA COUNTY AUTOMATED INSPECTION REQUEST SYSTEM (SELECTRON), AT PHONE NUMBER (707) 565-3551.

4.

PRMD MAY REQUIRE PROFESSIONAL INSPECTIONS AND CERTIFICATIONS TO VERIFY PROPER COMPLETION OF THE WORK, WHERE THE USE OF PROFESSIONAL PERSONNEL IS REQUIRED, THESE PERSONNEL SHALL IMMEDIATELY REPORT IN WRITING TO PRMD AND THE CONTRACTOR ANY INSTANCE OF WORK NOT IN COMPLIANCE WITH THE APPROVED PLANS, SPECIFICATIONS, OR ANY PERMIT CONDITIONS. IF PROFESSIONAL PERSONNEL IS CHANGED DURING THE COURSE OF THE WORK, THE WORK SHALL BE STOPPED UNTIL THE REPLACEMENT INDIVIDUAL HAS NOTIFIED PRMD IN WRITING OF THEIR AGREEMENT TO ACCEPT RESPONSIBILITY FOR APPROVAL OF THE COMPLETED WORK WITHIN THE AREA OF THEIR TECHNICAL COMPETENCE.

5.

PRMD SHALL FINAL A PERMIT WHEN ALL WORK, INCLUDING THE INSTALLATION OF ALL DRAINAGE IMPROVEMENTS AND THEIR PROTECTIVE DEVICES, AND ALL STORM WATER BMP'S, HAVE BEEN COMPLETED IN COMPLIANCE WITH THE APPROVED PLANS AND SPECIFICATIONS, AND ALL FINAL REPORTS REQUIRED BY SCC 11.14.040.A HAVE BEEN SUBMITTED AND ACCEPTED. FINAL REPORTS MAY INCLUDE: AS-BUILT PLANS, TESTING RECORDS, PROFESSIONAL OPINIONS, AND DECLARATIONS ABOUT COMPLETED WORK FROM PROFESSIONAL PERSONNEL. SIMILAR REPORTS MAY BE REQUIRED AT OTHER STAGES OF THE WORK.

6.

CONTRACTOR SHALL PROVIDE ADEQUATE AND SAFE ACCESS TO THE PROJECT SITE FOR INSPECTION DURING THE PERFORMANCE OF ALL WORK.

7.

DURING CONSTRUCTION ACTIVITIES, THE PROJECT SITE ADDRESS SHALL BE POSTED AS FOLLOWS:

(1)

THE STREET NUMBERS MUST BE AT LEAST FOUR INCHES TALL, WITH A REFLECTIVE SURFACE.

(2)

THE ADDRESS MUST BE VISIBLE FROM BOTH DIRECTIONS ALONG THE ROAD.

(3)

THE ADDRESS MUST BE POSTED AT ALL FORKS IN ANY ACCESS ROAD AND AT THE PROJECT SITE.

Northern Sonoma County Fire Protection District

Station 1 Modernization

20975 Geyserville Ave, Geyserville, CA 95441

Set Purpose

Editions History

Schematic Design Set

1/9/2026

50% Progress Set

3/19/2026

Sheet Subject

SPECIFICATIONS

C8

Program / Owner Proj #

DTA Project #

2525

PRELIMINARY PLANSET
(NOT FOR CONSTRUCTION)
04/20/2026

DTA

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